

**Fortran programs
for a 3-D four-noded curved beam element:
ARCHCODE version 1.0**

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Appendix to the thesis

**A geometrically exact curved beam theory
and
its finite element formulation/implementation**

submitted to the faculty of the
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ARCHCODE Version 1.0**

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1. Introduction

The ARCHCODE is a special purpose Fortran program using a 3-D four-noded curved beam element (and 1-DOF hinge model) based on a geometrically exact curved beam theory with rigid cross-section assumption.

The beam element has the following features:

- (i) can be used for 3-D natural curved/twisted slender and thick beams;
- (ii) has finite strains and finite displacements/rotations;
- (iii) can handle classical coupled cross-section material constants with an initial curvature correction term; and
- (iv) uses iso-parametric Lagrangian interpolation with uniform reduced integration.

However, the ARCHCODE Version 1.0 handles only slender beams and cross-section material constants without coupling.

The ARCHCODE Version 1.0 is used to simulate the static responses, such as, nodal displacements, buckling and post-buckling responses, as well as search for the lowest nonlinear and linearized bifurcation (branching) point.

The loads considered now are point loads and distributed loads (such as, self-weight, snow load, and pressure load, as given by load density per arc-length of the beam).

2. Lists of ARCHCODE version 1.0 Fortran source files and data files

Make sure that **all** files are put into the same subdirectory or folder.

The ARCHCODE Version 1.0 includes the following Fortran 77 source files:

analy0.f
analy1.f
analy2.f
analy3.f
analy4.f
analy5.f
analy6.f
eigenrgrgg.f

The ARCHCODE Version 1.0 has the following data files as opened in **analy0.f** as follows:

```
OPEN (UNIT=5,FILE='archcode.in', STATUS='OLD')  
OPEN (UNIT=6,FILE='archcode.out', STATUS='UNKNOWN')  
OPEN (UNIT=8,FILE='archu', STATUS='UNKNOWN ')  
OPEN (UNIT=9,FILE='archv', STATUS='UNKNOWN ')  
OPEN (UNIT=11,FILE='archw', STATUS='UNKNOWN ')  
OPEN (UNIT=13,FILE='archrx', STATUS='UNKNOWN ')  
OPEN (UNIT=14,FILE='archry', STATUS='UNKNOWN ')  
OPEN (UNIT=15,FILE='archrz', STATUS='UNKNOWN ')  
OPEN (UNIT=10,FILE='bifurcation.out', status = 'UNKNOWN ')  
OPEN (UNIT=12,FILE='archdet', status = 'UNKNOWN ')  
OPEN (UNIT=18,FILE='archshape', STATUS='UNKNOWN')
```

archcode.in:

The main one input data file.

archcode.out:

The main output data file.

archu, archv, archw, archrx, archry, archrz:

Output data files, storing the translation displacements in X, Y, Z directions, and rotation displacement (in rad) around X, Y, Z axes, respectively, for a specified node as given interactively when the ARCHCODE is run. Those data are given in TECPLOT format .

bifurcation.out:

Output data file, storing the information during the process of searching bifurcation point or the eigen-problem solution information when linearized bifurcation point is evaluated.

archdet:

Output data file, storing the relative determinants of the tangential stiffness matrix via loading factors during the process of searching bifurcation point. The data are given in TECPLOT format.

archshape:

Output data file, storing the nodal coordinates of the undeformed and deformed curved beam/arch/frame for each load step. The data are given in TECPLOT format.

3. How to compile and run the ARCHCODE version 1.0?

Here the instructions for compiling and running the ARCHCODE version 1.0 are given for unix workstation (IRIX 6.2).

To compile, type:

```
f77 -col120 analy0.f analy1.f analy2.f analy3.f analy4.f analy5.f analy6.f eigenrgrgg.f
```

then press ENTER or RETURN key.

Rename the execution file **a.out** to **archrun**:

```
mv a.out archrun
```

then press ENTER or RETURN key.

When the input data file **archcode.in** has been prepared, the ARCHCODE version 1.0 can be run by simply typing:

```
archrun
```

then press ENTER or RETURN key.

The ARCHCODE version 1.0 will ask you to enter some optional input information:

Choose the problem to be solved:

- 1 - for load control (with bifurcation analysis)**
- 2 - for arclength control (with bifurcation analysis)**
- 3 - for linearized bifurcation analysis**

For load control option, for example, type:

```
1
```

and then press ENTER or RETURN key. You will be asked to give further information:

Need to locate bifurcation point ??

if yes, enter 2; if no, enter 1:

Type

```
2
```

to locate bifurcation point if applicable, then press ENTER or RETURN key; or type

1

if you don't want to locate bifurcation point, then press ENTER or RETURN key.

And so on.

4. The format of the input file: archcode.in

Note that free field format is used in **archcode.in**. Data fields are separated by commas or blanks.

In the following tables, the first column is the comment, which does not appear in the file, the data fields start from the second column.

Data set 1: General information of the problem

1: Title

Format	Title
Example	Post-buckling analysis of a clamped-hinged circular arch

Title: any string of ascii codes; the total string length is less than 80

2: The scale of the finite element mesh

Format	NTNODE	NELM
Example	19	6

NTNODE: integer, the total number of nodes

NELM: integer, the total number of elements

3: The number of 1 DOF hinges

Format	NHINGE1
Example	0

NHINGE1: integer, the total number of 1 DOF hinges

Data set 2: Nodal coordinates

Format	X	Y	Z
Example	95.3717	-30.0706	0.0000

X, Y, Z: real, the X, Y, Z coordinates in the global coordinate system.

Each line consists of the coordinates of one node. The total number of record lines for the coordinates of the nodes is NNTNODE, in the order of the nodes numbered, from 1 to NNTNODE.

Data set 3: Element connection matrix: MELM

Format	Node 1	Node 2	Node 3	Node 4
Example	1	2	3	4

Node I: integer, the corresponding global ID number of I-th node in the beam element; I = 1, 2, 3, 4;

Each line is for one element. The total number of record lines for the element connection matrix is NELM, in the order of the elements numbered, from 1 to NELM.

Data set 4: 1 DOF hinge connection matrix: MHING1

Format	P_node	Q_node
Example	19	20

P_node: integer, the corresponding global ID number of Master node of the hinge;
Q_node: integer, the corresponding global ID number of Slave node of the hinge;
Each line is for one hinge. The total number of record lines for the hinge connection matrix is NHINGE1, in the order of the hinges numbered, from 1 to NHINGE1. If there is no 1 DOF hinge at all, i.e., NHINGE1 = 0, there will no need to fill in this data set.

Note that the 1 DOF hinge is that two beam members are connected at a spatial point, say, S, which is on the center line of the both beam members; the two beam members can only have a relative rotation around an axis, say H, through the spatial point S; the orientation of the axis H is fixed relative to the two beam members. When a finite element model is created, two nodes are created through that spatial point, and one node, say P, is fixed at one of the beam members, and the other, say Q, is fixed at the other beam member.

Data set 5: 1 DOF hinge direction matrix: HING1

Format	Hx	Hy	Hx	K
Example	0.	0.	1.	10.00

Hx, Hy, Hz: real, the X, Y, Z components of the direction vector H for the 1 DOF hinge.

K: real, the torsional spring constant of the 1 DOF hinge.

Each line is for one hinge. The total number of record lines for the hinge direction matrix is NHINGE1, in the order of the hinges numbered, from 1 to NHINGE1. If there is no 1 DOF hinge at all, i.e., NHINGE1 = 0, there will no need to fill in this data set.

Data set 6: Material constant matrices: CN, CM

The material constants of a 4-noded beam element at the first node:

Format	EA	GAn	GAb	GJt	EIn	EIb
Example	1.E8	1.E8	1.E8	1.E8	1.E7	1.E6

EA: real, the extensional stiffness associated with normal force

GAn: real, the shearing stiffness associated with shearing force along the normal direction

GAb: real, the shearing stiffness associated with shearing force along the bi-normal direction

GJt: real, the torsional stiffness associated with torque around the tangent direction

EIn: real, the bending stiffness associated with bending moment around the normal direction

EI_b: real, the bending stiffness associated with bending moment around the bi-normal direction

The same format for the second, third, and fourth node. Each line is for one node. Four lines are for four nodes in one element.

The above are repeated NELM times, consisting of 4*NELM record lines, in the order of the elements numbered.

Data set 7: Prescribed zero displacements

1: The total number of prescribed zero displacements:

Format	NPVBC
Example	62

NPVBC: integer, the total number of prescribed zero displacements

2: Information for prescribed zero displacements

Format	INOD	IDOF
Example	1	1

INOD, IDOF: integer, they indicate that the IDOF *-th* component of the INOD *-th* node has prescribed zero displacement.

- IDOF = 1, translation component in X direction
- 2, translation component in Y direction
- 3, translation component in Z direction
- 4, rotational component around X direction
- 5, rotational component around Y direction
- 6, rotational component around Z direction
- (Note: right-handed screw rule is used.)

Each line is for one zero displacement component. The total number of record lines for the zero displacement components is NPVBC, in any order that is convenient.

Data set 8: Prescribed non-zero point loads

1: The total number of prescribed non-zero point loads:

Format	NSVBC
Example	62

NSVBC: integer, the total number of prescribed non-zero point load components

2: The data of prescribed non-zero point loads

Format	INOD	IDOF	ALOD
Example	10	2	-1000.

INOD, IDOF: integer, they indicate that the IDOF *-th* component of the INOD *-th* node has prescribed non-zero point load

- IDOF = 1, translation component in X direction

- 2, translation component in Y direction
 - 3, translation component in Z direction
 - 4, rotational component around X direction
 - 5, rotational component around Y direction
 - 6, rotational component around Z direction
- (Note: right-handed screw rule is used.)

ALOD: real, point load value

- Note: No. of the component = 1: Px
 = 2: Py
 = 3: Pz
 = 4: Mx
 = 5: My
 = 6: Mz

Otherwise, wrong data for point loads!

Each line is for one non-zero point load component. The total number of record lines for the non-zero point load components is NSVBC, in any order that is convenient. If there is no non-zero point load at all, i.e., NSVBC = 0, there will no need to fill in this data set.

Data set 9: Non-zero weight density

1: The total number of non-zero weight elements

Format	NSVBCG
Example	6

NSVBCG: integer, the total number of nonzero weight elements.

2: The weight density of a 4-noded beam element

2.1 The ID number of the beam element

Format	IDELEM
Example	4

IDELEM: The ID number of the beam element

2.2 The weight density of a 4-noded beam element for the first node:

Format	X-comp	Y-comp	Z-comp
Example	0.	-100.	0.

The same format for the second, third, and fourth node. Each line is for one node. Four lines are for four nodes in one element.

2.1 and 2.2 are repeated NSVBCG times, consisting of 5*NSVBCG record lines, in any order that is convenient.

If there is no non-zero weight elements at all, i.e., $NSVBCG = 0$, there will no need to fill in this data set.

Data set 10: Non-zero direction fixed distributed load density

1: The total number of non-zero direction fixed distributed load elements

Format	NSVBCN
Example	6

NSVBCN: integer, the total number of nonzero direction fixed distributed load density elements

2: The load density of a 4-noded beam element

2.1 The ID number of the beam element

Format	IDELEM
Example	4

IDELEM: The ID number of the beam element

2.2 The load density of a 4-noded beam element for the first node:

Format	X-comp	Y-comp	Z-comp
Example	0.	-100.	0.

The same format for the second, third, and fourth node.

Four lines are for four nodes in one element.

2.1 and 2.2 are repeated NSVBCN times, consisting of $5*NSVBCN$ record lines, in any order that is convenient.

If there is no non-zero weight elements at all, i.e., $NSVBCN = 0$, there will no need to fill in this data set.

Data set 11: Non-zero pressure load density

1: The total number of nonzero pressure load elements

Format	NSVBCP
Example	6

NSVBCP: integer, the total number of nonzero pressure load density elements

2: The load density of a 4-noded beam element

2.1 The ID number of the beam element

Format	IDELEM
Example	4

IDELEM: The ID number of the beam element

2.2 The load density of a 4-noded beam element for the first node:

Format	X-comp	Y-comp	Z-comp
Example	0.	-100.	0.

The same format for the second, third, and fourth node.

Four lines are for four nodes in one element.

2.1 and 2.2 are repeated NSVBCP times, consisting of $5*NSVBCP$ record lines, in any order that is convenient.

If there is no non-zero weight elements at all, i.e., $NSVBCP = 0$, there will no need to fill in this data set.

5. Some sample archcode.in files for the examples in the thesis

Note that in all the examples for **archcode.in** file, the comment lines starting with **Data set** do not appear in the real **archcode.in** file.

Example 1 Unrolling and re-rolling of a circular cantilever beam

Data set 1: General information of the problem

Unrolling and re-rolling of a circular cantilever beam

13 4
0

Data set 2: Nodal coordinates

0.0000000000000000E+00	1.591549430918954	0.0000000000000000E+00
0.7957747154594766	1.378322238554480	0.0000000000000000E+00
1.378322238554480	0.7957747154594770	0.0000000000000000E+00
1.591549430918954	9.7454295812984397E-17	0.0000000000000000E+00
1.378322238554480	-0.7957747154594764	0.0000000000000000E+00
0.7957747154594773	-1.378322238554480	0.0000000000000000E+00
1.9490859162596879E-16	-1.591549430918954	0.0000000000000000E+00
-0.7957747154594764	-1.378322238554480	0.0000000000000000E+00
-1.378322238554480	-0.7957747154594774	0.0000000000000000E+00
-1.591549430918954	-2.9236288743895316E-16	0.0000000000000000E+00
-1.378322238554481	0.7957747154594756	0.0000000000000000E+00
-0.7957747154594774	1.378322238554480	0.0000000000000000E+00
-3.8981718325193759E-16	1.591549430918954	0.0000000000000000E+00

Data set 3: Element connection matrix: MELM

1	2	3	4
4	5	6	7
7	8	9	10
10	11	12	13

Data set 4: 1 DOF hinge connection matrix: MHING1

Data set 5: 1 DOF hinge direction matrix: HING1

Data set 6: Material constant matrices: CN, CM

1200.000000000000	500.000000000000	500.000000000000
200.000000000000	100.000000000000	100.000000000000
1200.000000000000	500.000000000000	500.000000000000
200.000000000000	100.000000000000	100.000000000000
1200.000000000000	500.000000000000	500.000000000000
200.000000000000	100.000000000000	100.000000000000
1200.000000000000	500.000000000000	500.000000000000
200.000000000000	100.000000000000	100.000000000000
1200.000000000000	500.000000000000	500.000000000000
200.000000000000	100.000000000000	100.000000000000
1200.000000000000	500.000000000000	500.000000000000
200.000000000000	100.000000000000	100.000000000000

1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000
1200.0000000000000	500.0000000000000	500.0000000000000
200.0000000000000	100.0000000000000	100.0000000000000

Data set 7: Prescribed zero displacements

6
1 1
1 2
1 3
1 4
1 5
1 6

Data set 8: Prescribed non-zero point loads

1
13 6 62.83185307179586

Data set 9: Non-zero weight density

0

Data set 10: Non-zero direction fixed distributed load density

0

Data set 11: Non-zero pressure load density

0

Example 2 Post-buckling analysis of a clamped-hinged circular arch

Data set 1: General information of the problem

Post-buckling analysis of a clamped-hinged circular arch

19 6
0

Data set 2: Nodal coordinates

95.37169507482270	-30.07057995042731	0.0000000000000000E+00
99.53027957931658	-9.681087070317911	0.0000000000000000E+00
99.37895171394428	11.12762131982996	0.0000000000000000E+00
94.92426435730339	31.45447561516136	0.0000000000000000E+00
86.35911671778987	50.41927170956703	0.0000000000000000E+00
74.05440131090047	67.20078605555223	0.0000000000000000E+00
58.54294337699407	81.07233671702122	0.0000000000000000E+00
40.49642820326738	91.43325053161793	0.0000000000000000E+00
20.69631545515058	97.83487377505476	0.0000000000000000E+00
2.8327694488239898E-14	100.00000000000000	0.0000000000000000E+00
-20.69631545515052	97.83487377505476	0.0000000000000000E+00
-40.49642820326733	91.43325053161794	0.0000000000000000E+00
-58.54294337699400	81.07233671702126	0.0000000000000000E+00
-74.05440131090042	67.20078605555229	0.0000000000000000E+00
-86.35911671778985	50.41927170956707	0.0000000000000000E+00
-94.92426435730337	31.45447561516141	0.0000000000000000E+00
-99.37895171394426	11.12762131983003	0.0000000000000000E+00
-99.53027957931660	-9.681087070317876	0.0000000000000000E+00
-95.37169507482271	-30.07057995042727	0.0000000000000000E+00

Data set 3: Element connection matrix: MELM

1	2	3	4
4	5	6	7
7	8	9	10
10	11	12	13
13	14	15	16
16	17	18	19

Data set 4: 1 DOF hinge connection matrix: MHING1

Data set 5: 1 DOF hinge direction matrix: HING1

Data set 6: Material constant matrices: CN, CM

10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000
10000000.0000000	10000000.0000000	10000000.0000000

19 1
19 2
19 3
19 4
19 5
19 6
2 3
2 4
2 5
3 3
3 4
3 5
4 3
4 4
4 5
5 3
5 4
5 5
6 3
6 4
6 5
7 3
7 4
7 5
8 3
8 4
8 5
9 3
9 4
9 5
10 3
10 4
10 5
11 3
11 4
11 5
12 3
12 4
12 5
13 3
13 4
13 5
14 3
14 4
14 5
15 3

15 4
15 5
16 3
16 4
16 5
17 3
17 4
17 5
18 3
18 4
18 5

Data set 8: Prescribed non-zero point loads

1
10 2 -1000.000000000000

Data set 9: Non-zero weight density

0

Data set 10: Non-zero direction fixed distributed load density

0

Data set 11: Non-zero pressure load density

0

Example 3 45-degree bend

Data set 1: General information of the problem

45 degree bend

4 1
0

Data set 2: Nodal coordinates

100.00000000000000	0.0000000000000000E+00	0.0000000000000000E+00
96.59258262890683	25.88190451025207	0.0000000000000000E+00
86.60254037844388	49.99999999999999	0.0000000000000000E+00
70.71067811865476	70.71067811865474	0.0000000000000000E+00

Data set 3: Element connection matrix: MELM

1 2 3 4

Data set 4: 1 DOF hinge connection matrix: MHING1

Data set 5: 1 DOF hinge direction matrix: HING1

Data set 6: Material constant matrices: CN, CM

1000000.00000000	416666.666666667	416666.666666667
703000.000000000	833333.333333333	833333.333333333
1000000.00000000	416666.666666667	416666.666666667
703000.000000000	833333.333333333	833333.333333333
1000000.00000000	416666.666666667	416666.666666667
703000.000000000	833333.333333333	833333.333333333
1000000.00000000	416666.666666667	416666.666666667
703000.000000000	833333.333333333	833333.333333333

Data set 7: Prescribed zero displacements

6
1 1
1 2
1 3
1 4
1 5
1 6

Data set 8: Prescribed non-zero point loads

1
4 3 600.0000000000000

Data set 9: Non-zero weight density

0

Data set 10: Non-zero direction fixed distributed load density

0

Data set 11: Non-zero pressure load density

0

Example 4 Cantilever beam subjected to conservative and non-conservative distributed loads (the data are for conservative load case)

Data set 1: General information of the problem

Cantilever beam subjected to conservative and non-conservative distributed loads

7 2
0

Data set 2: Nodal coordinates

0.0000000000000000E+00 0.0000000000000000E+00 0.0000000000000000E+00
 8.3333333333333329E-02 0.0000000000000000E+00 0.0000000000000000E+00
 0.1666666666666667 0.0000000000000000E+00 0.0000000000000000E+00
 0.2500000000000000 0.0000000000000000E+00 0.0000000000000000E+00
 0.3333333333333333 0.0000000000000000E+00 0.0000000000000000E+00
 0.4166666666666666 0.0000000000000000E+00 0.0000000000000000E+00
 0.0 0.0000000000000000E+00 0.0000000000000000E+00

