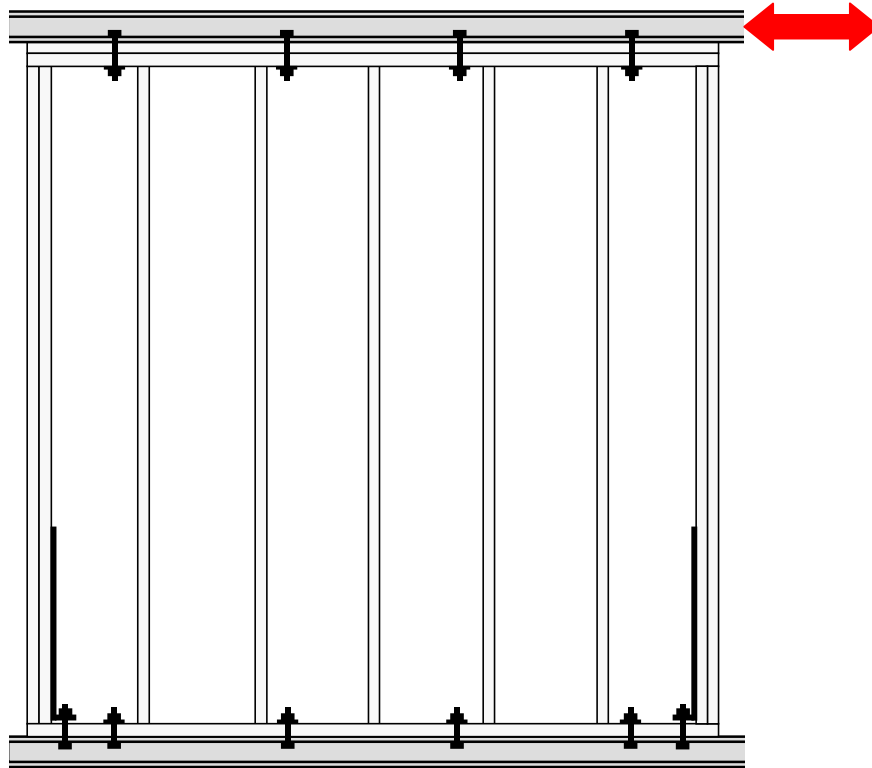


## Walls 08FAc



Walls:	08FAc1	08FAc2
<b>Manufactured:</b>	June 19, 1998 <sup>1</sup>	June 19, 1998 <sup>1</sup>
MOE data files:	8fac1p.prn 8fac1s.prn	8fac2p.prn 8fac2s.prn
MOE <sub>plates</sub> (10 <sup>6</sup> psi)	1.79	1.92
MOE <sub>studs</sub> (10 <sup>6</sup> psi)	1.52	1.60
Density <sub>plates</sub> (kg/m <sup>3</sup> )	547	582
Density <sub>studs</sub> (kg/m <sup>3</sup> )	475	489
<b>Date tested:</b>	August 3, 1998	August 3, 1998
Time tested:	13:22	16:23
LTC files:	alex_cyc	30alex8
Data files:	08FAc1.dat	08FAc2.dat
Excel files:	08FAc1_data 08FAc1_UTP	08FAc2_data 08FAc2_UTP
Photo files:	729-739	740-748

<sup>1</sup> Sheathing attached with ½+¼-in. edge distance along the top and bottom plates.

**Wall 08FAc1**

*Observations:* The initial elastic stiffness was significantly higher than the stabilized (approx. 27%), and the latter was just slightly lower than the elastic stiffness in the corresponding monotonic tests. The negative and positive envelopes were almost symmetrical. The average peak load 4.4 Kips (0.55 Kips/ft.) was developed at 1.8-in. amplitude. The wall yielded for several cycles and the resistance degraded 20% at 2.8-in. amplitude.

*Failure mode:* Sheathing unzipped along the top plate where the nails tore through the edge (Photo 379). Most of the nails along the right end stud and along the bottom plate failed in fatigue (Photo 739). The left end stud separated from the top plate and sheathing nails pulled out of wood. The wood density of this stud was  $420 \text{ kg/m}^3$  -- less than average.

**Wall 08FAc2**

*Observations:* This wall performed very similar to 08FAc1 wall. The average peak load 4.3 Kips (0.54 Kip/ft.) was reached at 1.8-in. amplitude. The wall yielded for several cycles and the resistance degraded 20% at 2.8-in. amplitude.

*Failure mode:* The failure mode was similar to that observed during 08FAc1 test. Sheathing unzipped along the top plate where the nails tore through the edge. It can be seen from the Photo 744 that the edge distance along the top plate was probably less than  $3/8$  in. The left end stud separated from the top plate and sheathing nails failed in fatigue. Most of the nails along the intermediate and the right end stud started pulling out of wood (Photo 745). Nails along the bottom plate failed in fatigue (Photos 742, 748).

**General**

Comparing the results of cyclic and monotonic tests (08FAc vs. 08FAM) the following conclusions were made:

- 1) Cyclic elastic stiffness (initial envelope) was 14% higher than the monotonic; stabilized stiffness was 17% less than monotonic.
- 2) Average cyclic strength (initial envelope) was 20% less than the monotonic strength; average stabilized strength was 30% less than the monotonic.

Most likely, the combined effect of the cyclic loading and the small edge distance of the sheathing-to-framing connections lead to the significant strength and ductility reduction. (See also comments to 12FAc series.) During the cycling, the sheathing nails start pulling out of wood or fail in fatigue provided the edge distance is adequate and the wood density is sufficient to hold the nails. Studs with low wood density allow the nail withdrawal easier.

Table 08FAc1. Data summary.

Specimen		08FAc1	Per unit length	
Tie-Down Anchors		cyclic test		
Wall length		8.00ft.	2.438m	
Date:	8-03-1998	Time:	13:22	
EEEP Parameters		units	initial	stabilized
Peak unit load, $v_{peak}$	Kip/ft.	0.550	0.479	
	KN/m	8.024	6.991	
Drift at peak load, $\Delta_{peak}$	in.	1.800	1.660	
	mm	45.73	42.16	
Yield unit load, $v_{yield}$	Kip/ft.	0.501	0.441	
	KN/m	7.317	6.440	
Drift at yield load, $\Delta_{yield}$	in.	0.372	0.448	
	mm	9.44	11.38	
Proportional limit, $0.4v_{peak}$	Kip/ft.	0.220	0.192	
	KN/m	3.210	2.796	
Drift at prop. limit, $\Delta@0.4v_{peak}$	in.	0.163	0.195	
	mm	4.14	4.94	
Unit load at failure or $0.8v_{peak}$	Kip/ft.	0.440	0.383	
	KN/m	6.420	5.593	
Drift at failure, $\Delta_{failure}$	in.	2.835	2.675	
	mm	72.01	67.94	
Shear modulus, G $@0.4v_{peak}$	Kip/in.	10.794	7.882	
	KN/mm	1.890	1.380	
Work until failure per unit length	Kip-ft./ft.	1.379	1.299	
	KN-m/m	6.134	5.779	
Unit load, $v_{1/300}$ $@ 0.32$ in. (8.13 mm)	Kips/ft.	0.321	0.304	
	KN/m	4.678	4.439	
Unit load, $v_{1/200}$ $@ 0.48$ in.(12.19 mm)	Kips/ft.	0.384	0.356	
	KN/m	5.600	5.196	
Unit load, $v_{1/100}$ $@ 0.96$ in. (24.38 mm)	Kips/ft.	0.504	0.445	
	KN/m	7.349	6.493	
Unit load, $v_{1/60}$ $@ 1.6$ in. (40.64 mm)	Kips/ft.	0.545	0.477	
	KN/m	7.951	6.957	
EVDR $@v_{peak}$			0.134	0.124

SEAOSC parameters		units	negative	positive	average
Yield Limit State	$v_{YLS}$	Kips/ft.	-0.315	0.265	0.290
		KN/m	-4.604	3.871	4.237
	$\Delta_{YLS}$	in.	-0.302	0.220	0.261
		mm	-7.67	5.58	6.62
Strength Limit State	$G'_{YLS}$	Kip/in.	8.363	9.659	8.909
		KN/mm	1.465	1.691	1.560
	$v_{SLS}$	Kips/ft.	-0.551	0.547	0.549
KN/m		-8.044	7.975	8.010	
$\Delta_{SLS}$	in.	-1.816	2.112	1.964	
	mm	-46.14	53.65	49.89	
$G'_{SLS}$	Kip/in.	2.428	2.070	2.235	
	KN/mm	0.425	0.362	0.391	