Data set 3: Element connection matrix: MELM

1 2 3 4
4 5 6 7

Data set 4: 1 DOF hinge connection matrix: MHING1

Data set 5: 1 DOF hinge direction matrix: HING1

Data set 6: Material constant matrices: CN, CM

15000.000000000000	6249.999999999999	6249.999999999999
0.2500000000000000	0.1250000000000000	0.1250000000000000
15000.000000000000	6249.999999999999	6249.999999999999
0.2500000000000000	0.1250000000000000	0.1250000000000000
15000.000000000000	6249.999999999999	6249.999999999999
0.2500000000000000	0.1250000000000000	0.1250000000000000
15000.000000000000	6249.999999999999	6249.999999999999
0.2500000000000000	0.1250000000000000	0.1250000000000000
15000.000000000000	6249.999999999999	6249.999999999999
0.2500000000000000	0.1250000000000000	0.1250000000000000
15000.000000000000	6249.999999999999	6249.999999999999
0.2500000000000000	0.1250000000000000	0.1250000000000000
15000.000000000000	6249.999999999999	6249.999999999999
0.2500000000000000	0.1250000000000000	0.1250000000000000
15000.000000000000	6249.999999999999	6249.999999999999
0.2500000000000000	0.1250000000000000	0.1250000000000000

Data set 7: Prescribed zero displacements

6
1 1
1 2
1 3
1 4
1 5
1 6

Data set 8: Prescribed non-zero point loads

0

Data set 9: Non-zero weight density

0

Data set 10: Non-zero direction fixed distributed load density

2

1

0.0 1.0 0.0

0.0 1.0 0.0

0.0 1.0 0.0

0.0 1.0 0.0

2

0.0 1.0 0.0

0.0 1.0 0.0

0.0 1.0 0.0

0.0 1.0 0.0

Data set 11: Non-zero pressure load density

2

1

0.0 0.0 0.0

0.0 0.0 0.0

0.0 0.0 0.0

0.0 0.0 0.0

2

0.0 0.0 0.0

0.0 0.0 0.0

0.0 0.0 0.0

0.0 0.0 0.0

Example 5 Bifurcation buckling of a circular arch with two hinges (h/L = 0.2)

Data set 1: General information of the problem

Bifurcation buckling of a circular arch with two hinges at ends

15 4
2

Data set 2: Nodal coordinates

0.4999999999999999	0.5249999999999998	0.0000000000000000E+00
0.4999999999999999	0.5249999999999998	0.0000000000000000E+00
0.4295733585622031	0.5840305896214587	0.0000000000000000E+00
0.3522453295933466	0.6336783314740015	0.0000000000000000E+00
0.2692582403567252	0.6731456008918129	0.0000000000000000E+00
0.1819453347593019	0.7017983294076193	0.0000000000000000E+00
9.1709353883548561E-02	0.7191761914929206	0.0000000000000000E+00
4.4393446469091548E-17	0.7249999999999999	0.0000000000000000E+00
-9.1709353883548478E-02	0.7191761914929206	0.0000000000000000E+00
-0.1819453347593018	0.7017983294076193	0.0000000000000000E+00
-0.2692582403567250	0.6731456008918130	0.0000000000000000E+00
-0.3522453295933465	0.6336783314740015	0.0000000000000000E+00
-0.4295733585622031	0.5840305896214587	0.0000000000000000E+00
-0.4999999999999998	0.5249999999999999	0.0000000000000000E+00
-0.4999999999999998	0.5249999999999999	0.0000000000000000E+00

Data set 3: Element connection matrix: MELM

2	3	4	5
5	6	7	8
8	9	10	11
11	12	13	14

Data set 4: 1 DOF hinge connection matrix: MHING1

1	2
15	14

Data set 5: 1 DOF hinge direction matrix: HING1

0.	0.	1.	0.
0.	0.	1.	0.

Data set 6: Material constant matrices: CN, CM

120000.0000000000	49999.99999999999	49999.99999999999
2.000000000000000	25.00000000000000	1.000000000000000
120000.0000000000	49999.99999999999	49999.99999999999
2.000000000000000	25.00000000000000	1.000000000000000
120000.0000000000	49999.99999999999	49999.99999999999
2.000000000000000	25.00000000000000	1.000000000000000
120000.0000000000	49999.99999999999	49999.99999999999
2.000000000000000	25.00000000000000	1.000000000000000
120000.0000000000	49999.99999999999	49999.99999999999
2.000000000000000	25.00000000000000	1.000000000000000
120000.0000000000	49999.99999999999	49999.99999999999

2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999
2.0000000000000000	25.000000000000000	1.0000000000000000
120000.00000000000	49999.99999999999	49999.99999999999

Data set 7: Prescribed zero displacements

12
1 1
1 2
1 3
1 4
1 5
1 6
15 1
15 2
15 3
15 4
15 5
15 6

Data set 8: Prescribed non-zero point loads

0

Data set 9: Non-zero weight density

0

Data set 10: Non-zero direction fixed distributed load density

0

Data set 11: Non-zero pressure load density

4
1
0.0 1.0 0.0
0.0 1.0 0.0

0.0 1.0 0.0
0.0 1.0 0.0
2
0.0 1.0 0.0
0.0 1.0 0.0
0.0 1.0 0.0
0.0 1.0 0.0
3
0.0 1.0 0.0
0.0 1.0 0.0
0.0 1.0 0.0
0.0 1.0 0.0
4
0.0 1.0 0.0
0.0 1.0 0.0
0.0 1.0 0.0
0.0 1.0 0.0

6. ARCHCODE version 1.0 Fortran source files

```
C analy0.f:
C
c2345678901234567890123456789012345678901234567890123456789012345678901
2
      IMPLICIT REAL*8 (A-H,O-Z)
      PARAMETER (MXNOD=300,MXELM=100,MXEBC=1212,MXNEQ=10000)
      DIMENSION XYZ(3*MXNOD),XYZN(3*MXNOD),X0(12*MXELM)
      DIMENSION MELM(4*MXELM),NODBC(6*MXNOD)
      DIMENSION mhing1(100),mhing3(100)
      DIMENSION hing1(400),idofh1(100)
      DIMENSION LM(24),MAXH(MXNEQ+1),MHT(MXNEQ+1)
      DIMENSION PTLOAD(6*MXNOD),DISTGG(12*MXELM),DISTNN(12*MXELM)
      DIMENSION DISTPP(12*MXELM)
      DIMENSION AST(290000),DISP(6*MXNOD)
      DIMENSION CNCM(24*MXELM)
      DIMENSION xnode1(12*MXELM), ttrg(54*MXELM),
&          omegag(18*MXELM), da(6*MXNOD),dq(6*MXNOD),
&          qext0(6*MXNOD),qint(6*MXNOD),
&          da0(6*MXNOD),dq0(6*MXNOD),IPVT(6*MXNOD),WORK(6*MXNOD),
&          atotl(6*MXNOD),atotl0(6*MXNOD),aext0(6*MXNOD),aint(6*MXNOD)
      DIMENSION t6rg(54*MXELM),om6gag(18*MXELM)
      DIMENSION st6rg(54*MXELM),som6gg(54*MXELM)
      DIMENSION sxnode(12*MXELM),sttrg(54*MXELM),
&          somegg(18*MXELM),sda(6*MXNOD),sdq(6*MXNOD)

      DIMENSION WR(6*MXNOD),WI(6*MXNOD),Z(290000),LOC(6*MXNOD)
      DIMENSION FV1(6*MXNOD),IV1(6*MXNOD), AST1(290000),diag(6*MXNOD)

      write(*,*) ' Choose the problem to be solved: '
      write(*,*) '           '
      write(*,*) ' 1 - for load control (with bifurcation analysis)
      '
      write(*,*) ' 2 - for arclength control (with bifurcation
analysis)'
      write(*,*) ' 3 - for linearized bifurcation analysis'
      write(*,*) '           '
      read(*,*) isol
      if(isol.ne.3) then
        write(*,*) '           '
        write(*,*) ' Need to locate bifurcation point ??'
        write(*,*) ' if yes, enter 2; if no, enter 1:'
        write(*,*) '           '
        read(*,*) ip
        write(*,*) ' Enter incremental load factor : '
        read(*,*) alinc
        write(*,*) ' Enter the maximum load factor : '
        read(*,*) toltld
        write(*,*) ' Enter the node number '
        write(*,*) 'at which displacment will be printed:'
        read(*,*) nodepr
      end if
      if(isol.eq.3) then
        write(*,*) ' Choose the method to be used:'
```

```

        write(*,*) '          '
        write(*,*) ' 1 - for classical solution: neglect rotations '
        write(*,*) ' 2 - for complete linearized solution'
        write(*,*) '          '
        read(*,*) ip2
    end if
C
C      CN,CM = MATERIAL CONSTANTS
C
        OPEN (UNIT=5,FILE='archcode.in',STATUS='OLD')
        OPEN (UNIT=6,FILE='archcode.out',STATUS='UNKNOWN')
C      OPEN (UNIT=7,FILE='mohan.out',STATUS='UNKNOWN')
        OPEN (UNIT=8,FILE='archu',STATUS='unknown')
        OPEN (UNIT=9,FILE='archv',STATUS='unknown')
        OPEN (UNIT=11,FILE='archw',STATUS='unknown')
        OPEN (UNIT=13,FILE='archrx',STATUS='unknown')
        OPEN (UNIT=14,FILE='archry',STATUS='unknown')
        OPEN (UNIT=15,FILE='archrz',STATUS='unknown')
        open(10,file = 'bifurcation.out', status = 'unknown')
        open(12, file = 'archdet', status = 'unknown')
        OPEN (UNIT=16,FILE='arch1',STATUS='UNKNOWN')
        OPEN (UNIT=18,FILE='archshape',STATUS='unknown')
        OPEN (UNIT=19,FILE='archdisp',STATUS='unknown')
C
C      INPUT THE
C          TITLE:                UP TO 80 LETTERS AND NUMBERS
C          TOTAL NUMBER OF NODES:  NTNODE
C          TOTAL NUMBER OF ELEMENT: NELM
C
        CALL INPUT1(NTNODE,NELM,nhing1,nhing3)
C
C      INPUT ANALYSIS DATA AND PROCESS BOUNDARY CONDITIONS
C
        CALL INPUT2(NTNODE,NELM,nhing1,nhing3,
&                XYZ,X0,MELM,mhing1,mhing3,
&                hing1,
&                NODBC,PTLOAD,DISTGG,DISTNN,DISTPP,
&                CNCM,MAX)
C
        MAXEQN=MAX-1
        if(isol.ne.3) then
            write(8,*)'TITLE = "Displacent U at node = ',nodepr,'"'
            write(8,*)'VARIABLES = "U","load factor `l"'
            write(8,*)'ZONE T=" U ", I= '
            write(9,*)'TITLE = "Displacent V at node = ',nodepr,'"'
            write(9,*)'VARIABLES = "V","load factor `l"'
            write(9,*)'ZONE T=" V ", I= '
            write(11,*)'TITLE = "Displacent W at node = ',nodepr,'"'
            write(11,*)'VARIABLES = "W","load factor `l"'
            write(11,*)'ZONE T=" W ", I= '
            write(13,*)'TITLE = "Rotation around X at node = ',nodepr,'"'
            write(13,*)'VARIABLES = "R_x","load factor `l"'
            write(13,*)'ZONE T=" Rx ", I= '

```

```

        write(14,*)'TITLE = "Rotation around Y at node = ',nodepr,'"'
        write(14,*)'VARIABLES = "R_y","load factor `l"'
        write(14,*)'ZONE T=" Ry ", I= '
        write(15,*)'TITLE = "Rotation around Z at node = ',nodepr,'"'
        write(15,*)'VARIABLES = "R_z","load factor `l"'
        write(15,*)'ZONE T=" Rz ", I= '
        write(12,*)'TITLE = "Relative det(Kt) vs load factor"'
        write(12,*)'VARIABLES = "load factor `l","Relative det(Kt)"'
        write(12,*)'ZONE T="det ", I= '
    end if

    write(18,*)'TITLE = "Structural shapes on X-Y plane"'
    write(18,*)'VARIABLES = "X","Y","Z"'
C    write(18,*)'ZONE T="shape at load =',alambd,'" , I=',NTNODE
C
C
C    PREPARE TO DO ANALYSIS
C
        NWK = max*max
C
C    INITIALIZE xnode1,ttrg,omegag
C
        CALL initl(NELM,x0,xnode1,ttrg,omegag)

        CALL initl(NELM,x0,xnode1,t6rg,om6gag)

C*****GEOMETRICALLY NONLIN. STATIC LOAD*****
C    ISOL=1      LOAD CONTROL
C    ISOL=2      arclength control (RIKS-WEMPNER METHOD)
c    ISOL=3      linearized bifurcation analysis (and branching)
c234567890123456789012345678901234567890123456789012345678901234567890123456789012345678901
2
C
C    dq0 --- initial unbalanced load at current load level
C    da0 --- displ due to initial unbalanced load dq0
C    dq  --- unbalanced load at current iteration loop and current load
level
C    da  --- displ due to unbalanced load dq
C
C    NLD --- load level number
C    TLOD --- total load factor at NLD
C    ALINC --- incremental load factor
C
C
        maxnld = 2000      ! maximum number of load steps: nld<<maxnld
                          !                          nld<<maxnld

        maxldn = 10       ! maximum number of load reducing steps:
                          !                          ldown << maxldn

        maxitr = 70       ! maximum number of iterations inside each
load step:
                          !                          itr << maxitr

```

```

c      toltld = 50.          ! tolerance for maximum loading factor

      tolda= 0.00000001     ! tolerance for displacement increment

      toldq= 0.00000001     ! tolerance for load residual
      if(isol.le.2.and.ip.eq.2) then
        tolda= 0.00000001     ! tolerance for displacement increment
        toldq= 0.00000001     ! tolerance for load residual
      end if
      toldadq = 0.000005    ! tolerance for displacement
increment&load residual

      tolrdet = 0.000001    ! tolerance for relative determinant of
tangent
                                !      stiffness matrix to locate bifurcation
point

      cadd = 0.000001       ! scalar to add eigenvecotor to
displacment vector

      Npr=0
      NLD=0
      alamb0=0.0D0          ! total load factor
      alambd=0.0D0

      det0=1.
      rdet=1.

c      ISOL=3

C
C      INITIALIZE xnodel,ttrg,omegag
C

      CALL initl(NELM,x0,xnodel,ttrg,omegag)

      CALL initl(NELM,x0,xnodel,t6rg,om6gag)
      if(isol.ne.3) then
        CALL PRDISP(NTNODE,NODBC,atotl,nodepr,alambd)
        write(12,*) '0.00000          1.0000000'

      end if

      CALL CLVEC(dq0,MAX)
      CALL CLVEC(da0,MAX)
      CALL CLVEC(dq,MAX)
      CALL CLVEC(da,MAX)
      CALL CLVEC(atotl,MAX)
      CALL CLVEC(atotl0,MAX)
      call tempsv(maxeqn,nelm,
&                xnodel,ttrg,omegag,da,dq,
&                sxnode,sttrg,somegg,sda,sdq)

      call tempsv(maxeqn,nelm,
&                xnodel,t6rg,om6gag,da,dq,
&                sxnode,st6rg,som6gg,sda,sdq)

```

```

alamb0 = alambd

if(isol.eq.1) then
  alinc0 = alinc
  call LODSTP(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&             LM,MAXH,MHT,
&             X0,XYZN,xnode1,melm,NODBC,
&             nhing1,nhing3,
&             mhing1,mhing3,
&             hing1,idofh1,
&             cncm,
&             ptload,distgg,distnn,distpp,
&             qext0,qint,da,dq,da0,dq0,
&             sda,sdq,sxnode,
&             ttrg,t6rg,omegag,om6gag,
&             sttrg,st6rg,somegg,som6gg,
&             AST,AST1,diag,IPVT,WORK,
&             atotl,atotl0,aext0,aint,
&             wr,wi,z,loc,fv1,iv1,
&             maxnld,maxldn,maxitr,
&             NLD,lldown,ilast,
&             toltld,tolda,toldq,toldadq,
&             alamb0,alinc0,det0,rdet,nodepr,ip)

end if

if(isol.eq.2) then

  alinc0 = alinc
  call ARCSTP(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&             LM,MAXH,MHT,
&             X0,XYZN,xnode1,melm,NODBC,
&             nhing1,nhing3,
&             mhing1,mhing3,
&             hing1,idofh1,
&             cncm,
&             ptload,distgg,distnn,distpp,
&             qext0,qint,da,dq,da0,dq0,
&             sda,sdq,sxnode,
&             ttrg,t6rg,omegag,om6gag,
&             sttrg,st6rg,somegg,som6gg,
&             AST,AST1,diag,IPVT,WORK,
&             atotl,atotl0,aext0,aint,
&             wr,wi,z,loc,fv1,iv1,
&             maxnld,maxldn,maxitr,
&             NLD,lldown,ilast,
&             toltld,tolda,toldq,toldadq,cadd,
&             alamb0,alinc0,det0,rdet,nodepr,ip)

end if

if(isol.eq.3) then

  call BIFURL(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&             LM,MAXH,MHT,
&             X0,XYZN,xnode1,melm,NODBC,

```

```
&          nhing1,nhing3,  
&          mhing1,mhing3,  
&          hing1,idofh1,  
&          cncm,  
&          ptload,distgg,distnn,distpp,  
&          qext0,qint,da,dq,da0,dq0,  
&          sda,sdq,sxnode,  
&          ttrg,t6rg,omegag,om6gag,  
&          sttrg,st6rg,somegg,som6gg,  
&          AST,AST1,diag,IPVT,WORK,  
&          atotl,atotl0,  
&          wr,wi,z,loc,fv1,iv1,  
&          maxnld,maxldn,maxitr,  
&          NLD,lldown,niter,  
&          toltld,tolda,toldq,toldadq,  
&          alambd,alamb0,det0,rdet,ip2)
```

```
end if
```

```
STOP  
END
```

```

C  analy1.f:
C
      SUBROUTINE INPUT1(NTNODE,NELM,nhing1,nhing3)
      INTEGER TITLE(20)

      READ(5,10) TITLE
      WRITE(6,20) TITLE

      READ(5,*) NTNODE,NELM
      WRITE(6,30) NTNODE,NELM
      read(5,*) nhing1
      WRITE(6,*) 'The number of 1 dof hinges: nhing1=',nhing1
c      read(5,*) nhing3
c      WRITE(6,50) nhing3
10      FORMAT(20A4)
20      FORMAT(1X,20A4//)
30      FORMAT(1X,'THE TOTAL NUMBER OF      NODES: NTNODE = ',I3/
&          1X,'THE TOTAL NUMBER OF ELEMENTS:  NELM  = ',I3//)
35      FORMAT(1X,'THE TOTAL NUMBER OF FREE 1DOF HINGES: NHINGE =
',I3/)
50      FORMAT(1X,'THE TOTAL NUMBER OF FREE 3DOF HINGES: NHINGE =
',I3/)

      RETURN
      END

      SUBROUTINE INPUT2(NTNODE,NELM,nhing1,nhing3,
&          XYZ,X0,MELM,mhing1,mhing3,
&          hing1,
&          NODBC,PTLOAD,DISTGG,DISTNN,DISTPP,
&          CNCM,MAX)

      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION
XYZ(3,NTNODE),X0(3,4,NELM),MELM(NELM,4),NODBC(NTNODE,6)
      DIMENSION mhing1(2,*),mhing3(2,*)
      DIMENSION hing1(5,*)
      DIMENSION PTLOAD(*),DISTGG(3,4,NELM),DISTNN(3,4,NELM)
      DIMENSION DISTPP(3,4,NELM)
      DIMENSION CNCM(6,4,NELM)

C
C  NODAL COORDINATES: XYZ
C
      READ(5,*) ((XYZ(IJK,I),IJK=1,3),I=1,NTNODE)
      WRITE(6,*)'
      WRITE(6,*)'
      WRITE(6,*)'
      WRITE(6,*)'=====
      WRITE(6,*)' NO. OF NODE      X      Y      Z      '
      WRITE(6,*)'

      DO I=1,NTNODE
      WRITE(6,10) I,(XYZ(IJK,I),IJK=1,3)
10      FORMAT(5X,I3,6X,3D13.5)
      END DO

C
C  ELEMENT CONNECTION MATRIX: MELM
C

```



```

READ(5,*) ((MELM(IDELEM,I),I=1,4),IDELEM=1,NELM)
WRITE(6,*)'
WRITE(6,*)'
WRITE(6,*)'ELEMENT CONNECTION
WRITE(6,*)'=====
WRITE(6,*)'NO. OF ELEMENT   NODE 1   NODE 2   NODE 3   NODE 4'
WRITE(6,*)'
DO IDELEM=1,NELM
    WRITE(6,20) IDELEM,(MELM(IDELEM,I),I=1,4)
20    FORMAT(7X,I3,10X,I3,6X,I3,6X,I3,6X,I3)
END DO

C
C 1DOF HINGE CONNECTION MATRIX: MHING1
C

    if(nhing1.ne.0) then
        READ(5,*) ((MHING1(I,IHINGE),I=1,2),IHINGE=1,NHING1)
        WRITE(6,*)'
        WRITE(6,*)'
        WRITE(6,*)'1DOF HINGE CONNECTION
        WRITE(6,*)'=====
        WRITE(6,*)'NO. OF ELEMENT   P NODE   Q NODE'
        WRITE(6,*)'
        DO IHINGE=1,NHING1
            WRITE(6,21) IHINGE,(MHING1(I,IHINGE),I=1,2)
21            FORMAT(7X,I3,10X,I3,6X,I3)
        END DO

C
C 1DOF HINGE direction MATRIX: HING1
C

        read(5,*) ((HING1(I,IHINGE),I=1,4),IHINGE=1,NHING1)
        WRITE(6,*)'
        WRITE(6,*)'
        WRITE(6,*)'1DOF HINGE Direction
        WRITE(6,*)'=====
        WRITE(6,*)'NO. OF 1 dof hinge  x-dir   y-dir   z-dir   k
        WRITE(6,*)'
        DO IHINGE=1,NHING1
            WRITE(6,22) IHINGE,(HING1(I,IHINGE),I=1,4)
22            FORMAT(7X,I3,10X,E15.7,6X,E15.7,6X,E15.7,6X,E15.7)
        END DO
        DO IHINGE=1,NHING1
            HING1(5,IHINGE)=0.
        END DO

        end if

C
C 3DOF HINGE CONNECTION MATRIX: MHING3
C

c    READ(5,*) ((MHING3(I,IHINGE),I=1,2),IHINGE=1,NHING3)
c    WRITE(6,*)'
c    WRITE(6,*)'
c    WRITE(6,*)'3DOF HINGE CONNECTION
c    WRITE(6,*)'=====
c    WRITE(6,*)'NO. OF ELEMENTT  P NODE   Q NODE'
c    WRITE(6,*)'
c

```

```

c      DO IHINGE=1,NHING3
c          WRITE(6,25) IHINGE,(MHING3(I,IHINGE),I=1,2)
25      FORMAT(7X,I3,10X,I3,6X,I3)
c      END DO
c
c      TRANSFER XYZ TO X0
c
c
c      DO IDELEM = 1, NELM
c          DO NOD = 1,4
c
c              DO IDOF = 1,3
c                  X0(IDOF,NOD,IDELEM) = XYZ(IDOF,MELM(IDELEM,NOD))
c              END DO
c
c          END DO
c
c      END DO
c
c      MATERTIAL CONSTANT MATRICES: CN,CM
c
c      WRITE(6,*)'
c      WRITE(6,*)'                CROSS-SECTIONAL PROPERTIES
c      WRITE(6,*)'=====
c      WRITE(6,*)'
c      DO IDELEM = 1, NELM
c          WRITE(6,*) 'ELEMENT NO. ',IDELEM
c          WRITE(6,*)'
c          DO NOD = 1, 4
c              READ(5,*) (CNCM(IDOF,NOD,IDELEM), IDOF=1,6)
c              WRITE(6,*) 'AT NODE ',NOD
c              WRITE(6,*)'
c              WRITE(6,*)'          EAt          GAn          GAb
c              WRITE(6,*)'
c              WRITE(6,27) (CNCM(IDOF,NOD,IDELEM), IDOF=1,3)
c              WRITE(6,*)'
c              WRITE(6,*)'          GJt          EIn          E Ib
c              WRITE(6,*)'
c              WRITE(6,27) (CNCM(IDOF,NOD,IDELEM), IDOF=4,6)
c              WRITE(6,*)'
c          END DO
c      END DO
c
27      format(4x,3D13.5)
c
c      DISPLACEMENT B.C.
c
c      DO I=1,NTNODE
c          DO IDOF=1,6
c              NODBC(I,IDOF)=1
c          END DO

```

```

END DO
READ(5,*) NPVBC

WRITE(6,*) '
WRITE(6,*) 'THE TOTAL NUMBER OF PRESCRIBED ZERO DISPLACEMENT:'
WRITE(6,*) '===== '
WRITE(6,*) '
WRITE(6,30) NPVBC
30  FORMAT(1X,5X, 'NPVBC = ',I3)
DO I=1,NPVBC
    READ(5,*) INOD,IDOF
    NODBC(INOD,IDOF)=0
END DO
WRITE(6,*) '

CALL DISPBC(NTNODE,NODBC,MAX)
WRITE(6,*) '

C
C
C  LOADS
C
C          POINT LOADS: PTLOAD
C          LINE WEIGHT DENSIT: DISTGG
C          LINE DISTRIBUTED LOAD DENSITY: DISTNN
C
C
C
C
C
C  POINT LOAD
C

DO I=1,MAX-1
    PTLOAD(I)=0.d0
END DO

READ(5,*) NSVBC
WRITE(6,*) '
WRITE(6,*) 'THE TOTAL NUMBER OF PRESCRIBED NONZERO POINT
LOADS:'
WRITE(6,*)
'===== '
WRITE(6,*) '
WRITE(6,40) NSVBC
40  FORMAT(1X,5X, 'NSVBC = ',I3)
write(6,*) '
write(6,*) ' THE EFFECTIVE NONZERO POINT LOADS:
WRITE(6,*)
'===== '
WRITE(6,*) '
write(6,*) ' No. of Node      No. of Component      Value
if(NSVBC.ne.0) then
    DO I=1,NSVBC
        READ(5,*) INOD,IDOF,ALOD
        NLDOF=NODBC(INOD,IDOF)
        if(NLDOF.NE.0) THEN
            WRITE(6,50) INOD,IDOF,ALOD
            PTLOAD(NLDOF)=ALOD
        END IF

```

```

        END DO
    end if
50    FORMAT(5X,I3,16X,I3,6X,D15.6)
    WRITE(6,*) '
    WRITE(6,*) ' Note: No. of Component = 1: Px
    WRITE(6,*) ' = 2: Py
    WRITE(6,*) ' = 3: Pz
    WRITE(6,*) ' = 4: Mx
    WRITE(6,*) ' = 5: My
    WRITE(6,*) ' = 6: Mz
    WRITE(6,*) ' Otherwise, wrong data for point loads !'
    WRITE(6,*) '
C
C WEIGHT DENSITY
C

        DO IDELEM = 1, NELM
            DO NOD = 1,4
                DO IDOF = 1,3
                    DISTGG(IDOF,NOD,IDELEM) = 0.D0
                END DO
            END DO
        END DO
    READ(5,*) NSVBCG
    WRITE(6,*) '
    WRITE(6,*) 'THE TOTAL NUMBER OF PRESCRIBED NONZERO WT
ELEMENTS:'
    WRITE(6,*)
    '=====
    WRITE(6,*) '
    WRITE(6,45) NSVBCG

45    FORMAT(1X,5X, 'NSVBCG = ',I3)
    if(NSVBCG.ne.0) then
        write(6,*) '
        write(6,*) ' THE WEIGHT DENSITY:
        WRITE(6,*)
    '=====
    WRITE(6,*) '
    write(6,*) ' No. of EL-NODE    X-COMP    Y-COMP    Z-COMP '

        DO I=1,NSVBCG
            READ(5,*) IDELEM
            DO NOD = 1,4
                READ(5,*) (DISTGG(IDOF,NOD,IDELEM),IDOF = 1,3)
                WRITE(6,55) IDELEM,NOD,(DISTGG(IDOF,NOD,IDELEM),IDOF = 1,3)
            END DO
        END DO
55    FORMAT(3X,I4,3X,I4,3X,3D11.3)
    end if

C
C DISTRIBUTED LOAD DENSITY
C

```

```

DO IDELEM = 1, NELM
  DO NOD = 1,4
    DO IDOF = 1,3
      DISTNN(IDOF,NOD,IDELEM) = 0.D0
    END DO
  END DO
END DO
READ(5,*) NSVBCN
WRITE(6,*) '
WRITE(6,*) 'THE TOTAL NUMBER OF NONZERO DISTRIB LOAD ELEMENTS:'
WRITE(6,*)
'=====
WRITE(6,*) '
WRITE(6,60) NSVBCN
60  FORMAT(1X,5X, 'NSVBCN = ',I3)
write(6,*) '
write(6,*) ' THE LOAD DENSITY:
WRITE(6,*)
'=====
WRITE(6,*) '
write(6,*) ' No. of EL-NODE      X-COMP      Y-COMP      Z-COMP '

if(NSVBCN.ne.0) then
  DO I=1,NSVBCN
    DO IDELEM
      DO NOD = 1,4
        READ(5,*) (DISTNN(IDOF,NOD,IDELEM),IDOF = 1,3)
        WRITE(6,55) IDELEM,NOD,(DISTNN(IDOF,NOD,IDELEM),IDOF = 1,3)
      END DO
    END DO
  end if

c
C  Hydrostatic load DENSITY
C
DO IDELEM = 1, NELM
  DO NOD = 1,4
    DO IDOF = 1,3
      DISTPP(IDOF,NOD,IDELEM) = 0.D0
    END DO
  END DO
END DO
READ(5,*) NSVBCP
WRITE(6,*) '
WRITE(6,*) 'THE TOTAL NUMBER OF NONZERO HYDRO LOAD ELEMENTS:'
WRITE(6,*)
'=====
WRITE(6,*) '
WRITE(6,70) NSVBCP
70  FORMAT(1X,5X, 'NSVBCP = ',I3)
write(6,*) '
write(6,*) ' THE LOAD DENSITY:
WRITE(6,*)
'=====
WRITE(6,*) '
write(6,*) ' No. of EL-NODE      t01-COMP      t02-COMP      t03-COMP '

```

```

if(NSVBCP.ne.0) then
  DO I=1,NSVBCP
    READ(5,*) IDELEM
    DO NOD = 1,4
      READ(5,*) (DISTPP(IDOF,NOD,IDELEM),IDOF = 1,3)
      WRITE(6,55) IDELEM,NOD,(DISTPP(IDOF,NOD,IDELEM),IDOF = 1,3)
    END DO
  END DO
end if

```

```

RETURN
END

```

C
C
C
C
C

```

SUBROUTINE DISPBC(NTNODE,NODBC,MAX)
DIMENSION NODBC(NTNODE,6)

```

```

WRITE(6,10)

```

```

NDOF=0
DO II=1, NTNODE
  DO JJ=1,6
    IF(NODBC(II,JJ).NE.0) THEN
      NDOF=NDOF+1
      NODBC(II,JJ)=NDOF
    END IF
  END DO
  WRITE(6,20) II,(NODBC(II,JJ),JJ=1,6)
END DO

```

```

MAX=NDOF+1

```

```

10  FORMAT(/1x,'Node No.',21x,'The Sequential D.O.F.'//
&      14x,'Ux',3x,'Uy',3x,'Uz',3x,
&      'Rx',3x,'Ry',3x,'Rz'/)
20  FORMAT(3x,i5,3x,6i5)

```

```

RETURN
END

```

C
C

```

SUBROUTINE NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION MELM(NELM,4),XNODEL(3,4,NELM),XYZN(3,NTNODE)

```

```

DO NOD=1,NTNODE
  DO IDOF=1,3
    XYZN(IDOF,NOD)=0.D0

```

```

      END DO
    END DO

    DO IDELEM = 1, NELM
      DO NOD = 1, 4

        DO IDOF = 1, 3
          XYZN( IDOF, MELM( IDELEM, NOD ) ) = XNODEL( IDOF, NOD, IDELEM )
        END DO

      END DO

    END DO

  END DO

write(18,*) 'ZONE T="shape at load =', alambd, '" , I=', NTNODE
DO NOD=1,NTNODE
  WRITE(18,*) (XYZN(IDOF,NOD),IDOF=1,3)
END DO

RETURN
END

```

C

```

SUBROUTINE PRDISP(NTNODE,NODBC,atotl,node,alambd)
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION NODBC(ntnode,6),atotl(*),atotlw(6,500)

DO NOD=1,NTNODE
  DO IDOF=1,6
    atotlw(IDOF,NOD)=0.D0
  END DO
END DO

DO NOD=1,NTNODE

  DO IDOF=1,6
    ndof=NODBC(NOD,IDOF)
    if(ndof.ne.0) then
      atotlw(IDOF,NOD)=atotl(ndof)
    end if
  end do

END DO

write(19,*) '      '
write(19,*) ' Nodal Displacement at load factor = ',alambd
write(19,*) '-----'
write(19,*) ' U          V          W          Rx          Ry          Rz '
write(19,*) '      '
do nod = 1,NTNODE
write(19,*) (atotlw(j,NOD), j=1,6)
end do

write(8,*) atotlw(1,NODE),alambd

write(9,*) atotlw(2,NODE),alambd

```

```

write(11,*) atotlw(3,NODE),alambd

write(13,*) atotlw(4,NODE),alambd

write(14,*) atotlw(5,NODE),alambd

write(15,*) atotlw(6,NODE),alambd
RETURN
END

SUBROUTINE RESIDL(qext0,qint,maxeqn,alambd,dq)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION qext0(*),qint(*),dq(*)

do ii=1,maxeqn
  dq(ii) = alambd*qext0(ii) - qint(ii)
end do

return
END
SUBROUTINE INCREA(aext0,aint,maxeqn,alambd,da)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION aext0(*),aint(*),da(*)

do ii=1,maxeqn
  da(ii) = alambd*aext0(ii) - aint(ii)
end do

return
END

```



```

C analy2.f:
C
CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC
C
C
C
C      This subroutine presents cubic lagrangian basis function
values
C
C      and their derivatives (wrt t) for a given value of the parameter t
C
C
CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC
C
      subroutine Lagran(t,ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4,
&      ddla1,ddla2,ddla3,ddla4,dddla1,dddla2,dddla3,dddla4,idv)
      implicit real*8 (a-h,o-z)

      if(idv.eq.0) then
c Compute the Lagrangian basis functions

      ala1 = (2.d0 - 11.d0*t + 18.d0*t**2 - 9.d0*t**3)/2.d0
      ala2 = 9.d0*t*(2.d0 - 5.d0*t + 3.d0*t**2)/2.d0
      ala3 = 9.d0*t*(-1.d0 + 4.d0*t - 3.d0*t**2)/2.d0
      ala4 = t*(2.d0 - 9.d0*t + 9.d0*t**2)/2.d0
      end if

      if(idv.eq.1) then
c Compute the 1st order derivatives of the Lagrangian basis functions

      dla1 = (-11.d0 + 36.d0*t - 27.d0*t**2)/2.d0
      dla2 = 9.d0 - 45.d0*t + 81.d0*t**2/2.d0
      dla3 = 9.d0*(-1.d0 + 8.d0*t - 9.d0*t**2)/2.d0
      dla4 = 1.d0 - 9.d0*t + 27.d0*t**2/2.d0

      end if

      if(idv.eq.2) then
c Compute the 2nd order derivatives of the Lagrangian basis functions

      ddla1 = 18.d0 - 27.d0*t
      ddla2 = -45.d0 + 81.d0*t
      ddla3 = 36.d0 - 81.d0*t
      ddla4 = -9.d0 + 27.d0*t

      end if

      if(idv.eq.3) then
c Compute the 3rd order derivatives of the Lagrangian basis functions

      dddla1 = - 27.d0
      dddla2 = 81.d0
      dddla3 = - 81.d0
      dddla4 = 27.d0

      end if

```

```

        return
        end

c
c
c234567890123456789012345678901234567890123456789012345678901234567890123456789012345678901
c
c
c
c
c      This subroutine is used to find the
c
c      unit tangential vector 't': tt0(ii,1)/
c      its first order derivative: dltt0(ii,1)
c
c      unit normal vector 'n': tt0(ii,2)/
c      its first order derivative: dltt0(ii,2) and
c
c      unit binormal vector 'b': tt0(ii,3)/
c      its first order derivative: dltt0(ii,3)
c
c
c of the curve piece 'i' at any point given by parameter t.
c
c The curve piece is in the form of cubic lagrangian interpolation
c function,
c
c with x0(ii,jj,idelem ) as jj th (1 <= jj <=3) component of the node
ii
c (1 <= ii <= 4) for the element idelem.
c
c The tangential vector 't', normal vector 'n', and binormal vector
c 'b' will be stored in the first, second, and third column of the 3
c by 3 matrix tt0(i,j), respectively.
c
c
c Besides, the determinant of Jacobian detj is an output.
c
c
c Memo:
c
c Note that the procedure may fail if the curve piece is straight or
c the curvature at this point is zero. If we make an assumption that
c the Frenet frame coincides with the principal axes of the curved
beam,
c then the problem is treatable. But, then, is that easy? It is easy
if
c the arch or frame is planar one. What about the spacial arch?
c
c Planar arch or frame: the binormal vector is known beforehand
except
c for the sign. If the curvature does not vanish, then the normal
vector
c 'n' can be determined. If the curvature vanishes, take any unit
vector

```

```

c  which is normal to the arch plane as binormal vector 'b'. Then the
c  normal vector 'n' can be determined without difficulty.
c
c  Spacial arch or frame: borrow that ideas for planar arch or frame,
c  we specify a binormal vector 'b' at the first knot on each B-spline
c  curve (which is composed of several Bezier curves). If the curvature
c  at this point is zero, then accept the prescribed binormal vector.
c  Otherwise, both the normal and binormal vectors can be calculated
c  from
c  the known curve. Therefore, the Frenet frame can be determined at
c  the
c  first point on the curve in any case. If the Frenet frame cannot be
c  determined from the curve itself at some point after the first
c  point,
c  then take the last Frenet frame at the last point as the current
c  frame.
c  This is reasonable as at the point of zero curvature, the derivative
c  of the Frenet frame and the torque are zero with continuous
c  conditions
c  for the frame.
c
c
c
c2345678901234567890123456789012345678901234567890123456789012345678901
2
c
      subroutine Frenet(idelem,t,x0,tt0,d1tt0,detj,
&                    ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4)

      implicit real*8 (a-h,o-z)
      dimension x0(3,4,*), tt0(3,3), d1tt0(3,3),d2tt0(3,3)
      dimension xyz(3),d1xyz(3),d2xyz(3),d3xyz(3)

      tol=1.d-20          ! Tolerance for curvature checking.
c
c  Start to find the 0 to 2nd derivatives of the position vector
c  xyz(ii) at t on the curve piece idelem.
c
c  with x0(jj,ii,idelem ) as jj th (1 <= jj <=3) component of the node
ii
c  (1 <= ii <= 4) for the element idelem.

      do idv=0,3
          call Lagran(t,ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4,
&                  ddla1,ddla2,ddla3,ddla4,dddla1,dddla2,dddla3,dddla4,idv)
      end do

      do jj=1,3
          xyz(jj)=ala1*x0(jj,1,idelem)+ala2*x0(jj,2,idelem)
&              +ala3*x0(jj,3,idelem)+ala4*x0(jj,4,idelem)
      end do

      do jj=1,3
          d1xyz(jj)=dla1*x0(jj,1,idelem)+dla2*x0(jj,2,idelem)
&                +dla3*x0(jj,3,idelem)+dla4*x0(jj,4,idelem)
      end do