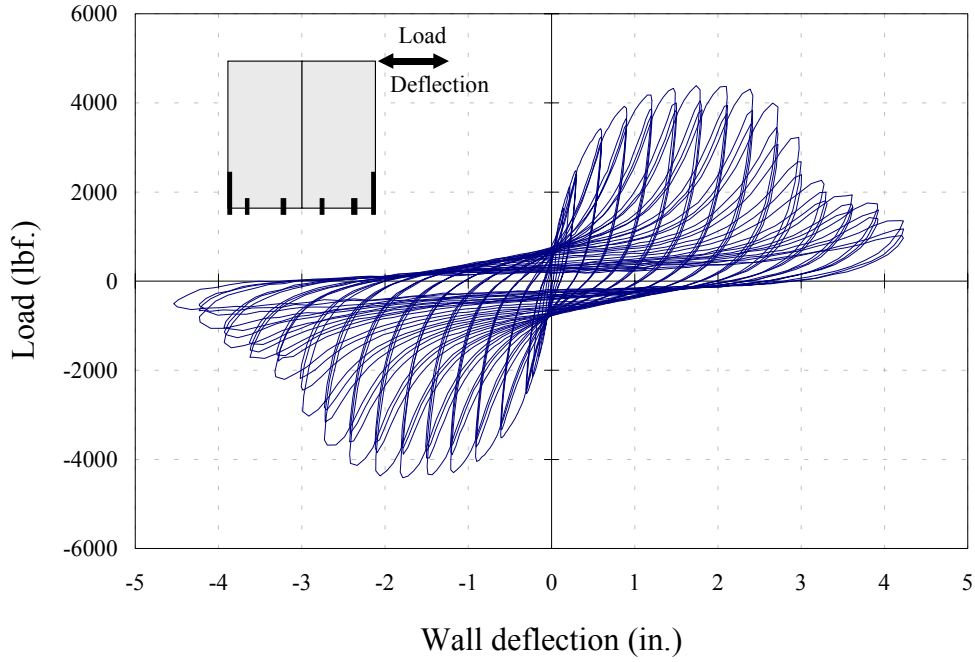


Figure 08FAc1- a. Observed load-deflection curve<sup>1</sup>.

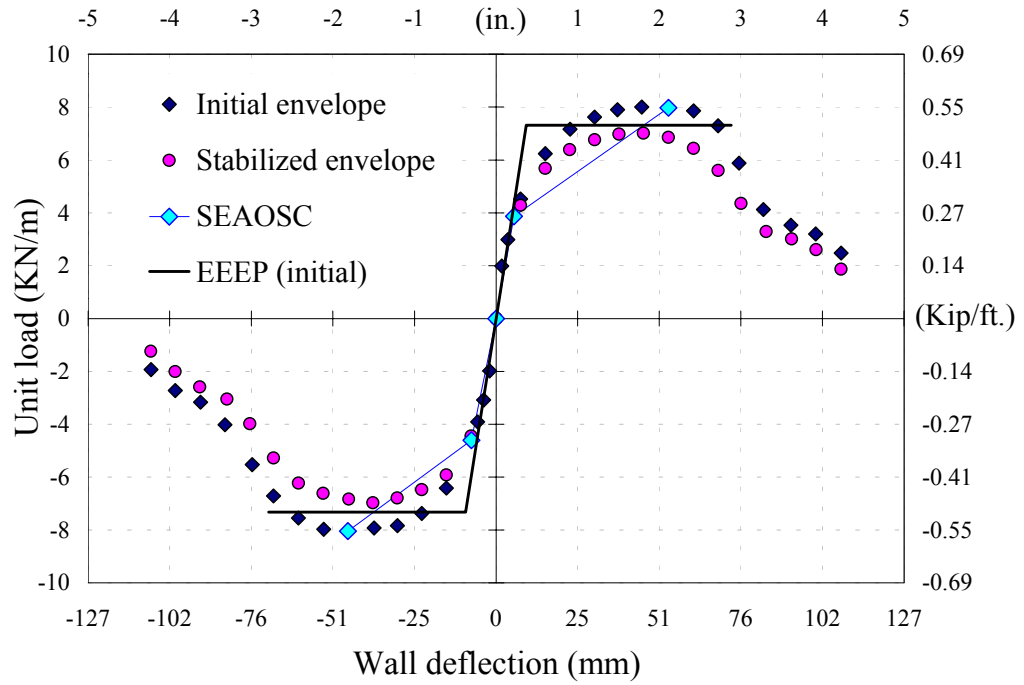


Figure 08FAc1- b. Envelopes, SEAOSC, and EEEP curves<sup>2</sup>.

<sup>1</sup> The scale of the graph varies between test series.

<sup>2</sup> The scale of the graph is uniform between test series for comparison purposes.

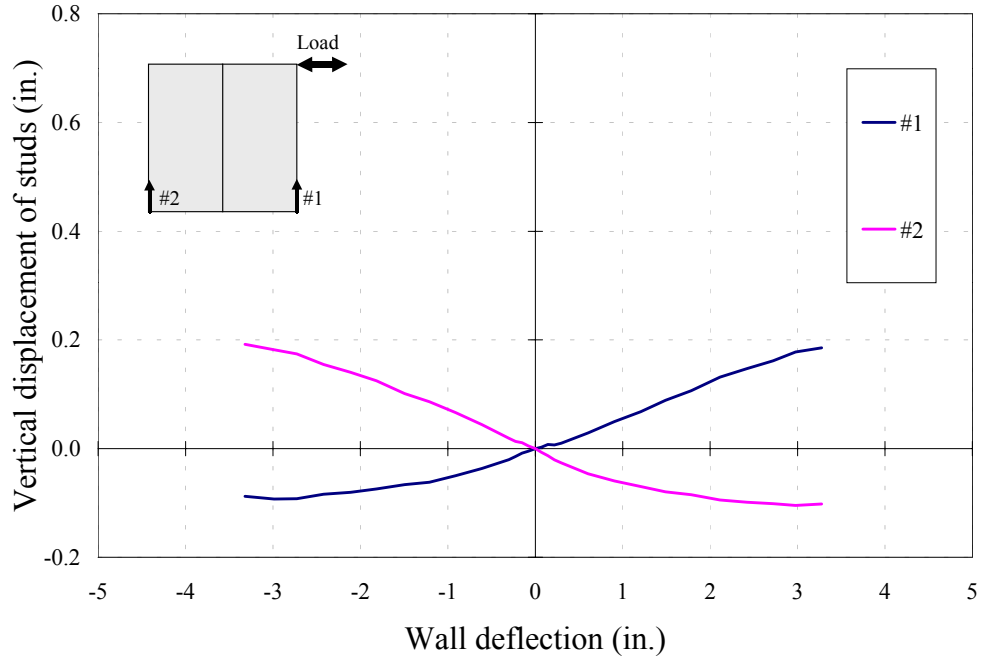


Figure 08FAc1- c. Vertical displacement of studs (initial envelope).