```

```

do jj=1,3
  d2xyz(jj)=ddla1*x0(jj,1,idelem)+ddla2*x0(jj,2,idelem)
&          +ddla3*x0(jj,3,idelem)+ddla4*x0(jj,4,idelem)
end do

do jj=1,3
  d3xyz(jj)=dddla1*x0(jj,1,idelem)+dddla2*x0(jj,2,idelem)
&          +dddla3*x0(jj,3,idelem)+dddla4*x0(jj,4,idelem)
end do

detj = dsqrt(dlxyz(1)*dlxyz(1)
&          +dlxyz(2)*dlxyz(2)
&          +dlxyz(3)*dlxyz(3))

do jj=1,3
  dlxyz(jj)= dlxyz(jj)/detj
end do

do jj=1,3
  d2xyz(jj)= d2xyz(jj)/(detj*detj)
end do

do jj=1,3
  d3xyz(jj)= d3xyz(jj)/(detj*detj*detj)
end do

c
c The end of finding the 0 to 2nd derivatives of the position
c vector xyz(jj) at t on the curve piece idelem.
c

do jj = 1,3
  tt0(jj,1) = dlxyz(jj)
end do

do jj = 1,3
  dltt0(jj,1) = d2xyz(jj)
end do

do jj = 1,3
  d2tt0(jj,1) = d3xyz(jj)
end do

C
C icurve = 1 curved beam, all three frame axes are calculated.
c the curved beam can be 3D curved, not limited to
C on X-Y plane
C
C icurve = 2 straight beam, 'b' is given as parallel to Z axis
C and assume that the beam is on X-Y plane
C

icurve = 2

if(icurve.eq.1) then ! curved beam on X-Y plane
c
c Start to find the unit binormal vector 'n'.
c

```

```

      adt01 = dsqrt(dlitt0(1,1)*dlitt0(1,1)
&              +dlitt0(2,1)*dlitt0(2,1)
&              +dlitt0(3,1)*dlitt0(3,1))

      dadt01 = (dlitt0(1,1)*d2tt0(1,1)
&             + dlitt0(2,1)*d2tt0(2,1)
&             + dlitt0(3,1)*d2tt0(3,1))/adt01

      do jj = 1,3
        tt0(jj,2) = dlitt0(jj,1)/adt01
      end do
c
c      End to find the unit binormal vector 'n'.
c
c      Start to find the unit binormal vector 'b'.
c
      tt0(1,3) = tt0(2,1)*tt0(3,2)-tt0(3,1)*tt0(2,2)
      tt0(2,3) = tt0(3,1)*tt0(1,2)-tt0(1,1)*tt0(3,2)
      tt0(3,3) = tt0(1,1)*tt0(2,2)-tt0(2,1)*tt0(1,2)

c
c      The end of finding the unit binormal vector 'b'.
c
      dlitt0(1,3) = dlitt0(2,1)*tt0(3,2)+tt0(2,1)*dlitt0(3,2)
&              -dlitt0(3,1)*tt0(2,2)-tt0(3,1)*dlitt0(2,2)
      dlitt0(2,3) = dlitt0(3,1)*tt0(1,2)+tt0(3,1)*dlitt0(1,2)
&              -dlitt0(1,1)*tt0(3,2)-tt0(1,1)*dlitt0(3,2)
      dlitt0(3,3) = dlitt0(1,1)*tt0(2,2)+tt0(1,1)*dlitt0(2,2)
&              -dlitt0(2,1)*tt0(1,2)-tt0(2,1)*dlitt0(1,2)

      end if

      if(icurve.eq.2) then      ! straight beam on X-Y plane
c
c      Start to find the unit binormal vector 'b'.
c

      tt0(1,3) = 0.d0
      tt0(2,3) = 0.d0
      tt0(3,3) = -1.d0

c
c      Start to find the unit binormal vector 'n'.
c
      tt0(1,2) = tt0(2,3)*tt0(3,1)-tt0(3,3)*tt0(2,1)
      tt0(2,2) = tt0(3,3)*tt0(1,1)-tt0(1,3)*tt0(3,1)
      tt0(3,2) = tt0(1,3)*tt0(2,1)-tt0(2,3)*tt0(1,1)

      end if

      return
      end

```



```

do jj=1,3
  dlxyz(jj)=dla1*x0(jj,1,idelem)+dla2*x0(jj,2,idelem)
&      +dla3*x0(jj,3,idelem)+dla4*x0(jj,4,idelem)
end do

detj = dsqrt(dlxyz(1)*dlxyz(1)
&      +dlxyz(2)*dlxyz(2)
&      +dlxyz(3)*dlxyz(3)) ! compute detj

call getttr(idelem,ig,ttrg,ttr)
call getomg(idelem,ig,omegag,omega)
c write(6,*) 'detj=',detj
c write(6,*) 'ttr=',ttr
c write(6,*) 'omega=', omega
call rtmatr(t,detj,dwnod,ttr,omega)

do ii = 1,3
  do jj = 1,3
    ttrg(ii,jj,ig,idelem) = ttr(ii,jj)
  end do
  omegag(ii,ig,idelem) = omega(ii)
end do

end do
end do

return
end

subroutine t6romg(NTNODE,NELM,MELM,NODBC,max,
&      x0,da,ttrg,omegag)
implicit real*8 (a-h,o-z)

dimension ttrg(3,3,6,*),omegag(3,6,*),ttr(3,3),omega(3)
dimension gauspt(6),gauswt(6),x0(3,4,*),dlxyz(3)
dimension da(*),dwnod(3,4)
dimension MELM(NELM,4),NODBC(NTNODE,6)

NGPT=3
CALL GAUSS(GAUSPT,GAUSWT,NGPT)

do idelem = 1,nelm

do nod = 1,4 ! get dwnod from da
  NODE=MELM(IDELEM,NOD)
  do ii = 1,3
    NDOF=NODBC(NODE,3+ii)
    dwnod(ii,nod)=0.d0
    IF(NDOF.NE.0) THEN
      dwnod(ii,nod) = da(NDOF)
    end if
  end do
end do ! get dwnod from da
c write(6,*) 'da=',(da(ii),ii=1,max-1)
c write(6,*) 'dw=',((dwnod(ii,nod),ii=1,3),nod=1,4)
DO IG=1,NGPT ! GAUSS INTEGRATION LOOP

```

```

      U=GAUSPT(IG)
      W=GAUSWT(IG)
      T=0.5D0*(U+1.D0)

      do idv=0,1
      &       call Lagran(t,ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4,
      &       ddla1,ddla2,ddla3,ddla4,dddla1,dddla2,dddla3,dddla4,idv)
      end do

      do jj=1,3
      &       dlxyz(jj)=dla1*x0(jj,1,idelem)+dla2*x0(jj,2,idelem)
      &       +dla3*x0(jj,3,idelem)+dla4*x0(jj,4,idelem)
      end do

      detj = dsqrt(dlxyz(1)*dlxyz(1)
      &       +dlxyz(2)*dlxyz(2)
      &       +dlxyz(3)*dlxyz(3)) ! compute detj

      call getttr(idelem,ig,ttrg,ttr)
      call getomg(idelem,ig,omegag,omega)
      c write(6,*) 'detj=',detj
      c write(6,*) 'ttr=',ttr
      c write(6,*) 'omega=', omega
      call rtmatr(t,detj,dwnod,ttr,omega)

      do ii = 1,3
      do jj = 1,3
      ttrg(ii,jj,ig,idelem) = ttr(ii,jj)
      end do
      omegag(ii,ig,idelem) = omega(ii)
      end do

      end do
      end do

      return
      end

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
c
c
c Update the total rotational matrix 'ttr' and the spatial rotational
c strain measure 'omega' at point t for an element
c
c ttr(ii,jj) - input, old total rotational matrix;
c             output, new total rotational matrix
c
c omega(jj) - input the old rotational strain measure;
c             output the new rotational strain measure
c
c dwnod(ii,nod) - input, element nodal rotational vector increment
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
c

      subroutine rtmatr(t,detj,dwnod,ttr,omega)

```



```

implicit real*8 (a-h,o-z)
dimension dwnod(3,4),ttr(3,3),omega(3)
dimension tmatdw(3,3),dtmtdw(3,3)
dimension wk0(3,3),wk1(3,3),wk2(3,3),wk3(3,3)

call  rtmatd(t,detj,dwnod,tmatdw,dtmtdw)

do ii = 1,3
  do jj = 1,3
    wk1(ii,jj) = ttr(ii,jj)
  end do
end do

call matmat(tmatdw,wk1,ttr)

do ii = 1,3
  do jj = 1,3
    wk1(ii,jj) = tmatdw(jj,ii)
  end do
end do

call vskew(omega,wk0)

call matmat(dtmtdw,wk1,wk2)

call matmat(tmatdw,wk0,wk3)

call matmat(wk3,wk1,wk0)

do ii = 1,3
  do jj = 1,3
    wk3(ii,jj) = wk2(ii,jj) + wk0(ii,jj)
  end do
end do

call skewv(wk3,omega)

return
end
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
c
c  Rotational matrix 'tmatdw' and its derivative 'dtmtdw' with respect
c  to arc length parameterized by rotational vector-dw.
c
c  dwnod(ii,nod) - input, nodal rotational vector
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc

```

```

subroutine rtmatd(t,detj,dwnod,tmatdw,dtmtdw)

implicit real*8 (a-h,o-z)
dimension dwnod(3,4),tmatdw(3,3),dtmtdw(3,3)
dimension dwvec(3),ddwvec(3)
dimension sk(3,3),dsk(3,3)
dimension wk1(3,3),wk2(3,3),wk3(3,3)

```

```

        idv = 0
        call Lagran(t,ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4,
&          ddla1,ddla2,ddla3,ddla4,dddla1,dddla2,dddla3,dddla4,idv)
        idv = 1
        call Lagran(t,ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4,
&          ddla1,ddla2,ddla3,ddla4,dddla1,dddla2,dddla3,dddla4,idv)

do jj=1,3

        dwvec(jj) = ala1*dwnod(jj,1)
&                +ala2*dwnod(jj,2)
&                +ala3*dwnod(jj,3)
&                +ala4*dwnod(jj,4)
        ddwvec(jj) = (dla1*dwnod(jj,1)
&                  +dla2*dwnod(jj,2)
&                  +dla3*dwnod(jj,3)
&                  +dla4*dwnod(jj,4))/detj

end do

dw = dsqrt(dwvec(1)*dwvec(1)
&         +dwvec(2)*dwvec(2)
&         +dwvec(3)*dwvec(3))

ddw = (dwvec(1)*ddwvec(1)
&      +dwvec(2)*ddwvec(2)
&      +dwvec(3)*ddwvec(3))/dw

do ii = 1,3
  do jj = 1,3
    tmatdw(ii,jj) = 0.d0
    dtmtdw(ii,jj) = 0.d0
  end do
  tmatdw(ii,ii) = 1.d0
end do
if(dw.lt.1.d-10) then

  return

end if
call vskew(dwvec,sk)
call vskew(ddwvec,dsk)
call matmat(sk,sk,wk1)
call matmat(dsk,sk,wk2)
call matmat(sk,dsk,wk3)

r1= dsin(dw)/dw
r2= (1.d0-dcos(dw))/dw/dw
dr1 = (dcos(dw) - r1)/dw*ddw
dr2 = (r1-2.d0*r2)/dw*ddw

do ii = 1,3
  do jj = 1,3

```

```

        tmatdw(ii,jj)=tmatdw(ii,jj)+r1*sk(ii,jj)+r2*wk1(ii,jj)
    end do
end do

do ii = 1,3
    do jj = 1,3
        dtmtdw(ii,jj)=dtmtdw(ii,jj)+dr1*sk(ii,jj)+r1*dsk(ii,jj)
&             +dr2*wk1(ii,jj)+r2*(wk2(ii,jj)+wk3(ii,jj))
    end do
end do

return
end

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc
c
c  Update the overall orthogonal matrix 'tt'.
c
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc

subroutine otmat(ttr,tt0,tt)
    implicit real*8 (a-h,o-z)
    dimension ttr(3,3),tt0(3,3),tt(3,3)

    call matmat(ttr,tt0,tt)

return
end

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc
c
c  Update the global nodal displacement vectors "atotl".
c
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc

subroutine upatot(ntnode,NODBC,atotl,da)

IMPLICIT REAL*8 (A-H,O-Z)
dimension atotl(*), dwvec(3), theta(3), dtheta(3)
dimension NODBC(ntnode,6),da(*)

do ii=1,ntnode
    do jj=1,3
        ndof=NODBC(II,JJ)
        if(ndof.ne.0) then
            atotl(ndof) = atotl(ndof) + da(ndof)
        end if
    end do
    dwvec(1)=0.
    dwvec(2)=0.
    dwvec(3)=0.
    theta(1)=0.

```

```

        theta(2)=0.
        theta(3)=0.
    do jj=1,3
        ndof=NODBC(II,JJ+3)
        if(ndof.ne.0) then
            theta(jj)=atotl(ndof)
            dwvec(jj)= da(ndof)
        end if
    end do
    call dwthet(dwvec,theta,dtheta)
    do jj=1,3
        ndof=NODBC(II,JJ+3)
        if(ndof.ne.0) then
            atotl(ndof)=atotl(ndof)+dtheta(jj)
        end if
    end do

end do

return
end
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc
c
c  get dtheta from dwvec
c
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc

subroutine dwthet(dwvec,theta,dtheta)
implicit real*8 (a-h,o-z)
dimension dwvec(3),theta(3),dtheta(3),Tinv(3,3)

call invTmt(theta,Tinv)

do ii = 1,3
    dtheta(ii) = 0.
end do

do ii = 1,3
    dtheta(ii) = dtheta(ii)
&           + Tinv(ii,1)*dwvec(1)
&           + Tinv(ii,2)*dwvec(2)
&           + Tinv(ii,3)*dwvec(3)
end do

return
end

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc
c
c  get inverse of T(theta): Tinv
c
c

```



```

        skv(3,3) = 0.d0

        return
    end

CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC
c
c   Get vector 'v' from skew matrix 'skv'
c
CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC
    subroutine skewv(skv,v)
        implicit real*8 (a-h,o-z)
        dimension v(3),skv(3,3)

        v(1) = 0.5d0*(skv(3,2) - skv(2,3))
        v(2) = 0.5d0*(skv(1,3) - skv(3,1))
        v(3) = 0.5d0*(skv(2,1) - skv(1,2))

        return
    end

CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC
c
c   Get product 'c' of two 3 by 3 matrices 'a' an 'b':
c
c           c = a b
c
CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC

    subroutine matmat(a,b,c)
        implicit real*8 (a-h,o-z)
        dimension a(3,3),b(3,3),c(3,3)

        do i = 1,3
            do j = 1,3
                c(i,j) = 0.d0
                do k = 1,3
                    c(i,j) = c(i,j) + a(i,k)*b(k,j)
                end do
            end do
        end do
        return
    end

CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC
cc
c
c   Updated
c   position vector 'phiv' and its derivative 'dphiv' at 't'
c   for an element
c
CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC
cc

    subroutine posvec(t,detj,x,phiv,dphiv)
        implicit real*8 (a-h,o-z)

```

```

dimension x(3,4),phiv(3),dphiv(3)
idv =0
call Lagran(t,ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4,
&      ddla1,ddla2,ddla3,ddla4,dddla1,dddla2,dddla3,dddla4,idv)

idv =1
call Lagran(t,ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4,
&      ddla1,ddla2,ddla3,ddla4,dddla1,dddla2,dddla3,dddla4,idv)

do ii = 1,3

    phiv(ii) = ala1*x(ii,1)
&           +ala2*x(ii,2)
&           +ala3*x(ii,3)
&           +ala4*x(ii,4)

    dphiv(ii) = (dla1*x(ii,1)
&              +dla2*x(ii,2)
&              +dla3*x(ii,3)
&              +dla4*x(ii,4))/detj

end do

return
end

```

```

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
c
c   Calculate:
c          ccnm = tt.Cnm.Transpose(tt)
c
c          cnm = cn or cm
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
subroutine cnmmat(tt,cnm,ccnm)
implicit real*8 (a-h,o-z)
dimension tt(3,3),cnm(3,3),ccnm(3,3)
ccnm(1,1) = cnm(1,1)*tt(1,1)*tt(1,1)
&          + cnm(2,2)*tt(1,2)*tt(1,2)
&          + cnm(3,3)*tt(1,3)*tt(1,3)
ccnm(1,2) = cnm(1,1)*tt(1,1)*tt(2,1)
&          + cnm(2,2)*tt(1,2)*tt(2,2)
&          + cnm(3,3)*tt(1,3)*tt(2,3)
ccnm(1,3) = cnm(1,1)*tt(1,1)*tt(3,1)
&          + cnm(2,2)*tt(1,2)*tt(3,2)
&          + cnm(3,3)*tt(1,3)*tt(3,3)
ccnm(2,1) = cnm(1,1)*tt(1,1)*tt(2,1)
&          + cnm(2,2)*tt(1,2)*tt(2,2)
&          + cnm(3,3)*tt(1,3)*tt(2,3)
ccnm(2,2) = cnm(1,1)*tt(2,1)*tt(2,1)
&          + cnm(2,2)*tt(2,2)*tt(2,2)
&          + cnm(3,3)*tt(2,3)*tt(2,3)
ccnm(2,3) = cnm(1,1)*tt(2,1)*tt(3,1)
&          + cnm(2,2)*tt(2,2)*tt(3,2)
&          + cnm(3,3)*tt(2,3)*tt(3,3)
ccnm(3,1) = cnm(1,1)*tt(1,1)*tt(3,1)

```



```

c    Calculate the real stress resultants:
c
c      rn = transpose[tt].sn
c      rm = transpose[tt].sm
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
subroutine rstrss(tt,sn,sm,rn,rm)
implicit real*8 (a-h,o-z)
dimension tt(3,3),sn(3),sm(3),rn(3),rm(3)
do ii = 1,3
  rn(ii) = tt(1,ii)*sn(1)+tt(2,ii)*sn(2)+tt(3,ii)*sn(3)
  rm(ii) = tt(1,ii)*sm(1)+tt(2,ii)*sm(2)+tt(3,ii)*sm(3)
end do
return
end

```

```

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
c
c
c
c
c    Note: Degrees of freedom for an element
c
c      Incremental displacements           Internal loads
c
c                                         ( External loads :)
c                                         (change 'int' to 'ext')
c
c    DOF
c
c    1    dae(1) = dx(1,1)       qinte(1) = pinte(1,1)
c    2    dae(2) = dx(2,1)       qinte(2) = pinte(2,1)
c    3    dae(3) = dx(3,1)       qinte(3) = pinte(3,1)
c    4    dae(4) = dwnod(1,1)    qinte(4) = aminte(1,1)
c    5    dae(5) = dwnod(2,1)    qinte(5) = aminte(2,1)
c    6    dae(6) = dwnod(3,1)    qinte(6) = aminte(3,1)
c    7    dae(7) = dx(1,2)       qinte(7) = pinte(1,2)
c    8    dae(8) = dx(2,2)       qinte(8) = pinte(2,2)
c    9    dae(9) = dx(3,2)       qinte(9) = pinte(3,2)
c    10   dae(10) = dwnod(1,2)   qinte(10) = aminte(1,2)
c    11   dae(11) = dwnod(2,2)   qinte(11) = aminte(2,2)
c    12   dae(12) = dwnod(3,2)   qinte(12) = aminte(3,2)
c    13   dae(13) = dx(1,3)      qinte(13) = pinte(1,3)
c    14   dae(14) = dx(2,3)      qinte(14) = pinte(2,3)
c    15   dae(15) = dx(3,3)      qinte(15) = pinte(3,3)
c    16   dae(16) = dwnod(1,3)   qinte(16) = aminte(1,3)
c    17   dae(17) = dwnod(2,3)   qinte(17) = aminte(2,3)
c    18   dae(18) = dwnod(3,3)   qinte(18) = aminte(3,3)
c    19   dae(19) = dx(1,4)      qinte(19) = pinte(1,4)
c    20   dae(20) = dx(2,4)      qinte(20) = pinte(2,4)
c    21   dae(21) = dx(3,4)      qinte(21) = pinte(3,4)
c    22   dae(22) = dwnod(1,4)   qinte(22) = aminte(1,4)
c    23   dae(23) = dwnod(2,4)   qinte(23) = aminte(2,4)
c    24   dae(24) = dwnod(3,4)   qinte(24) = aminte(3,4)
c