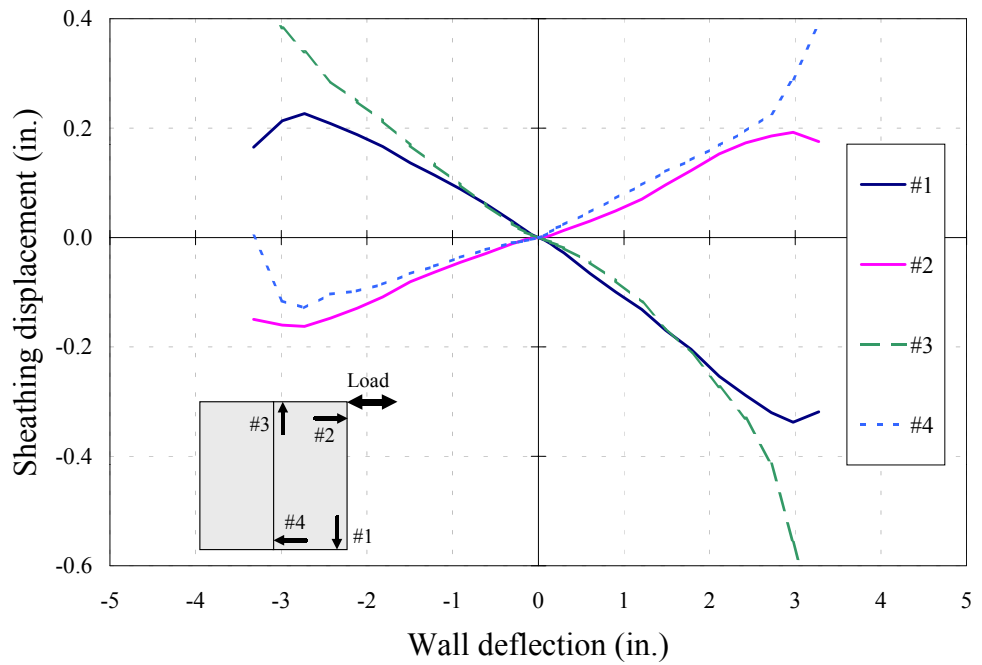


Figure 08FAc1- d. Sheathing displacement (initial envelope).

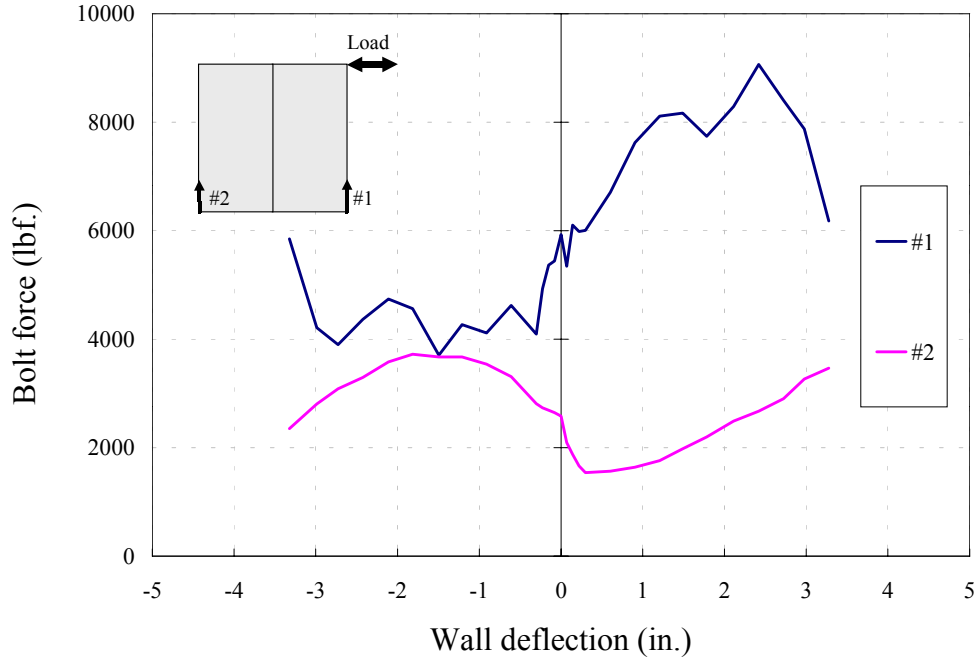


Figure 08FAc1- e. Forces in anchor bolts (initial envelope).