```

```

c
c
c
c2345678901234567890123456789012345678901234567890123456789012345678901
2
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
c
c
c      Element tangent Stiffness matrix:
c
c      stifel(ii,jj) -- (ii=1,2,...,24; jj=1,2,...,24)
c
c
c
c
c      subroutine elstif(idelem,x0,xnode1,ttrg,omegag,cncm,
&          t6rg,om6gag,distrn,distrp,stifel)
c      implicit real*8 (a-h,o-z)
c      dimension stifel(24,24),stifem(24,24),stifeg(24,24)
c      dimension stifen(24,24),stifep(24,24)
c      dimension gauspt(6),gauswt(6)
c      dimension x0(3,4,*),xnode1(3,4,*),x(3,4)
c      dimension tt0(3,3),dltt0(3,3)
c      dimension ttrg(3,3,6,*),ttr(3,3),tt(3,3)
c      dimension t6rg(3,3,6,*),t6r(3,3)
c      dimension omegag(3,6,*),omega(3)
c      dimension om6gag(3,6,*),om6g(3,3)
c      dimension phiv(3),dphiv(3),dphisk(3,3),gamma(3)
c      dimension cncm(6,4,*),cnc(3,3),ccn(3,3),cm(3,3),ccm(3,3)
c      dimension sn(3),snsk(3,3),sm(3),smsk(3,3),rn(3),rm(3)
c      dimension ccnphi(3,3),phiccn(3,3),phiphi(3,3)
c      dimension phisn(3,3),snphi(3,3),shpn(4),dshpn(4)
c      dimension pv(3),pskw(3,3),pv1(3),distrn(3,4),distrp(3,4)
c      dimension anv(3),dv(3),wkn(3,3)

c      dv(1)= 1.d0
c      dv(1)= 0.d0
c      dv(1)= 0.d0

c      DO II=1,24
c        DO JJ=1,24
c          STIFel(II,JJ)=0.D0
c          STIFem(II,JJ)=0.D0
c          STIFeg(II,JJ)=0.D0
c          STIFen(II,JJ)=0.D0
c          STIFep(II,JJ)=0.D0
c        END DO
c      END DO

c      NGPT=3
c      CALL GAUSS(GAUSPT,GAUSWT,NGPT)

c      DO IG=1,NGPT      ! GAUSS INTEGRATION LOOP

c        U=GAUSPT(IG)
c        W=GAUSWT(IG)

```

```

T=0.5D0*(U+1.D0)

call Frenet(idelem,t,x0,tt0,d1tt0,detj,
&          ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4)

c write(6,*) 'idelem=',idelem
c write(6,*) 'tt0=', tt0

detjh=.5D0*DETJ

shpn(1) = ala1
shpn(2) = ala2
shpn(3) = ala3
shpn(4) = ala4
dshpn(1) = dla1/detj
dshpn(2) = dla2/detj
dshpn(3) = dla3/detj
dshpn(4) = dla4/detj

call getcnm(idelem,cncm,cm,cn,shpn)
call getttr(idelem,ig,ttrg,ttr)

call getomg(idelem,ig,omegag,omega)

call otmat(ttr,tt0,tt)

call getx(idelem,xnode1,x)

call posvec(t,detj,x,phiv,dphiv)

call gammav(tt0,ttr,dphiv,gamma)

call cnmmat(tt,cn,ccn)
call cnmmat(tt,cm,ccm)

call sstrss(ccn,ccm,gamma,omega,sn,sm)

call vskew(dphiv,dphisk)

call matmat(ccn,dphisk,ccnphi)

call matmat(dphisk,ccn,phiccn)

call matmat(dphisk,ccnphi,phiphi)    ! ??????????

call vskew(sn,snsk)
call vskew(sm,smsk)

call matmat(dphisk,snsk,phisn)
call matmat(snsk,dphisk,snphi)

call VECVEC(dphiv,dphiv,andphi,3)
andphi = dsqrt(andphi)
call VECVEC(dv,dphiv,ddphi,3)

coe1 = ddphi/(andphi*andphi)

```

```

coe2 = -1.d0/andphi

do jj = 1,3
  anv(jj)= shpn(1)*distrn(jj,1)+
&          shpn(2)*distrn(jj,2)+
&          shpn(3)*distrn(jj,3)+
&          shpn(4)*distrn(jj,4)
  pv(jj) = shpn(1)*distrp(jj,1)+
&          shpn(2)*distrp(jj,2)+
&          shpn(3)*distrp(jj,3)+
&          shpn(4)*distrp(jj,4)
end do

do ii = 1,3
  do jj = 1,3
    wkn(ii,jj) =anv(ii)*(coe1*dphiv(jj) + coe2*dv(jj))
  end do
end do

do jj = 1,3
  pv1(jj) = tt(jj,1)*pv(1)+
&          tt(jj,2)*pv(2)+
&          tt(jj,3)*pv(3)
end do

call vskew(pv1, pskw)

do ii = 1,4
  do jj = 1,4

    do i = 1,3
      do j = 1,3
        stifem(6*(ii-1)+i,6*(jj-1)+j) =
&          stifem(6*(ii-1)+i,6*(jj-1)+j)
&          + w*dshpn(ii)*dshpn(jj)*ccn(i,j)*detjh
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifem(6*(ii-1)+i,6*(jj-1)+3+j) =
&          stifem(6*(ii-1)+i,6*(jj-1)+3+j)
&          + w*dshpn(ii)*shpn(jj)*ccnphi(i,j)*detjh
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifem(6*(ii-1)+3+i,6*(jj-1)+j) =
&          stifem(6*(ii-1)+3+i,6*(jj-1)+j)
&          -w*shpn(ii)*dshpn(jj)*phiccn(i,j)*detjh
      end do
    end do
  end do
end do

```

```

do i = 1,3
  do j = 1,3
    stifem(6*(ii-1)+3+i,6*(jj-1)+3+j) =
&          stifem(6*(ii-1)+3+i,6*(jj-1)+3+j)
&          +w*(dshpn(ii)*dshpn(jj)*ccm(i,j)
&          -shpn(ii)*shpn(jj)*phiphi(i,j))*detjh
    end do
  end do

end do
end do

do ii = 1,4
  do jj = 1,4

    do i = 1,3
      do j = 1,3
        stifeg(6*(ii-1)+i,6*(jj-1)+j) =
&          stifeg(6*(ii-1)+i,6*(jj-1)+j)
&          + 0.d0
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifeg(6*(ii-1)+i,6*(jj-1)+3+j) =
&          stifeg(6*(ii-1)+i,6*(jj-1)+3+j)
&          - w*dshpn(ii)*shpn(jj)*snsk(i,j)*detjh
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifeg(6*(ii-1)+3+i,6*(jj-1)+j) =
&          stifeg(6*(ii-1)+3+i,6*(jj-1)+j)
&          + w*shpn(ii)*dshpn(jj)*snsk(i,j)*detjh
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifeg(6*(ii-1)+3+i,6*(jj-1)+3+j) =
&          stifeg(6*(ii-1)+3+i,6*(jj-1)+3+j)
&          + w*(shpn(ii)*shpn(jj)*phisn(i,j)
&          - dshpn(ii)*shpn(jj)*smsk(i,j))*detjh
      end do
    end do

  end do
end do

do ii = 1,4
  do jj = 1,4

    do i = 1,3
      do j = 1,3

```

```

        stifep(6*(ii-1)+i,6*(jj-1)+j) =
&           stifep(6*(ii-1)+i,6*(jj-1)+j)
&           + w*shpn(ii)*dshpn(jj)*wkn(i,j)*detjh
    end do
end do

do i = 1,3
  do j = 1,3
    stifep(6*(ii-1)+i,6*(jj-1)+3+j) =
&           stifep(6*(ii-1)+i,6*(jj-1)+3+j)
&           + 0.d0
  end do
end do

do i = 1,3
  do j = 1,3
    stifep(6*(ii-1)+3+i,6*(jj-1)+j) =
&           stifep(6*(ii-1)+3+i,6*(jj-1)+j)
&           + 0.d0
  end do
end do

do i = 1,3
  do j = 1,3
    stifep(6*(ii-1)+3+i,6*(jj-1)+3+j) =
&           stifep(6*(ii-1)+3+i,6*(jj-1)+3+j)
&           + 0.d0
  end do
end do

end do
end do

do ii = 1,4
  do jj = 1,4

    do i = 1,3
      do j = 1,3
        stifep(6*(ii-1)+i,6*(jj-1)+j) =
&           stifep(6*(ii-1)+i,6*(jj-1)+j)
&           + 0.d0
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifep(6*(ii-1)+i,6*(jj-1)+3+j) =
&           stifep(6*(ii-1)+i,6*(jj-1)+3+j)
&           + w*shpn(ii)*shpn(jj)*pskw(i,j)*detjh
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifep(6*(ii-1)+3+i,6*(jj-1)+j) =
&           stifep(6*(ii-1)+3+i,6*(jj-1)+j)
&           + 0.d0
      end do
    end do
  end do
end do

```

```

        end do
    end do

    do i = 1,3
        do j = 1,3
            stifep(6*(ii-1)+3+i,6*(jj-1)+3+j) =
&             stifep(6*(ii-1)+3+i,6*(jj-1)+3+j)
&             + 0.d0
        end do
    end do

    end do
end do

end do

do ii = 1,24
    do jj = 1,24
        stifel(ii,jj) = stifem(ii,jj)
&             + stifeg(ii,jj)
&             + stifen(ii,jj)
&             + stifep(ii,jj)
    end do
end do

return
end

subroutine getcnm(idelem,cncm,cm,cn,shpn)
implicit real*8 (a-h,o-z)
dimension cncm(6,4,*),cn(3,3),cm(3,3),shpn(4)

do ii=1,3
    do jj =1,3
        cn(ii,jj) = 0.
        cm(ii,jj) = 0.
    end do
end do

do ii = 1,3
    do nod = 1,4
        cn(ii,ii) = cn(ii,ii)+shpn(nod)*cncm(ii,nod,idelem)
        cm(ii,ii) = cm(ii,ii)+shpn(nod)*cncm(3+ii,nod,idelem)
    end do
end do

return
end

subroutine elstin(idelem,x0,xnodel,ttrg,omegag,cncm,
&             t6rg,om6gag,distrn,distrp,stifel)
implicit real*8 (a-h,o-z)
dimension stifel(24,24),stifem(24,24),stifeg(24,24)
dimension stifen(24,24),stifep(24,24)
dimension gauspt(6),gauswt(6)
dimension x0(3,4,*),xnodel(3,4,*),x(3,4)

```

```

dimension tt0(3,3),dltt0(3,3)
dimension ttrg(3,3,6,*),ttr(3,3),tt(3,3)
dimension t6rg(3,3,6,* )
dimension omegag(3,6,*),omega(3)
dimension om6gag(3,6,* )
dimension phiv(3),dphiv(3),dphisk(3,3),gamma(3)
dimension cncm(6,4,* )
dimension cn(3,3),ccn(3,3),cm(3,3),ccm(3,3)
dimension sn(3),snsk(3,3),sm(3),smsk(3,3), rn(3),rm(3)
dimension ccnphi(3,3),phiccn(3,3),phiphi(3,3)
dimension phisn(3,3),snphi(3,3),shpn(4),dshpn(4)
dimension pv(3),pskw(3,3),pv1(3),distrn(3,4),distrp(3,4)
dimension anv(3),dv(3),wkn(3,3)

dv(1)= 1.d0
dv(1)= 0.d0
dv(1)= 0.d0

DO II=1,24
  DO JJ=1,24
    STIFel(II,JJ)=0.D0
    STIFem(II,JJ)=0.D0
    STIFeg(II,JJ)=0.D0
    STIFen(II,JJ)=0.D0
    STIFep(II,JJ)=0.D0
  END DO
END DO

NGPT=3
CALL GAUSS(GAUSPT,GAUSWT,NGPT)

DO IG=1,NGPT      ! GAUSS INTEGRATION LOOP

  U=GAUSPT(IG)
  W=GAUSWT(IG)
  T=0.5D0*(U+1.D0)

  call Frenet(idelem,t,x0,tt0,dltt0,detj,
&              ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4)

c      write(6,*) 'idelem=',idelem
c      write(6,*) 'tt0=', tt0

  detjh=.5D0*DETJ

  shpn(1) = ala1
  shpn(2) = ala2
  shpn(3) = ala3
  shpn(4) = ala4
  dshpn(1) = dla1/detj
  dshpn(2) = dla2/detj
  dshpn(3) = dla3/detj
  dshpn(4) = dla4/detj

  call getcncm(idelem,cncm,cm,cn,shpn)
  call getttr(idelem,ig,ttrg,ttr)

```



```

call getomg(idelem,ig,omegag,omega)

call otmat(ttr,tt0,tt)

call getx(idelem,xnode1,x)

call posvec(t,detj,x,phiv,dphiv)

call gammav(tt0,ttr,dphiv,gamma)

call cnmmat(tt,cn,ccn)
call cnmmat(tt,cm,ccm)

call sstrss(ccn,ccm,gamma,omega,sn,sm)

call vskew(dphiv,dphisk)

call matmat(ccn,dphisk,ccnphi)

call matmat(dphisk,ccn,phiccn)

call matmat(dphisk,ccnphi,phiphi)    ! ??????????

call vskew(sn,snsk)
call vskew(sm,smsk)

call matmat(dphisk,snsk,phisn)
call matmat(snsk,dphisk,snphi)

call VECVEC(dphiv,dphiv,andphi,3)
andphi = dsqrt(andphi)
call VECVEC(dv,dphiv,ddphi,3)

coe1 = ddphi/(andphi*andphi)
coe2 = -1.d0/andphi

do jj = 1,3
  anv(jj)= shpn(1)*distrn(jj,1)+
&          shpn(2)*distrn(jj,2)+
&          shpn(3)*distrn(jj,3)+
&          shpn(4)*distrn(jj,4)
  pv(jj) = shpn(1)*distrp(jj,1)+
&          shpn(2)*distrp(jj,2)+
&          shpn(3)*distrp(jj,3)+
&          shpn(4)*distrp(jj,4)
end do

do ii = 1,3
  do jj = 1,3
    wkn(ii,jj) =anv(ii)*(coe1*dphiv(jj) + coe2*dv(jj))
  end do
end do

do jj = 1,3

```

```

        pv1(jj) = tt(jj,1)*pv(1)+
&                tt(jj,2)*pv(2)+
&                tt(jj,3)*pv(3)
    end do

    call vskew(pv1, pskw)

    do ii = 1,4
        do jj = 1,4

            do i = 1,3
                do j = 1,3
                    stifeg(6*(ii-1)+i,6*(jj-1)+j) =
&                        stifeg(6*(ii-1)+i,6*(jj-1)+j)
&                        + 0.d0
                end do
            end do

            do i = 1,3
                do j = 1,3
                    stifeg(6*(ii-1)+i,6*(jj-1)+3+j) =
&                        stifeg(6*(ii-1)+i,6*(jj-1)+3+j)
&                        - w*dshpn(ii)*shpn(jj)*snsk(i,j)*detjh
                end do
            end do

            do i = 1,3
                do j = 1,3
                    stifeg(6*(ii-1)+3+i,6*(jj-1)+j) =
&                        stifeg(6*(ii-1)+3+i,6*(jj-1)+j)
&                        + w*shpn(ii)*dshpn(jj)*snsk(i,j)*detjh
                end do
            end do

            do i = 1,3
                do j = 1,3
                    stifeg(6*(ii-1)+3+i,6*(jj-1)+3+j) =
&                        stifeg(6*(ii-1)+3+i,6*(jj-1)+3+j)
&                        + w*(shpn(ii)*shpn(jj)*phisn(i,j)
&                        - dshpn(ii)*shpn(jj)*smsk(i,j))*detjh
                end do
            end do

        end do
    end do

    do ii = 1,4
        do jj = 1,4

            do i = 1,3
                do j = 1,3
                    stifen(6*(ii-1)+i,6*(jj-1)+j) =
&                        stifen(6*(ii-1)+i,6*(jj-1)+j)
&                        + w*shpn(ii)*dshpn(jj)*wkn(i,j)*detjh
                end do
            end do
        end do
    end do

```

```

do i = 1,3
  do j = 1,3
    stifen(6*(ii-1)+i,6*(jj-1)+3+j) =
&          stifen(6*(ii-1)+i,6*(jj-1)+3+j)
&          + 0.d0
  end do
end do

do i = 1,3
  do j = 1,3
    stifen(6*(ii-1)+3+i,6*(jj-1)+j) =
&          stifen(6*(ii-1)+3+i,6*(jj-1)+j)
&          + 0.d0
  end do
end do

do i = 1,3
  do j = 1,3
    stifen(6*(ii-1)+3+i,6*(jj-1)+3+j) =
&          stifen(6*(ii-1)+3+i,6*(jj-1)+3+j)
&          + 0.d0
  end do
end do

end do
end do

do ii = 1,4
  do jj = 1,4

    do i = 1,3
      do j = 1,3
        stifep(6*(ii-1)+i,6*(jj-1)+j) =
&          stifep(6*(ii-1)+i,6*(jj-1)+j)
&          + 0.d0
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifep(6*(ii-1)+i,6*(jj-1)+3+j) =
&          stifep(6*(ii-1)+i,6*(jj-1)+3+j)
&          + w*shpn(ii)*shpn(jj)*pskw(i,j)*detjh
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifep(6*(ii-1)+3+i,6*(jj-1)+j) =
&          stifep(6*(ii-1)+3+i,6*(jj-1)+j)
&          + 0.d0
      end do
    end do

    do i = 1,3
      do j = 1,3
        stifep(6*(ii-1)+3+i,6*(jj-1)+3+j) =

```

```

&             stifep(6*(ii-1)+3+i,6*(jj-1)+3+j)
&             + 0.d0
             end do
             end do

             end do
             end do

             end do

do ii = 1,24
  do jj = 1,24
    stifel(ii,jj) = stifeg(ii,jj)
&                + stifen(ii,jj)
&                + stifep(ii,jj)
  end do
end do

return
end

```

```

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
c
c  Calculate internal nodal loading of an element
c
c
c
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
subroutine qintel(idelem,x0,xnodel,ttrg,omegag,
&               cncm,qinte)
IMPLICIT REAL*8 (A-H,O-Z)

dimension qinte(24)
dimension gauspt(6),gauswt(6)
dimension x0(3,4,*),xnodel(3,4,*),x(3,4)
dimension tt0(3,3),dltt0(3,3)
dimension ttrg(3,3,6,*),ttr(3,3),tt(3,3)
dimension omegag(3,6,*),omega(3)
dimension phiv(3),dphiv(3),dphisk(3,3),gamma(3)
dimension cncm(6,4,*)
dimension cn(3,3),ccn(3,3),cm(3,3),ccm(3,3)
dimension sn(3),sm(3),rn(3),rm(3)
dimension shpn(4),dshpn(4)
c  write(6,*) 'idelem=',idelem
c  write(6,*) ' '
do ii=1,24
  qinte(ii)=0.d0
end do

NGPT=3
CALL GAUSS(GAUSPT,GAUSWT,NGPT)

DO IG=1,NGPT ! GAUSS INTEGRATION LOOP

  U=GAUSPT(IG)

```

```

W=GAUSWT(IG)
T=0.5D0*(U+1.D0)

call Frenet(idelem,t,x0,tt0,d1tt0,detj,
&          ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4)

DETJH=.5D0*DETJ
shpn(1) = ala1
shpn(2) = ala2
shpn(3) = ala3
shpn(4) = ala4
dshpn(1) = dla1/detj
dshpn(2) = dla2/detj
dshpn(3) = dla3/detj
dshpn(4) = dla4/detj

call getcnm(idelem,cncm,cm,cn,shpn)
call getttr(idelem,ig,ttrg,ttr)
call getomg(idelem,ig,omegag,omega)
call otmat(ttr,tt0,tt)

call getx(idelem,xnode1,x)
call posvec(t,detj,x,phiv,dphiv)
call gammav(tt0,ttr,dphiv,gamma)
c      write(6,*) 'phiv=',phiv
c      write(6,*) 'dphiv=',dphiv
c      write(6,*) 'gamma=',gamma

call cnmmat(tt,cn,ccn)
call cnmmat(tt,cm,ccm)
c      write(6,*) 'tt0=',tt0

c      write(6,*) 'ttr='
c      do iii=1,3
c          write(6,*) (ttr(iii,jjj),jjj=1,3)
c      end do
call sstrss(ccn,ccm,gamma,omega,sn,sm)
c      write(6,*) 'sn=',sn
c      write(6,*) 'sm=',sm

call vskew(dphiv,dphisk)

call rstrss(tt,sn,sm,rn,rm)
c      write(6,*) 'rn=',rn
c      write(6,*) 'rm=',rm

do ii = 1,4
  do i = 1,3

      qinte(6*(ii-1)+i) = qinte(6*(ii-1)+i)
&                          +w*dshpn(ii)*sn(i)*detjh

      qinte(6*(ii-1)+3+i) = qinte(6*(ii-1)+3+i)
&                          + w*(dshpn(ii)*sm(i)
&                          - shpn(ii)*(dphisk(i,1)*sn(1)
&                          +dphisk(i,2)*sn(2)

```

```

& +dphisk(i,3)*sn(3))*detjh

      end do
    end do

  end do

  return
end

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
cc
c
c
c Calculate external nodal loading of an element
c
c distrg(3,4) -- distributed load density value of weight on nodes
c
c distrn(3,4) -- distributed load density value of force on nodes
c
c distrm(3,4) -- distributed load density value of moment on nodes
c
c qexte(24) -- nodal load array of external load
c
c x0(3,4,*) -- position vectors of nodes of elements (undeformed)
c
c xnodel(3,4,*) -- position vectors of nodes of elements (current)
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
cc
c
  subroutine qextel(idelem,x0,xnodel,
&                   distrg,distrn,distrp,ttrg,qexte)
  IMPLICIT REAL*8 (A-H,O-Z)

  dimension distrg(3,4),distrn(3,4),distrp(3,4)
  dimension distrm(3,4),qexte(24)
  dimension gauspt(6),gauswt(6),shpn(4),dlxyz(3)
  dimension x0(3,4,*),xnodel(3,4,*),x(3,4)
  dimension tt0(3,3),dltt0(3,3)
  dimension ttrg(3,3,6,*)
  dimension pv(3), pv1(3),ttr(3,3),tt(3,3)

c
c
c
c at this time distrm = 0, no distrib. moment load
c

  do i=1,3
    do j =1,4
      distrm(i,j)=0.d0
    end do
    pv1(i) = 0.
  end do

  do ii=1,24
    qexte(ii)=0.d0
  end do

```

```

NGPT=3
CALL GAUSS(GAUSPT,GAUSWT,NGPT)

DO IG=1,NGPT      ! GAUSS INTEGRATION LOOP

    U=GAUSPT(IG)
    W=GAUSWT(IG)
    T=0.5D0*(U+1.D0)

    call Frenet(idelem,t,x0,tt0,dltt0,detj,
&              ala1,ala2,ala3,ala4,dla1,dla2,dla3,dla4)

    shpn(1) = ala1
    shpn(2) = ala2
    shpn(3) = ala3
    shpn(4) = ala4

    call getttr(idelem,ig,ttrg,ttr)
    call otmat(ttr,tt0,tt)

    do jj = 1,3
        pv(jj) = shpn(1)*distrp(jj,1)+
&              shpn(2)*distrp(jj,2)+
&              shpn(3)*distrp(jj,3)+
&              shpn(4)*distrp(jj,4)
    end do

    do jj = 1,3
        pv1(jj) = tt(jj,1)*pv(1)+
&              tt(jj,2)*pv(2)+
&              tt(jj,3)*pv(3)
    end do

c        write(6,*) 'pv1=', pv1

    DETJH=.5D0*DETJ
    do ii = 1,4
        do i = 1,3

            qexpte(6*(ii-1)+i) = qexpte(6*(ii-1)+i)
&            +w*shpn(ii)*(shpn(1)*(distrg(i,1)+distrn(i,1)*tt(1,1))
&            +shpn(2)*(distrg(i,2)+distrn(i,2)*tt(1,1))
&            +shpn(3)*(distrg(i,3)+distrn(i,3)*tt(1,1))
&            +shpn(4)*(distrg(i,4)+distrn(i,4)*tt(1,1))
&            + pv1(i))*detjh

            qexpte(6*(ii-1)+3+i) = qexpte(6*(ii-1)+3+i)
&            +w*shpn(ii)*(shpn(1)*distrm(i,1)
&            +shpn(2)*distrm(i,2)
&            +shpn(3)*distrm(i,3)
&            +shpn(4)*distrm(i,4))*detjh

        end do
    end do

end do

```

```

        return
    end

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
cc
c
c   Update the
c     nodal position vector 'xnodel(ii,nod,idelem)' element by
element
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
cc
c
c

    subroutine upxnod(ntnode,nelm,MELM,NODBC,xnodel,da)

    IMPLICIT REAL*8 (A-H,O-Z)
    dimension xnodel(3,4,*), x(3,4)
    dimension MELM(NELM,4),NODBC(ntnode,6),da(*)

    do idelem = 1,nelm

        do nod = 1,4
            NODE=MELM(IDELEM,NOD)
            do ii = 1,3
                NDOF=NODBC(NODE,ii)
                IF(NDOF.NE.0) THEN
                    xnodel(ii,nod,idelem) = xnodel(ii,nod,idelem)
&                                         +da(NDOF)
                    end if
                end do
            end do

        end do

    return
end

    subroutine getx(idelem,xnodel,x)
    IMPLICIT REAL*8 (A-H,O-Z)
    dimension xnodel(3,4,*), x(3,4)

    do nod = 1,4
        do ii = 1,3
            x(ii,nod) = xnodel(ii,nod,idelem)
        end do
    end do

    return
end

    subroutine getttr(idelem,ig,ttrg,ttr)
    IMPLICIT REAL*8 (A-H,O-Z)
    dimension ttrg(3,3,6,*),ttr(3,3)

```



```

do ii = 1,3
  do jj = 1,3
    ttr(ii,jj) =ttrg(ii,jj,ig,idelem)
  end do
end do
return
end

subroutine getomg(idelem,ig,omegag,omega)
IMPLICIT REAL*8 (A-H,O-Z)
dimension omeag(3,6,*),omega(3)
do ii = 1,3
  omega(ii) = omeag(ii,ig,idelem)
end do
return
end

subroutine getgld(idelem,distgg,distrg,alambd)
IMPLICIT REAL*8 (A-H,O-Z)
dimension distgg(3,4,*),distrg(3,4)

do i = 1,3
  do j = 1,4
    distrg(i,j)=alambd*distgg(i,j,idelem)
  end do
end do

return
end

subroutine getnld(idelem,distnn,distrn,alambd)
IMPLICIT REAL*8 (A-H,O-Z)
dimension distnn(3,4,*),distrn(3,4)

do i = 1,3
  do j = 1,4
    distrn(i,j)=alambd*distnn(i,j,idelem)
  end do
end do

return
end

subroutine getpld(idelem,distpp,distrp,alambd)
IMPLICIT REAL*8 (A-H,O-Z)
dimension distpp(3,4,*),distrp(3,4)

do i = 1,3
  do j = 1,4
    distrp(i,j)=alambd*distpp(i,j,idelem)
  end do
end do

return
end

```

```

SUBROUTINE GAUSS(X,W,N)
C
C
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION X(N),W(N)

if(n.eq.2) then
  X(1)=-0.577350269189626D0
  X(2)=-X(1)
  W(1)=1.D0
  W(2)=1.D0
end if

if(n.eq.3) then
  X(1)=-0.774596669241483D0
  X(2)=0.d0
  X(3)=-X(1)
  W(1)=0.5555555555555555D0
  W(2)=0.8888888888888889D0
  W(3)=W(1)
end if

if(n.eq.4) then
  X(1)=-0.861136311594053D0
  X(2)=-0.339981043584856D0
  X(3)=-X(2)
  X(4)=-X(1)
  W(1)=0.347854845137454D0
  W(2)=0.652145154862546D0
  W(3)=W(2)
  W(4)=W(1)
end if

if(n.eq.5) then
  X(1)=-0.906179845938664D0
  X(2)=-0.538469310105683D0
  X(3)=0.000000000000D0
  X(4)=-X(2)
  X(5)=-X(1)
  W(1)=0.236926885056189D0
  W(2)=0.478628670499366D0
  W(3)=0.568888888888889D0
  W(4)=W(2)
  W(5)=W(1)
end if

if(n.eq.6) then
  X(1)=-0.932469514283197d0
  X(2)=-0.661209386466265d0
  X(3)=-0.238619186083197d0
  X(4)= 0.238619186083197d0
  X(5)= 0.661209386466265d0
  X(6)= 0.932469514283197d0
  W(1)= 0.17132449237970d0
  W(2)= 0.360761573048139d0
  W(3)= 0.467913934572691d0

```

```

W(4)= 0.467913934572691d0
W(5)= 0.360761573048139d0
W(6)= 0.17132449237970d0
end if

```

```

RETURN
END

```

```

subroutine initl(nelem,x0,xnodel,ttrg,omegag)
IMPLICIT REAL*8 (A-H,O-Z)
dimension x0(3,4,*),xnodel(3,4,*),
dimension ttrg(3,3,6,*),omegag(3,6,*),
ngpt=6
do idelem = 1,nelem
  do ig = 1,ngpt
    do ii = 1,3

      do jj = 1,3
        ttrg(ii,jj,ig,idelem) = 0.d0
      end do
      ttrg(ii,ii,ig,idelem) = 1.d0

      omegag(ii,ig,idelem) = 0.d0

    end do
  end do

  do nod = 1,4
    do ii = 1,3
      xnodel(ii,nod,idelem) = x0(ii,nod,idelem)
    end do
  end do

end do

return
end

```

```

c
c
c
c

```

```

subroutine tempsv(maxeqn,nelem,
&                xnodel,ttrg,omegag,da,dq,
&                sxnode,sttrg,somegg,sda,sdq)
IMPLICIT REAL*8 (A-H,O-Z)
dimension xnodel(3,4,*),
dimension sxnode(3,4,*),
dimension ttrg(3,3,6,*),omegag(3,6,*),
dimension sttrg(3,3,6,*),somegg(3,6,*),
dimension da(*),dq(*),sda(*),sdq(*)
ngpt=6

```

```

do idelem = 1,nelem
  do ig = 1,ngpt
    do ii = 1,3
      do jj = 1,3
        sttrg(ii,jj,ig,idelem)=ttrg(ii,jj,ig,idelem)
      end do
      somegg(ii,ig,idelem)=omegag(ii,ig,idelem)
    end do
  end do

  do nod = 1,4
    do ii = 1,3
      sxnode(ii,nod,idelem) = xnodel(ii,nod,idelem)
    end do
  end do

end do

do ii = 1,maxeqn
  sda(ii)=da(ii)
  sdq(ii)=dq(ii)
end do
c OPEN (UNIT=,FILE='archcode.in',STATUS='UNKNOWN')

return
end
subroutine tempgt(maxeqn,nelem,
&                xnodel,ttrg,omegag,da,dq,
&                sxnode,sttrg,somegg,sda,sdq)
IMPLICIT REAL*8 (A-H,O-Z)
dimension xnodel(3,4,*)
dimension sxnode(3,4,*)
dimension ttrg(3,3,6,*),omegag(3,6,*)
dimension sttrg(3,3,6,*),somegg(3,6,*)
dimension da(*),dq(*),sda(*),sdq(*)
ngpt=6
do idelem = 1,nelem
  do ig = 1,ngpt
    do ii = 1,3
      do jj = 1,3
        ttrg(ii,jj,ig,idelem)=sttrg(ii,jj,ig,idelem)
      end do
      omeagag(ii,ig,idelem)=somegg(ii,ig,idelem)
    end do
  end do

  do nod = 1,4
    do ii = 1,3
      xnodel(ii,nod,idelem)=sxnode(ii,nod,idelem)
    end do
  end do

end do

do ii = 1,maxeqn
  da(ii)=sda(ii)

```

```

    dq(ii)=sdq(ii)
end do

return
end

```

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2

```

SUBROUTINE ASSEMB(NTNODE,NELM,melm,x0,xnodel,cncm,NODBC,MAXEQN,
&             nhing1,nhing3,
&             mhing1,mhing3,
&             hing1,idofh1,
&             distgg,distnn,distpp,ptload,alambd,
&             ttrg,t6rg,omegag,om6gag,da,
&             LM,MAXH,MHT,NWK,MK,
&             AST,qint,qext0,LCODE)
IMPLICIT REAL*8 (A-H,O-Z)

DIMENSION x0(3,4,*),xnodel(3,4,*),cncm(6,4,*)
DIMENSION MELM(NELM,4),NODBC(NTNODE,6)
DIMENSION mhing1(2,*),mhing3(2,*)
DIMENSION hing1(5,*),idofh1(*)
DIMENSION LM(24),MAXH(MAXEQN),MHT(MAXEQN)
dimension ttrg(3,3,6,*),omegag(3,6,*),da(*)
dimension t6rg(3,3,6,*),om6gag(3,6,*)
DIMENSION AST(MAXEQN,MAXEQN)
DIMENSION MAXA(24),STIFEL(24,24)
dimension qext0(*),qint(*),qexte0(24),qinte(24)
dimension dae(24)
dimension distgg(3,4,*),distnn(3,4,*),distpp(3,4,*),ptload(*)
dimension distrg(3,4),distrn(3,4),distrp(3,4)
DIMENSION HPQ(3),SPQA(6,6),SPQC(6,6),WK(6,6),HPQsk(3,3)

MAX = MAXEQN+1
if(LCODE.EQ.2) then
  do ii = 1, MAX
    qext0(ii) = ptload(ii) !
    qint(ii) = 0.0
  end do

end if

DO IDELEM=1,NELM

  if(LCODE.EQ.1) then ! assemble stiffness matrix only

    call getnld(idelem,distnn,distrn,alambd)
    call getpld(idelem,distpp,distrp,alambd)

    call elstif(idelem,x0,xnodel,ttrg,omegag,CNCM,
&             t6rg,om6gag,distrn,distrp,stifel)
    ipr=2
    if(ipr.eq.1) then
      open(20,file='stiff.dat',status='unknown')

```

```

        nepr=nelm/2+1
        if(idelem.eq.nepr) then
            write(20,*) ((stifel(ii,jj),ii=1,24),jj=1,24)
        end if
    end if
end if ! end of assembling stiffness matrix only

DO IT=1,24
    LM(IT)=0
END DO

IT=0
DO INOD=1,4
    NODNUM=MELM(IDELEM,INOD)
    DO IDOF=1,6
        IT=IT+1
        LM(IT)=NODBC(NODNUM,IDOF)
    END DO
END DO
    if(LCODE.EQ.2) then ! assemble load vectors only
        call clvec(dae,24) !get element node displ
increment

        do nod=1,4
            NODE=MELM(IDELEM,NOD)
            DO IDOF=1,6
                NDOF=NODBC(NODE,IDOF)
                dae(6*(NOD-1)+IDOF)=0.d0
                IF(NDOF.NE.0) THEN
                    dae(6*(NOD-1)+IDOF)=da(NDOF)
                END IF
            END DO
        END DO !get element node displ increment
c        write(6,*) 'dae=',dae

        call qintel(idelem,x0,xnode1,t6rg,om6gag,
&                CNCM,qinte)
        almbd=1.
        call getgld(idelem,distgg,distrg,almbd)
        call getnld(idelem,distnn,distrn,almbd)
        call getpld(idelem,distpp,distrp,almbd)

&        call qextel(idelem,x0,xnode1,
                    distrg,distrn,distrp,ttrg,qexte0)

        end if ! end of assembling load vectors only
DO IR=1,24

    IST=LM(IR)
    IF(IST.NE.0) THEN
        if(LCODE.EQ.1) then ! assemble stiffness matrix only
            DO JC=1,24
                IF(LM(JC).NE.0) THEN
                    JST=LM(JC)

                    AST(IST,JST)=AST(IST,JST)+STIFEL(IR,JC)
                END IF
            END DO
        end if
    end if

```

```

        END DO
        end if ! end of assembling stiffness matrix only
        if(LCODE.EQ.2) then ! assemble load vectors only

            qext0(ist) = qext0(ist) + qexte0(ir)
            qint(ist) = qint(ist) + qinte(ir)

        end if ! end of assembling load vectors only

    END IF
END DO

END DO

if(LCODE.EQ.1) then
c write(22,*) 'ast0=', (ast(ii,ii), ii=1, maxeqn)
DO nhing =1, nhing1
    do i=1,3
        HPQ(i)=hing1(i,nhing)
    end do
    IP=mhing1(1,nhing)
    IQ=mhing1(2,nhing)
c write(6,*) 'HPQ=',HPQ
    call vskew(HPQ,HPQsk)
    do i = 1,3
        IR=NODBC(IP,3+i)

        if(IR.ne.0) then
            do j=1,3
                IC=NODBC(IP,3+j)
                if(IC.ne.0) then
                    AST(IR,IC)=AST(IR,IC)+
& hing1(4,nhing)*(hing1(5,nhing)*HPQsk(i,j)+
& HPQ(i)*HPQ(j))

                    end if
                    IC=NODBC(IQ,3+j)
                    if(IC.ne.0) then
                        AST(IR,IC)=AST(IR,IC)-hing1(4,nhing)*HPQ(i)*HPQ(j)
                    end if

                end do
            end if
        end do

    do i = 1,3
        IR=NODBC(IQ,3+i)
        if(IR.ne.0) then
            do j=1,3
                IC=NODBC(IP,3+j)
                if(IC.ne.0) then
                    AST(IR,IC)=AST(IR,IC)-hing1(4,nhing)*HPQ(i)*HPQ(j)
                end if
                IC=NODBC(IQ,3+j)
                if(IC.ne.0) then

```

```

        AST(IR,IC)=AST(IR,IC)+
&         hingl(4,nhing)*(hingl(5,nhing)*HPQsk(i,j)+
&         HPQ(i)*HPQ(j))
        end if

        end do
        end if
    end do

    call HING1M(HPQ,SPQA,SPQC,iching)
c   write(6,*) 'SPQA=',SPQA
c   write(6,*) 'SPQC=',SPQC
    idofh1(nhing)=iching

    DO nod = 1,NTNODE

        do i = 1,6
            do j = 1,6
                WK(i,j)=0.
            end do
        end do

        do IDOFR = 1,6

            IR=NODBC(nod,IDOFR)
            if(IR.NE.0) then
                do IDOFC = 1,6
                    ICP=NODBC(IP,IDOFC)
                    if(ICP.NE.0) then
                        do j = 1,6
                            ICQ=NODBC(IQ,j)
                            if(ICQ.ne.0) then
                                AST(IR,ICP)=AST(IR,ICP)
&                                +AST(IR,ICQ)*SPQA(j,IDOFC)
                            end if
                        end do
                    end do
                end if
            end do

        end do

    end do

    DO nod = 1,NTNODE
        do i = 1,6
            do j = 1,6
                WK(i,j)=0.
            end do
        end do

        do IDOFR = 1,6

            IR=NODBC(nod,IDOFR)
            if(IR.NE.0) then