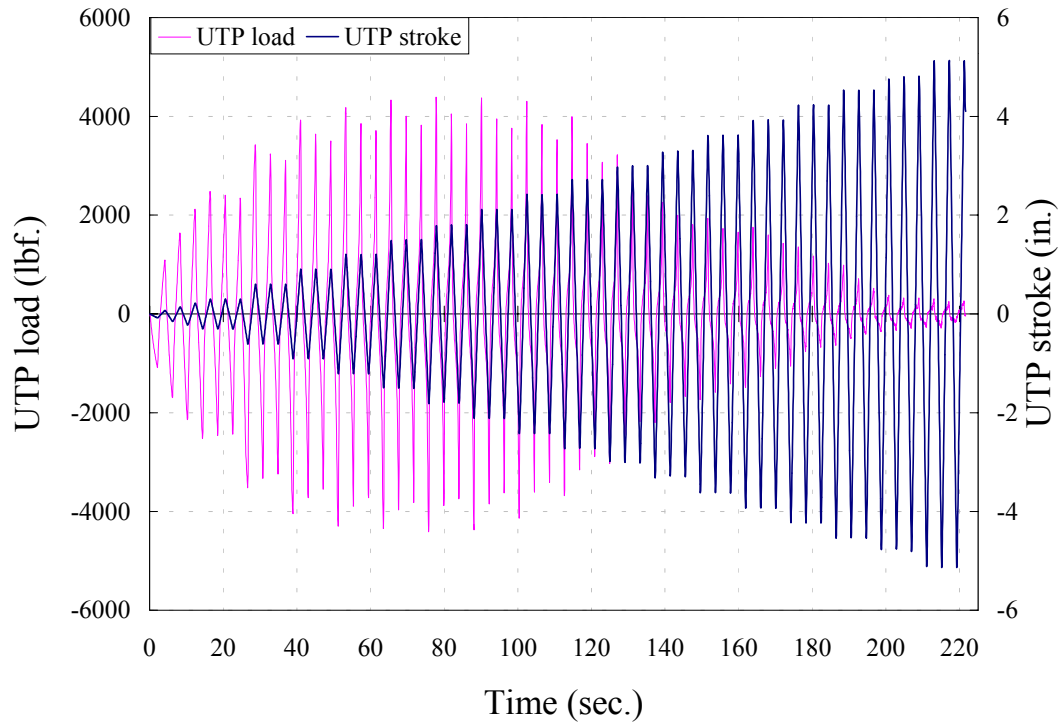


Figure 08FAc1- f. Load- and displacement-time record.

Table 08FAc2. Data summary.

Specimen		08FAc2	Per unit length	
Tie-Down Anchors		cyclic test		
Wall length		8.00ft.	2.438m	
Date:	8-03-1998	Time:	16:23	
EEEP Parameters		units	initial	stabilized
Peak unit load, $v_{peak}$	Kip/ft.	0.540	0.473	
	KN/m	7.888	6.903	
Drift at peak load, $\Delta_{peak}$	in.	1.802	1.507	
	mm	45.76	38.29	
Yield unit load, $v_{yield}$	Kip/ft.	0.489	0.432	
	KN/m	7.138	6.303	
Drift at yield load, $\Delta_{yield}$	in.	0.382	0.458	
	mm	9.69	11.64	
Proportional limit, $0.4v_{peak}$	Kip/ft.	0.216	0.189	
	KN/m	3.155	2.761	
Drift at prop. limit, $\Delta@0.4v_{peak}$	in.	0.169	0.201	
	mm	4.28	5.10	
Unit load at failure or $0.8v_{peak}$	Kip/ft.	0.432	0.378	
	KN/m	6.310	5.522	
Drift at failure, $\Delta_{failure}$	in.	2.865	2.694	
	mm	72.77	68.43	
Shear modulus, G $@0.4v_{peak}$	Kip/in.	10.357	7.539	
	KN/mm	1.814	1.320	
Work until failure per unit length	Kip-ft./ft.	1.402	1.318	
	KN-m/m	6.236	5.861	
Unit load, $v_{1/300}$ $@ 0.32$ in. (8.13 mm)	Kips/ft.	0.305	0.291	
	KN/m	4.446	4.249	
Unit load, $v_{1/200}$ $@ 0.48$ in.(12.19 mm)	Kips/ft.	0.368	0.343	
	KN/m	5.373	5.009	
Unit load, $v_{1/100}$ $@ 0.96$ in. (24.38 mm)	Kips/ft.	0.492	0.437	
	KN/m	7.182	6.381	
Unit load, $v_{1/60}$ $@ 1.6$ in. (40.64 mm)	Kips/ft.	0.536	0.470	
	KN/m	7.829	6.865	
EVDR $@v_{peak}$			0.139	0.126