```



```

do IDOFC = 1,6
  ICQ=NODBC(IQ,IDOFC)
  if(ICQ.NE.0) then
    do j = 1,6
      jj = NODBC(IQ,j)
      if(jj.ne.0) then
        WK(IDOFR,IDOFC) = WK(IDOFR,IDOFC)+
&          AST(IR,jj)*SPQC(j,IDOFC)
      end if
    end do
  end if
end do

do IDOFR = 1,6

  IR=NODBC(nod,IDOFR)
  if(IR.NE.0) then
    do IDOFC = 1,6
      ICQ=NODBC(IQ,IDOFC)
      if(ICQ.NE.0) then
        AST(IR,ICQ) = WK(IDOFR,IDOFC)
      end if
    end do
  end if

end do

end do

DO nod = 1,NTNODE
  do IDOFC = 1,6
    IC=NODBC(nod,IDOFC)
    if(IC.NE.0) then
      do IDOFR = 1,6
        IRP=NODBC(IP,IDOFR)
        IRQ=NODBC(IQ,IDOFR)
        if(IRP.NE.0) then
          if(IRQ.NE.0) then
&            AST(IRP,IC)=AST(IRP,IC)
&              +AST(IRQ,IC)
          end if
        end if
      end do
    end if
  end do
end do

DO IDOFR=1,3
  IRQ=NODBC(IQ,IDOFR)
  IRP=NODBC(IP,IDOFR)
  if(IRQ.ne.0.) then

    DO nod = 1,NTNODE
      DO IDOFC=1,6

```

```

        IC=NODBC(nod,IDOFC)
        if(IC.ne.0) then
            AST(IRQ,IC)=0.
            if(IRQ.eq.IC) then
                AST(IRQ,IC)=1.
            end if
        end if
    end do
end do

end if

end do
cccccccccccc
DO IDOFR=1,3
    IRQ=NODBC(IQ,IDOFR)
    IRP=NODBC(IP,IDOFR)
    if(IRP.ne.0.) then
        if(IRQ.eq.0.) then

            DO nod = 1,NTNODE
                DO IDOFC=1,6
                    IC=NODBC(nod,IDOFC)
                    if(IC.ne.0) then
                        AST(IRP,IC)=0.
                        if(IRP.eq.IC) then
                            AST(IRP,IC)=1.
                        end if
                    end if
                end do
            end do

        end if
    end if

end do

end do
cccccccccccc
DO IDOFR=1,3
    IR=NODBC(IQ,IDOFR+3)
    if(IR.ne.0) then
        DO nod = 1,NTNODE
            DO IDOFC=1,6
                IC=NODBC(nod,IDOFC)
                if(IC.ne.0) then
                    wk1=0.
                    do k = 1,3
                        kr = NODBC(IQ,k+3)
                        if(kr.ne.0) then
                            wk1 = wk1 + HPQ(IDOFR)*HPQ(k)*AST(kr,IC)
                        end if
                    end do
                    AST(IR,IC) = wk1
                end if
            end do
        end do
    end do
end if

```

```

end do

DO nod = 1,NTNODE
  do IDOFC = 1,6
    IC=NODBC(nod,IDOFC)
    if(IC.NE.0) then
      do IDOFR = 4,6
        IRP=NODBC(IP,IDOFR)
        IRQ=NODBC(IQ,IDOFR)
        if(IRP.NE.0) then
          if(IRQ.NE.0) then
            AST(IRP,IC)=AST(IRP,IC)
            & -AST(IRQ,IC)
          end if
        end if
      end do
    end if
  end do
end do

DO IDOFR=1,3
  IRQ=NODBC(IQ,IDOFR+3)
  IRP=NODBC(IP,IDOFR+3)
  if(IRQ.ne.0) then
    DO nod = 1,NTNODE
      DO IDOFC=1,6
        IC=NODBC(nod,IDOFC)
        if(IC.ne.0) then
          if(IDOFR.ne.iching) then
            if(IC.eq.IRQ) then
              AST(IRQ,IC) = 1.
            end if
          end if
        end if
      end do
    end do
  end if
end do
cccccccccc
DO IDOFR=1,3
  IRQ=NODBC(IQ,IDOFR+3)
  IRP=NODBC(IP,IDOFR+3)
  if(IRP.ne.0) then
    if(IRQ.eq.0) then
      DO nod = 1,NTNODE
        DO IDOFC=1,6
          IC=NODBC(nod,IDOFC)
          if(IC.ne.0) then
            if(IDOFR.ne.iching) then
              if(IC.eq.IRP) then
                AST(IRP,IC) = 1.
              end if
            end if
          end if
        end do
      end do
    end if
  end if
end do

```

```

        end if
    end do
ccccccccccc

    end do
c   write(22,*) 'ast1=',(ast(ii,ii), ii=1, maxeqn)
    end if

    if(LCODE.EQ.2) then
c   write(6,*) 'qext3=',(qext0(iii),iii=1,maxeqn)
c   write(6,*) 'qint3=',(qint(iii),iii=1,maxeqn)
    do nhing =1, nhing1

        do i=1,3
            HPQ(i)=hing1(i,nhing)
        end do

        IP=mhing1(1,nhing)
        IQ=mhing1(2,nhing)
c   write(6,*) 'spr=', hing1(4,nhing), 'thet=',hing1(5,nhing)
        do i=1,3
            IR=NODBC(IP,3+i)
            wk1 =0.
            if(IR.ne.0) then

                qint(IR)=qint(IR)+hing1(4,nhing)*hing1(5,nhing)*HPQ(i)
            end if
        end do

        do i=1,3
            IR=NODBC(IQ,3+i)
            if(IR.ne.0) then
                qint(IR)=qint(IR)-hing1(4,nhing)*hing1(5,nhing)*HPQ(i)
            end if
        end do
c   write(6,*) 'qext4=',(qext0(iii),iii=1,maxeqn)
c   write(6,*) 'qint4=',(qint(iii),iii=1,maxeqn)
        do i = 1,6
            IRP = NODBC(IP,i)
            IRQ = NODBC(IQ,i)
            if(IRP.NE.0) then
                if(IRQ.NE.0) then
                    qext0(IRP) = qext0(IRP)+qext0(IRQ)
                    qint(IRP) = qint(IRP)+qint(IRQ)
                end if
            end if
        end do

        do i = 1,3
            IRQ = NODBC(IQ,i)
            if(IRQ.NE.0) then
                qext0(IRQ) = 0.
                qint(IRQ) = 0.
            end if
        end do
cccccccc

        do i = 1,3

```

```

        IRQ = NODBC(IQ,i)
        IRP = NODBC(IP,i)
        if(IRP.NE.0) then

            if(IRQ.EQ.0) then
                qext0(IRP) = 0.
                qint(IRP) = 0.
            end if

        end if
    end do
cccccccc
do i = 1,3
    IR = NODBC(IQ,i+3)
    if(IR.NE.0) then
        wk1=0.
        wk2=0.
        do k = 1, 3
            kr = NODBC(IQ,k+3)
            if(kr.ne.0) then
                wk1 = wk1 + HPQ(i)*HPQ(k)*qext0(kr)
                wk2 = wk2 + HPQ(i)*HPQ(k)*qint(kr)
            end if
        end do
c        write(6,*) 'wk1,wk2 =', wk1,wk2
        qext0(IR) = wk1
        qint(IR) = wk2
    end if
end do

do i = 1,3
    IRP = NODBC(IP,i+3)
    IRQ = NODBC(IQ,i+3)
    if(IRP.NE.0) then
        if(IRQ.NE.0) then
            qext0(IRP) = qext0(IRP)-qext0(IRQ)
            qint(IRP) = qint(IRP)-qint(IRQ)
        end if
    end if
end do
cccccccc
do i = 1,3
    IRP = NODBC(IP,i+3)
    IRQ = NODBC(IQ,i+3)
    if(IRP.NE.0) then
        if(IRQ.EQ.0) then
            qext0(IRP) = 0.
            qint(IRP) = 0.
        end if
    end if
end do
cccccccc
c    write(6,*) 'qext5=',(qext0(iii),iii=1,maxeqn)
c    write(6,*) 'qint5=',(qint(iii),iii=1,maxeqn)
end do
end if

```

```
RETURN
END
```

```
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```
      SUBROUTINE ASSEMN(NTNODE,NELM,melm,x0,xnodel,cncm,NODBC,MAXEQN,
&                    nhing1,nhing3,
&                    mhing1,mhing3,
&                    hing1,idofh1,
&                    distgg,distnn,distpp,ptload,alambd,
&                    ttrg,t6rg,omegag,om6gag,da,
&                    LM,MAXH,MHT,NWK,MK,
&                    AST,qint,qext0,LCODE)
      IMPLICIT REAL*8 (A-H,O-Z)

      DIMENSION x0(3,4,*),xnodel(3,4*),cncm(6,4,*)
      DIMENSION MELM(NELM,4),NODBC(NTNODE,6)
      DIMENSION mhing1(2,*),mhing3(2,*)
      DIMENSION hing1(5,*),idofh1(*)
      DIMENSION LM(24),MAXH(MAXEQN),MHT(MAXEQN)
      dimension ttrg(3,3,6,*),omegag(3,6,*),da(*)
      dimension t6rg(3,3,6,*),om6gag(3,6,*)
      DIMENSION AST(MAXEQN,MAXEQN)
      DIMENSION MAXA(24),STIFEL(24,24)
      dimension qext0(*),qint(*),qexte0(24),qinte(24)
      dimension dae(24)
      dimension distgg(3,4*),distnn(3,4*),distpp(3,4*),ptload(*)
      dimension distrg(3,4),distrn(3,4),distrp(3,4)
      DIMENSION HPQ(3),SPQA(6,6),SPQC(6,6),WK(6,6),HPQsk(3,3)

      MAX = MAXEQN+1

      DO IDELEM=1,NELM

         if(LCODE.EQ.1) then ! assemble stiffness matrix only
            call getnld(idelem,distnn,distrn,alambd)
            call getpld(idelem,distpp,distrp,alambd)
            call elstin(idelem,x0,xnodel,ttrg,omegag,CNCM,
&                    t6rg,om6gag,distrn,distrp,stifel)

         end if ! end of assembling stiffness matrix only

         DO IT=1,24
            LM(IT)=0
         END DO

         IT=0
         DO INOD=1,4
            NODNUM=MELM(IDELEM,INOD)
            DO IDOF=1,6
               IT=IT+1
               LM(IT)=NODBC(NODNUM,IDOF)
            END DO
         END DO
      END DO
```

```

        END DO
    END DO

    DO IR=1,24

        IST=LM(IR)
        IF(IST.NE.0) THEN
            if(LCODE.EQ.1) then      ! assemble stiffness matrix only
                DO JC=1,24
                    IF(LM(JC).NE.0) THEN
                        JST=LM(JC)

                        AST(IST,JST)=AST(IST,JST)+STIFEL(IR,JC)
                    END IF
                END DO
            end if      ! end of assembling stiffness matrix only

        END IF
    END DO

END DO

if(LCODE.EQ.1) then
c   write(22,*) 'ast0=', (ast(ii,ii), ii=1, maxeqn)
DO nhing =1, nhing1
    do i=1,3
        HPQ(i)=hing1(i,nhing)
    end do
    IP=mhing1(1,nhing)
    IQ=mhing1(2,nhing)
c   write(6,*) 'HPQ=',HPQ
    call vskew(HPQ,HPQsk)
    do i = 1,3
        IR=NODBC(IP,3+i)

        if(IR.ne.0) then
            do j=1,3
                IC=NODBC(IP,3+j)
                if(IC.ne.0) then
c   AST(IR,IC)=AST(IR,IC)+
c   &   hing1(4,nhing)*(hing1(5,nhing)*HPQsk(i,j)+
c   &   HPQ(i)*HPQ(j))
                AST(IR,IC)=AST(IR,IC)+
&   hing1(4,nhing)*(hing1(5,nhing)*HPQsk(i,j)+
&   0.)
                end if
                IC=NODBC(IQ,3+j)
                if(IC.ne.0) then
c   AST(IR,IC)=AST(IR,IC)-hing1(4,nhing)*HPQ(i)*HPQ(j)
                AST(IR,IC)=AST(IR,IC)-0.
                end if
            end do
        end do
    end if
end do

```

```

do i = 1,3
  IR=NODBC(IQ,3+i)
  if(IR.ne.0) then
    do j=1,3
      IC=NODBC(IP,3+j)
      if(IC.ne.0) then
c        AST(IR,IC)=AST(IR,IC)-hing1(4,nhing)*HPQ(i)*HPQ(j)
c        AST(IR,IC)=AST(IR,IC)-0.
      end if
      IC=NODBC(IQ,3+j)
      if(IC.ne.0) then
c        AST(IR,IC)=AST(IR,IC)+
c        &      hing1(4,nhing)*(hing1(5,nhing)*HPQsk(i,j)+
c        &      HPQ(i)*HPQ(j))
c        AST(IR,IC)=AST(IR,IC)+
&      hing1(4,nhing)*(hing1(5,nhing)*HPQsk(i,j)+
&      0.)
      end if

    end do
  end if
end do

call HING1M(HPQ,SPQA,SPQC,iching)
c write(6,*) 'SPQA=',SPQA
c write(6,*) 'SPQC=',SPQC
idofhl(nhing)=iching

DO nod = 1,NTNODE

  do i = 1,6
    do j = 1,6
      WK(i,j)=0.
    end do
  end do

  do IDOFR = 1,6

    IR=NODBC(nod,IDOFR)
    if(IR.NE.0) then
      do IDOFC =1,6
        ICP=NODBC(IP,IDOFC)
        if(ICP.NE.0) then
          do j = 1,6
            ICQ=NODBC(IQ,j)
            if(ICQ.ne.0) then
c              AST(IR,ICP)=AST(IR,ICP)
&              +AST(IR,ICQ)*SPQA(j,IDOFC)
            end if
          end do
        end if
      end do
    end if
  end do

end do

```



```

end do

DO nod = 1,NTNODE
  do i = 1,6
    do j = 1,6
      WK(i,j)=0.
    end do
  end do

  do IDOFR = 1,6

    IR=NODBC(nod,IDOFR)
    if(IR.NE.0) then
      do IDOFC = 1,6
        ICQ=NODBC(IQ,IDOFC)
        if(ICQ.NE.0) then
          do j = 1,6
            jj = NODBC(IQ,j)
            if(jj.ne.0) then
              WK(IDOFR,IDOFC) = WK(IDOFR,IDOFC)+
&                AST(IR,jj)*SPQC(j,IDOFC)
            end if
          end do
        end if
      end do
    end if
  end do

  do IDOFR = 1,6

    IR=NODBC(nod,IDOFR)
    if(IR.NE.0) then
      do IDOFC = 1,6
        ICQ=NODBC(IQ,IDOFC)
        if(ICQ.NE.0) then
          AST(IR,ICQ) = WK(IDOFR,IDOFC)
        end if
      end do
    end if
  end do

end do

DO nod = 1,NTNODE
  do IDOFC = 1,6
    IC=NODBC(nod,IDOFC)
    if(IC.NE.0) then
      do IDOFR = 1,6
        IRP=NODBC(IP,IDOFR)
        IRQ=NODBC(IQ,IDOFR)
        if(IRP.NE.0) then
          if(IRQ.NE.0) then
            AST(IRP,IC)=AST(IRP,IC)
          end if
        end if
      end do
    end if
  end do
end do

```

```

&
                                +AST(IRQ,IC)
                                end if
                                end if
                                end do
                                end if
                                end do
                                end do

DO IDOFR=1,3
  IRQ=NODBC(IQ,IDOFR)
  IRP=NODBC(IP,IDOFR)
  if(IRQ.ne.0.) then

    DO nod = 1,NTNODE
      DO IDOFC=1,6
        IC=NODBC(nod,IDOFC)
        if(IC.ne.0) then
          AST(IRQ,IC)=0.
          if(IRQ.eq.IC) then
            AST(IRQ,IC)=1.
          end if
        end if
      end do
    end do

    end if

  end do
cccccccccccc
DO IDOFR=1,3
  IRQ=NODBC(IQ,IDOFR)
  IRP=NODBC(IP,IDOFR)
  if(IRP.ne.0.) then
    if(IRQ.eq.0.) then

      DO nod = 1,NTNODE
        DO IDOFC=1,6
          IC=NODBC(nod,IDOFC)
          if(IC.ne.0) then
            AST(IRP,IC)=0.
            if(IRP.eq.IC) then
              AST(IRP,IC)=1.
            end if
          end if
        end do
      end do

      end if
      end if

    end do
cccccccccccc
DO IDOFR=1,3
  IR=NODBC(IQ,IDOFR+3)
  if(IR.ne.0) then
    DO nod = 1,NTNODE

```

```

DO IDOFC=1,6
  IC=NODBC(nod,IDOFC)
  if(IC.ne.0) then
    wk1=0.
    do k = 1,3
      kr = NODBC(IQ,k+3)
      if(kr.ne.0) then
        wk1 = wk1 + HPQ(IDOFR)*HPQ(k)*AST(kr,IC)
      end if
    end do
    AST(IR,IC) = wk1
  end if
end do

DO nod = 1,NTNODE
  do IDOFC = 1,6
    IC=NODBC(nod,IDOFC)
    if(IC.NE.0) then
      do IDOFR = 4,6
        IRP=NODBC(IP,IDOFR)
        IRQ=NODBC(IQ,IDOFR)
        if(IRP.NE.0) then
          if(IRQ.NE.0) then
            AST(IRP,IC)=AST(IRP,IC)
            & -AST(IRQ,IC)
          end if
        end if
      end do
    end if
  end do

DO IDOFR=1,3
  IRQ=NODBC(IQ,IDOFR+3)
  IRP=NODBC(IP,IDOFR+3)
  if(IRQ.ne.0) then
    DO nod = 1,NTNODE
      DO IDOFC=1,6
        IC=NODBC(nod,IDOFC)
        if(IC.ne.0) then
          if(IDOFR.ne.iching) then
            if(IC.eq.IRQ) then
              AST(IRQ,IC) = 1.
            end if
          end if
        end if
      end do
    end do
  end if
end do
cccccccccc
DO IDOFR=1,3
  IRQ=NODBC(IQ,IDOFR+3)
  IRP=NODBC(IP,IDOFR+3)

```

```

        if(IRP.ne.0) then
        if(IRQ.eq.0) then
            DO nod = 1,NTNODE
                DO IDOFC=1,6
                    IC=NODBC(nod,IDOFC)
                    if(IC.ne.0) then
                        if(IDOFR.ne.iching) then
                            if(IC.eq.IRP) then
                                AST(IRP,IC) = 1.
                            end if
                        end if
                    end if
                end do
            end do
        end do
        end if
        end if
        end do
cccccccccccc

        end do
c      write(22,*) 'ast1=',(ast(ii,ii), ii=1, maxeqn)
        end if

        RETURN
        END

C
SUBROUTINE LUDCMP(A,N,NP,INDX,D)
    IMPLICIT REAL*8 (A-H,O-Z)
    DIMENSION A(NP,NP),INDX(N),VV(10000)
C
    TINY=1.D-20
    D=1.D0
    DO 12 I=1,N
        AAMAX=0.D0
        DO 11 J=1,N
            IF(DABS(A(I,J)).GT.AAMAX) AAMAX=DABS(A(I,J))
11      CONTINUE
        IF(DABS(AAMAX).LT.TINY/10.D0) THEN
100     WRITE(6,100)
            FORMAT(1X,'SINGULAR MATRIX')
            STOP
        ENDIF
        VV(I)=1.D0/AAMAX
12     CONTINUE
        DO 19 J=1,N
            IF(J.GT.1) THEN
                DO 14 I=1,J-1
                    SUM=A(I,J)
                    IF(I.GT.1) THEN
                        DO 13 K=1,I-1
                            SUM=SUM-A(I,K)*A(K,J)
13      CONTINUE