SEAOSC parameters		units	negative	positive	average
Yield Limit State	$v_{YLS}$	Kips/ft.	-0.294	0.261	0.278
		KN/m	-4.292	3.811	4.051
	$\Delta_{YLS}$	in.	-0.305	0.223	0.264
		mm	-7.74	5.66	6.70
Strength Limit State	$G'_{YLS}$	Kip/in.	7.722	9.380	8.422
		KN/mm	1.352	1.643	1.475
	$v_{SLS}$	Kips/ft.	-0.551	0.530	0.540
KN/m		-8.044	7.731	7.888	
$\Delta_{SLS}$	in.	-1.816	1.787	1.802	
	mm	-46.14	45.39	45.76	
$G'_{SLS}$	Kip/in.	2.428	2.371	2.400	
	KN/mm	0.425	0.415	0.420	

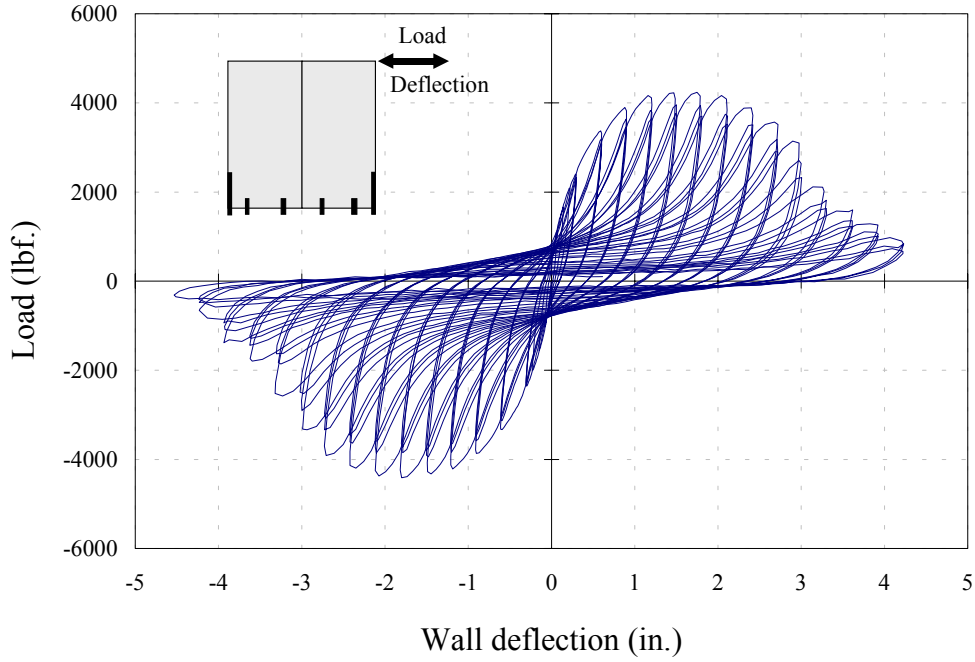


Figure 08FAc2- a. Observed load-deflection curve.

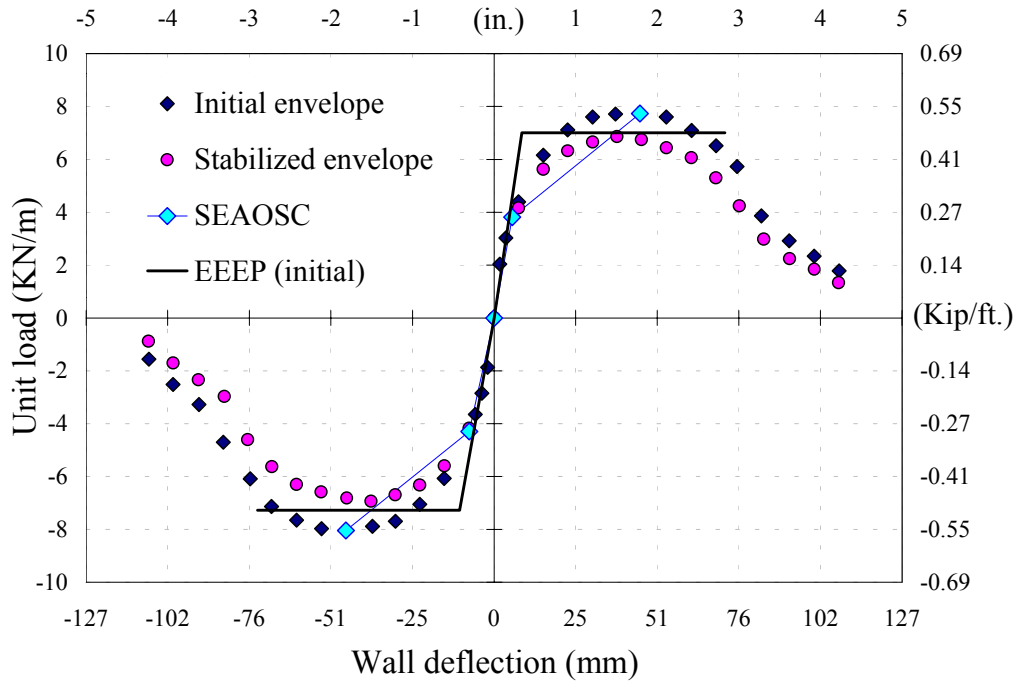


Figure 08FAc2- b. Envelopes, SEAOSC, and EEEP curves.



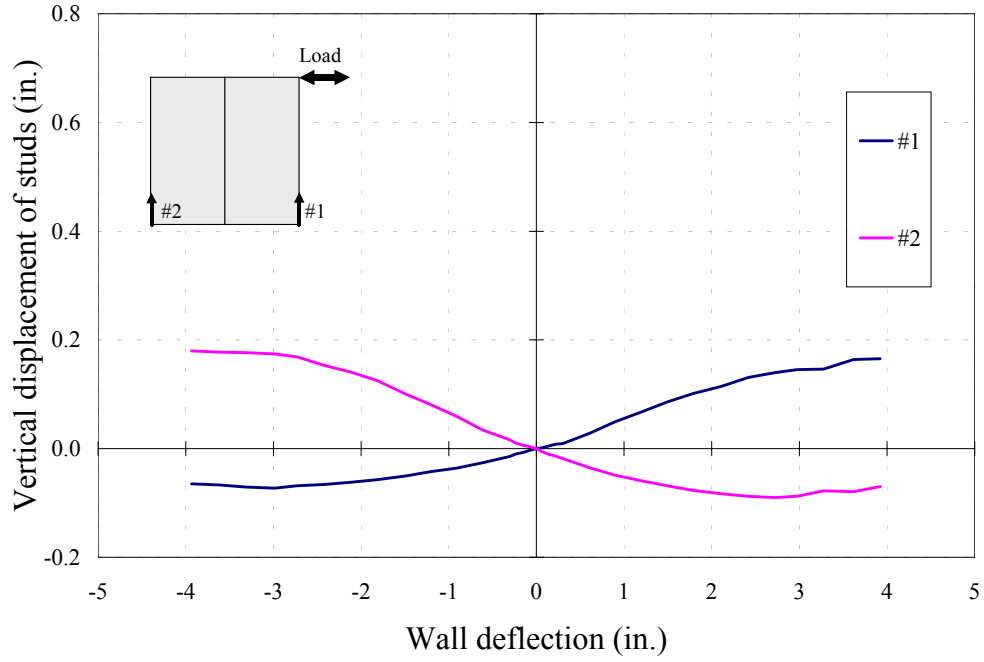


Figure 08FAc2- c. Vertical displacement of studs (initial envelope).

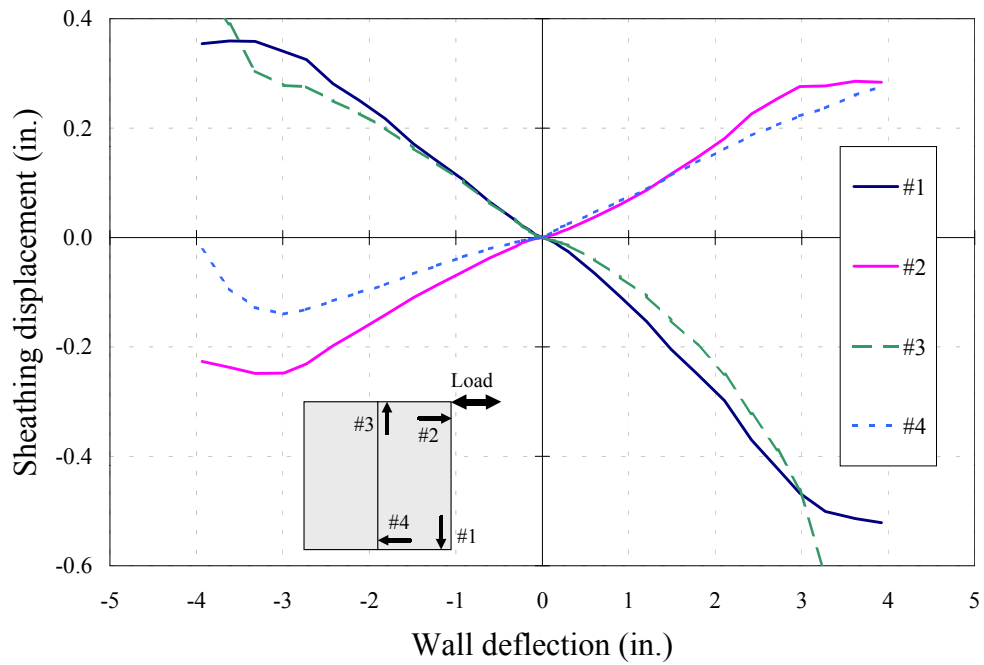


Figure 08FAc2- d. Sheathing displacement (initial envelope).

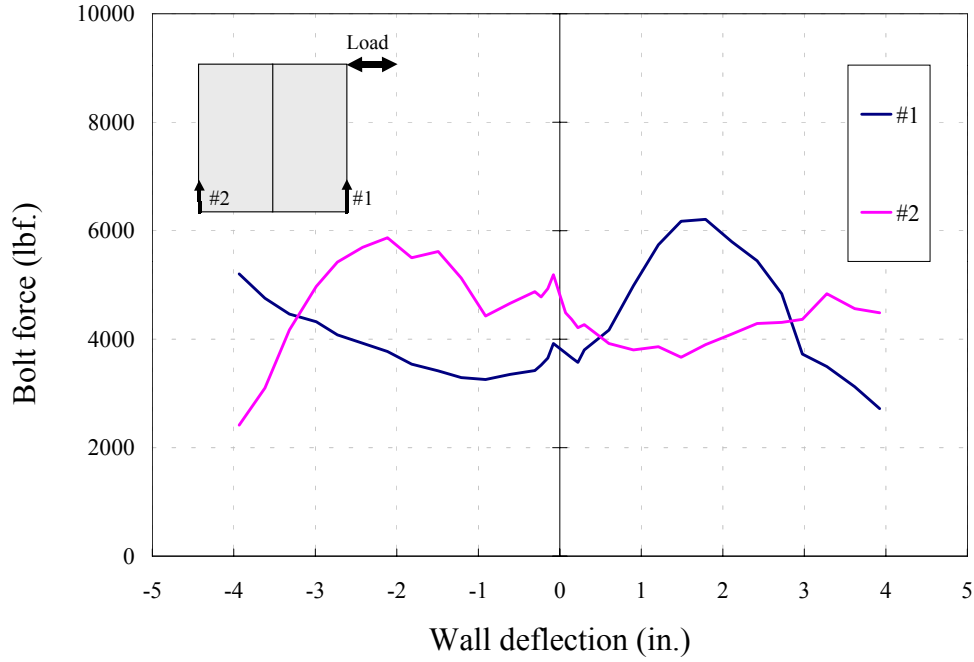


Figure 08FAc2- e. Forces in anchor bolts (initial envelope).

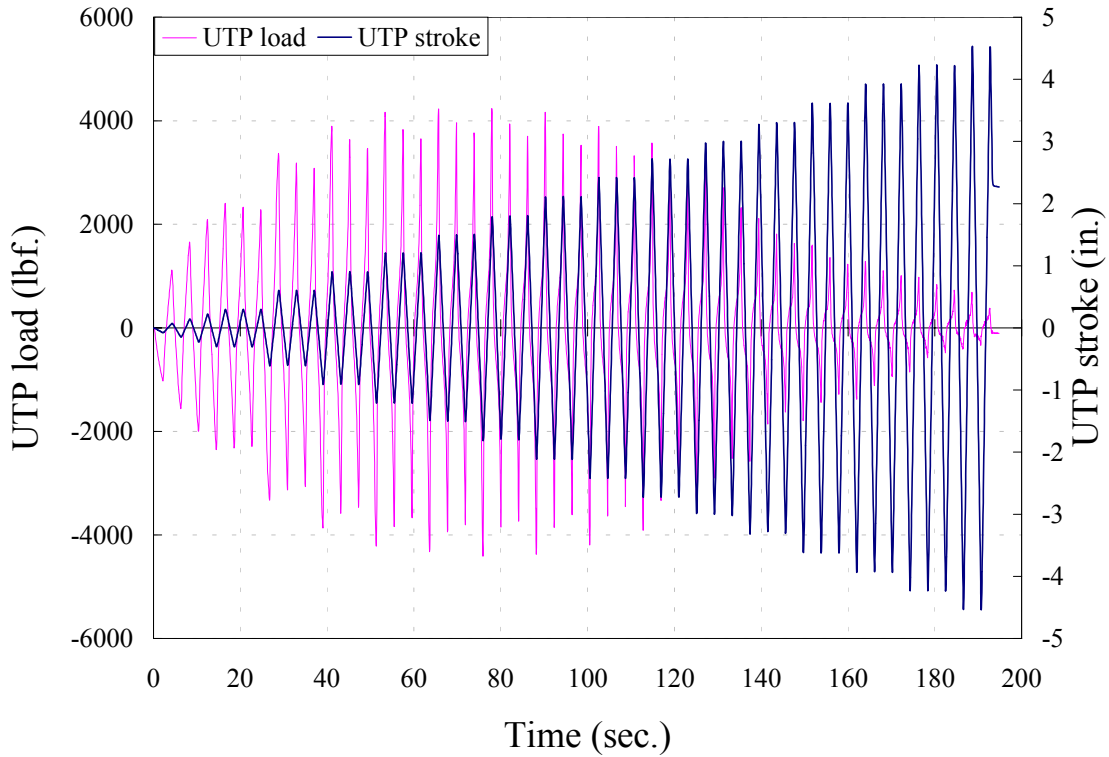


Figure 08FAc2- f. Load- and displacement-time record.