```

```

        A(I,J)=SUM
        ENDIF
14    CONTINUE
    ENDIF
    AAMAX=0.D0
    DO 16 I=J,N
        SUM=A(I,J)
        IF(J.GT.1) THEN
            DO 15 K=1,J-1
                SUM=SUM-A(I,K)*A(K,J)
15        CONTINUE
            A(I,J)=SUM
        ENDIF
        DUM=VV(I)*DABS(SUM)
        IF(DUM.GE.AAMAX) THEN
            IMAX=I
            AAMAX=DUM
        ENDIF
16    CONTINUE
        IF(J.NE.IMAX) THEN
            DO 17 K=1,N
                DUM=A(IMAX,K)
                A(IMAX,K)=A(J,K)
                A(J,K)=DUM
17        CONTINUE
            D=-D
            VV(IMAX)=VV(J)
        ENDIF
        INDX(J)=IMAX
        IF(J.NE.N) THEN
            IF(DABS(A(J,J)).LT.TINY) A(J,J)=TINY
            DUM=1.D0/A(J,J)
            DO 18 I=J+1,N
                A(I,J)=A(I,J)*DUM
18        CONTINUE
        ENDIF
19    CONTINUE
        IF(DABS(A(N,N)).LT.TINY) A(N,N)=TINY
        RETURN
    END

```

C

```

SUBROUTINE LUBKSB(A,N,NP,INDX,B)
    IMPLICIT REAL*8 (A-H,O-Z)
    DIMENSION A(NP,NP),INDX(N),B(N)

```

C

```

    II=0
    DO 12 I=1,N
        LL=INDX(I)
        SUM=B(LL)
        B(LL)=B(I)
        IF(II.NE.0) THEN
            DO 11 J=II,I-1
                SUM=SUM-A(I,J)*B(J)
11        CONTINUE
            ELSE IF(SUM.NE.0.D0) THEN
                II=I
            ENDIF

```

```

        B(I)=SUM
12 CONTINUE
    DO 14 I=N,1,-1
        SUM=B(I)
        IF(I.LT.N) THEN
            DO 13 J=I+1,N
                SUM=SUM-A(I,J)*B(J)
13         CONTINUE
            ENDIF
            B(I)=SUM/A(I,I)
14 CONTINUE
    RETURN
    END

C
    SUBROUTINE INVMAT(A,Y,N,NP)
C
C   PROGRAM FROM NUMERICAL RECIPES BY PRESS, WILLIAM H. ET. AL.
C
        IMPLICIT REAL*8 (A-H,O-Z)
        DIMENSION A(NP,NP),Y(NP,NP),INDX(10000)
C
        DO 12 J=1,N
            DO 11 I=1,N
                Y(I,J)=0.D0
11         CONTINUE
            Y(J,J)=1.D0
12 CONTINUE
        CALL LUDCMP(A,N,NP,INDX,D)
        DO 13 J=1,N
            CALL LUBKSB(A,N,NP,INDX,Y(1,J))
13 CONTINUE
    RETURN
    END

        SUBROUTINE CLVEC(A,N)
C
C
        IMPLICIT REAL*8 (A-H,O-Z)
        DIMENSION A(N)
        DO 1 I=1,N
            A(I)=0.0D0
1         CONTINUE
    RETURN
    END

C
CC
        SUBROUTINE VECVEC(A,B,C,M)
        IMPLICIT REAL*8 (A-H,O-Z)
        DIMENSION A(M),B(M)
C
        C=0.D0
        DO 1 I=1,M
            C=C+A(I)*B(I)
1         CONTINUE
    RETURN
    END
C

```

```

C
C
SUBROUTINE COLSOL(A,V,MAXA,NN,NWK,NNM,KKK)
C
C
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION A(NWK),V(1),MAXA(1)
IF(KKK-2)40,150,150
40 DO 140 N=1,NN
KN=MAXA(N)
KL=KN+1
KU=MAXA(N+1)-1
KH=KU-KL
IF(KH)110,90,50
50 K=N-KH
IC=0
KLT=KU
DO 80 J=1,KH
IC=IC+1
KLT=KLT-1
KI=MAXA(K)
ND=MAXA(K+1)-KI-1
IF(ND)80,80,60
60 KK=MIN0(IC,ND)
C=0.0D0
DO 70 L=1,KK
70 C=C+A(KI+L)*A(KLT+L)
A(KLT)=A(KLT)-C
80 K=K+1
90 K=N
B=0.0D0
DO 100 KK=KL,KU
K=K-1
KI=MAXA(K)
C=A(KK)/A(KI)
B=B+C*A(KK)
100 A(KK)=C
A(KN)=A(KN)-B
110 IF(A(KN))120,120,140
c 120 PRINT 2000,N,A(KN)
120 continue
c STOP
140 CONTINUE
RETURN
C
C REDUCE RIGHT HAND SIDE LOAD VECTER
C
150 DO 180 N=1,NN
KL=MAXA(N)+1
KU=MAXA(N+1)-1
IF(KU-KL)180,160,160
160 K=N
C=0.0D0
DO 170 KK=KL,KU
K=K-1
170 C=C+A(KK)*V(K)
V(N)=V(N)-C

```

```

180     CONTINUE
C
C
C
      DO 200 N=1,NN
      K=MAXA(N)
200     V(N)=V(N)/A(K)
      IF(NN.EQ.1)RETURN
      N=NN
      DO 230 L=2,NN
      KL=MAXA(N)+1
      KU=MAXA(N+1)-1
      IF(KU-KL)230,210,210
210     K=N
      DO 220 KK=KL,KU
      K=K-1
220     V(K)=V(K)-A(KK)*V(N)
230     N=N-1
      RETURN
2000    FORMAT(/10X,'STOP--MATRIX NOT POSITIVE DEFINITE',/10X,
&'NON POSITIVE PIVOT FOR EQUATION',I5/10X,'PIVOT=',E20.12)
      END
C
C
C
      SUBROUTINE DECOMP1(NDIM,N,A,COND,IPVT,NCHG,WORK)
      IMPLICIT REAL*8 (A-H, O-Z)
      DIMENSION A(NDIM,N), WORK(N), IPVT(N)
      nchg=0
      IPVT(N) = 1
      IF(N.EQ.1) GO TO 80
      NM1 = N-1
      ANORM = 0.0
      DO 10 J=1,N
      T=0.0
      DO 5 I = 1,N
      T = T +DABS(A(I,J))
5      CONTINUE
      IF(T.GT.ANORM) ANORM = T
10     CONTINUE
      DO 35 K = 1,NM1
      KP1 = K+1
      M=K
      DO 15 I=KP1,N
      IF(DABS(A(I,K)).GT.DABS(A(M,K))) M=I
15     CONTINUE
      IPVT(K) = M
      IF(M.NE.K) IPVT(N) = -IPVT(N)
      IF(M.NE.K) NCHG=NCHG+1
      T=A(M,K)
      A(M,K) =A(K,K)
      A(K,K) = T
      IF(T.EQ.0.0) GO TO 35
      DO 20 I=KP1, N
      A(I,K) = -A(I,K)/T
20     CONTINUE
      DO 30 J = KP1,N

```



```

T = A(M,J)
A(M,J) = A(K,J)
A(K,J) = T
IF(T.EQ.0.0) GO TO 30
DO 25 I = KP1, N
A(I,J) = A(I,J) + A(I,K)*T
25 CONTINUE
30 CONTINUE
35 CONTINUE
DO 50 K = 1,N
T = 0.0
IF(K.EQ.1) GO TO 45
KM1 = K-1
DO 40 I = 1,KM1
T = T + A(I,K)*WORK(I)
40 CONTINUE
45 EK = 1.0
IF(T.LT.0.0) EK = - 1.0
IF(A(K,K).EQ.0.0) GO TO 90
WORK(K) = -(EK+T)/A(K,K)
50 CONTINUE
DO 60 KB = 1,NM1
K = N - KB
T = 0.0
KP1 = K + 1
DO 55 I = KP1, N
T = T + A(I,K)*WORK(K)
55 CONTINUE
WORK(K) = T
M = IPVT(K)
IF(M.EQ.K) GO TO 60
T = WORK(M)
WORK(M) = WORK(K)
WORK(K) = T
60 CONTINUE
YNORM = 0.0
DO 65 I = 1,N
YNORM = YNORM + DABS(WORK(I))
65 CONTINUE
CALL SOLVE1(NDIM,N,A,WORK,IPVT)
ZNORM = 0.0
DO 70 I = 1,N
ZNORM = ZNORM + DABS(WORK(I))
70 CONTINUE
COND = ANORM*ZNORM/YNORM
IF(COND.LT.1.0) COND = 1.0
RETURN
80 COND = 1.0
IF(A(1,1).NE.0.0) RETURN
90 COND = 1.0E+32
RETURN
END

SUBROUTINE SOLVE1(NDIM,N,A,B,IPVT)
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION A(NDIM,N), B(N), IPVT(N)
IF(N.EQ.1) GO TO 50

```

```

      NM1 = N-1
      DO 20 K = 1,NM1
      KP1 = K+1
      M = IPVT(K)
      T = B(M)
      B(M) = B(K)
      B(K) = T
      DO 10 I = KP1,N
      B(I) = B(I) + A(I,K)*T
10    CONTINUE
20    CONTINUE
      DO 40 KB = 1,NM1
      KM1 = N - KB
      K = KM1 + 1
      B(K) = B(K)/A(K,K)
      T = -B(K)
      DO 30 I = 1,KM1
      B(I) = B(I) + A(I,K)*T
30    CONTINUE
40    CONTINUE
50    B(1) = B(1)/A(1,1)
      RETURN
      END

```

```

      subroutine recdal(da,nhing1,mhing1,hing1,idofh1,NODBC,NTNODE)

```

```

C
C this subroutine is used to recover the incremental displacements
C at node Q
C
C
C hingel(1,nhing),hingel(2,nhing), and hingel(3,nhing) give
C three components of unit hinge axis vector HPQ.
C
C hingel(4,idhing) stores torsional spring constant around HPQ.
C hingel(5,idhing) stores relative rotation angle between P and
C Q (around HPQ)
C
C idofh1(nhing) = 1: |HPQ1| = max{|HPQ1|,|HPQ2|,|HPQ3|}
C                 = 2: |HPQ2| = max{|HPQ1|,|HPQ2|,|HPQ3|}
C                 = 3: |HPQ3| = max{|HPQ1|,|HPQ2|,|HPQ3|}
C
C
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION mhing1(2,*),hing1(5,*),idofh1(*),NODBC(NTNODE,6)
      DIMENSION da(*),HPQ(3),SPQA(6,6),SPQC(6,6),daP(6),daQ(6),C(6)

      do nhing=1,nhing1
      IP=mhing1(1,nhing)
      IQ=mhing1(2,nhing)
      iching=idofh1(nhing)
      do i=1,3
      HPQ(i)=hing1(i,nhing)
      end do
      call HING1M(HPQ,SPQA,SPQC,ichin1)

      if(ichin1.ne.iching) then
      stop 'hinge calculation is NOT stable!!'

```

```

end if

do i = 1,6
  daP(i)=0.
  daQ(i)=0.
  C(i)=0.
end do

do i = 1,6
  idof=NODBC(IP,i)
  if(idof.ne.0) then
    daP(i)=da(idof)
  end if
end do

do i = 1,6
  idof=NODBC(IQ,i)
  if(idof.ne.0) then
    C(i)=da(idof)
  end if
end do

do i = 1,6
  wk1=0.
  wk2=0.
  do j = 1,6
    wk1=wk1 +SPQA(i,j)*daP(j)
  end do
  do j = 1,6
    wk2=wk2+SPQC(i,j)*C(j)
  end do

  daQ(i) = wk1 +wk2
end do

do i = 1,6
  idof=NODBC(IQ,i)
  if(idof.ne.0) then
    da(idof)=daQ(i)
  end if
end do
c   write(6,*) 'SPQA=',SPQA
c   write(6,*) 'SPQC=',SPQC
c   write(6,*) 'daP=',daP
c   write(6,*) 'C=',C
c   write(6,*) 'daQ=',daQ
end do

return
end
SUBROUTINE HING1M(HPQ,SPQA,SPQC,iching)
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION HPQ(3),SPQA(6,6),SPQC(6,6),HPQW(3,3),HPQC(3,3)

DO I = 1, 6

```

```

      DO J = 1, 6
        SPQA(I,J)= 0.
        SPQC(I,J)= 0.
      END DO
    END DO
  DO I = 1,3
    SPQA(I,I)= 1.

  END DO
  if (dabs(HPQ(1)).ge.dabs(HPQ(2)).and.
&    dabs(HPQ(1)).ge.dabs(HPQ(3))) then
    iching=1
    HPQW(1,1)=0.
    HPQW(1,2)=0.
    HPQW(1,3)=0.
    HPQW(2,1)=-HPQ(2)/HPQ(1)
    HPQW(2,2)=1.
    HPQW(2,3)=0.
    HPQW(3,1)=-HPQ(3)/HPQ(1)
    HPQW(3,2)=0.
    HPQW(3,3)=1.
    HPQC(1,1)=1.
    HPQC(1,2)=0.
    HPQC(1,3)=0.
    HPQC(2,1)=HPQ(2)/HPQ(1)
    HPQC(2,2)=0.
    HPQC(2,3)=0.
    HPQC(3,1)=HPQ(3)/HPQ(1)
    HPQC(3,2)=0.
    HPQC(3,3)=0.
  end if

  if (dabs(HPQ(2)).ge.dabs(HPQ(3)).and.
&    dabs(HPQ(2)).ge.dabs(HPQ(1))) then

    iching=2
    HPQW(1,1)=1.
    HPQW(1,2)=-HPQ(1)/HPQ(2)
    HPQW(1,3)=0.
    HPQW(2,1)=0.
    HPQW(2,2)=0.
    HPQW(2,3)=0.
    HPQW(3,1)=0.
    HPQW(3,2)=-HPQ(3)/HPQ(2)
    HPQW(3,3)=1.
    HPQC(1,1)=0.
    HPQC(1,2)=HPQ(1)/HPQ(2)
    HPQC(1,3)=0.
    HPQC(2,1)=0.
    HPQC(2,2)=1.
    HPQC(2,3)=0.
    HPQC(3,1)=0.
    HPQC(3,2)=HPQ(3)/HPQ(2)
    HPQC(3,3)=0.

  end if

```

```

    if(dabs(HPQ(3)).ge.dabs(HPQ(1)).and.
&      dabs(HPQ(3)).ge.dabs(HPQ(2))) then

        iching=3
        HPQW(1,1)=1.
        HPQW(1,2)=0.
        HPQW(1,3)=-HPQ(1)/HPQ(3)
        HPQW(2,1)=0.
        HPQW(2,2)=1.
        HPQW(2,3)=-HPQ(2)/HPQ(3)
        HPQW(3,1)=0.
        HPQW(3,2)=0.
        HPQW(3,3)=0.
        HPQC(1,1)=0.
        HPQC(1,2)=0.
        HPQC(1,3)=HPQ(1)/HPQ(3)
        HPQC(2,1)=0.
        HPQC(2,2)=0.
        HPQC(2,3)=HPQ(2)/HPQ(3)
        HPQC(3,1)=0.
        HPQC(3,2)=0.
        HPQC(3,3)=1.

    end if

    DO I = 1, 3
        DO J = 1, 3
            SPQA(3+I,3+J) = HPQW(I,J)
            SPQC(3+I,3+J) = HPQC(I,J)
        END DO
    END DO

    return
end

subroutine ttthet(tt,theta)
c
c this subroutine is used to extract rotation vector "theta"
c from rotaion matrix "tt". notice that it is valid only in the
c case that the norm of theta is from zero to 2 pi.
c
c
    implicit real*8(a-h,o-z)
    dimension tt(3,3),tth(3,3),tti(3,3),q(3),theta(3)

    am = -1000.
    imax = 0
    tr=0.

    do ii = 1,3
        tr = tr + tt(ii,ii)
        if(am.lt.tt(ii,ii)) then
            am= tt(ii,ii)
            imax = ii
        end if
    end do
end do

```

```

if(am.le.tr) then
  am=tr
end if

if(am.eq.tr) then
  q0 = 0.5*dsqrt(1. + tr)
  q(1) = (tt(3,2)-tt(2,3))/(4.*q0)
  q(2) = (tt(1,3)-tt(3,1))/(4.*q0)
  q(3) = (tt(2,1)-tt(1,2))/(4.*q0)
end if

if(am.ne.tr) then
  if(imax.eq.1) then
    q(1) = dsqrt(0.5*tt(1,1)+0.25*(1.-tr))
    q0 = (tt(3,2)-tt(2,3))/(4.*q(1))
    q(2) = (tt(2,1)+tt(1,2))/(4.*q(1))
    q(3) = (tt(3,1)+tt(1,3))/(4.*q(1))
  end if
  if(imax.eq.2) then
    q(2) = dsqrt(0.5*tt(2,2)+0.25*(1.-tr))
    q0 = (tt(1,3)-tt(3,1))/(4.*q(2))
    q(1) = (tt(1,2)+tt(2,1))/(4.*q(2))
    q(3) = (tt(3,2)+tt(2,3))/(4.*q(2))
  end if
  if(imax.eq.3) then
    q(3) = dsqrt(0.5*tt(3,3)+0.25*(1.-tr))
    q0 = (tt(2,1)-tt(1,2))/(4.*q(3))
    q(1) = (tt(1,3)+tt(3,1))/(4.*q(3))
    q(2) = (tt(2,3)+tt(3,2))/(4.*q(3))
  end if
end if

  do i = 1,3
    do j = 1,3
      tti(i,j) = 0.d0
      do k = 1,3
        tti(i,j) = tti(i,j) + tt(i,k)*ttt(k,j)
      end do
    end do
  end do

c   write(6,*) 'tti', tti
c   write(6,*) 'q =', q0,q

sumq = q(1)*q(1)+q(2)*q(2)+q(3)*q(3)

atheta= 2.*dasin(dsqrt(sumq))

do ii = 1,3
  theta(ii) = 0.
end do

if(sumq.gt.1.d-15) then
  do ii = 1,3
    theta(ii) = atheta*q(ii)/dsqrt(sumq)
  end do

```

```

end if

call thettt(theta,ttt)

do i = 1,3
  do j = 1,3
    tti(i,j) = 0.d0
    do k = 1,3
      tti(i,j) = tti(i,j) + tt(i,k)*ttt(k,j)
    end do
  end do
end do

anorm = 0.
do i = 1,3
  anorm = anorm + tti(i,i)
end do

if(dabs(anorm-3.).lt.5.d-5) then
  do i = 1,3
    theta(i) = -theta(i)
  end do
end if

return
end

```

```

subroutine thettt(theta,tt)

```

```

c
c this subroutine is used to obtain rotation matrix "tt" from
c rotation vector "theta".
c

```

```

implicit real*8(a-h,o-z)
dimension tt(3,3),q(3),theta(3)

atheta = dsqrt(theta(1)*theta(1)
&           +theta(2)*theta(2)
&           +theta(3)*theta(3))

q0=dcos(atheta/2.)

do ii = 1,3
  q(ii) = 0.
end do
if(atheta.gt.1.d-15) then
  do ii = 1,3
    q(ii) = (theta(ii)/atheta)*dsin(atheta/2.)
  end do
end if

tt(1,1) = 2.*(q0*q0+q(1)*q(1))-1.
tt(2,2) = 2.*(q0*q0+q(2)*q(2))-1.
tt(3,3) = 2.*(q0*q0+q(3)*q(3))-1.

tt(1,2) = 2.*(q(1)*q(2)-q(3)*q0)
tt(1,3) = 2.*(q(1)*q(3)+q(2)*q0)

```

```

tt(2,1) = 2.*(q(2)*q(1)+q(3)*q0)
tt(2,3) = 2.*(q(2)*q(3)-q(1)*q0)

```

```

tt(3,1) = 2.*(q(3)*q(1)-q(2)*q0)
tt(3,2) = 2.*(q(3)*q(2)+q(1)*q0)

```

```

return
end

```

```

c2345678901234567890123456789012345678901234567890123456789012345678901
2

```

```

subroutine
thetah(atotl,nhing1,mhing1,hing1,idofh1,NODBC,NTNODE)
C
C this subroutine is used to the relative rotation angle between
C node P and node Q.
C
C
C hingel(1,nhing),hingel(2,nhing), and hingel(3,nhing) give
C three components of unit hinge axis vector HPQ.
C
C hingel(4,idhing) stores torsional spring constant around HPQ.
C hingel(5,idhing) stores relative rotation angle between P and
C Q (around HPQ)
C
C idofh1(nhing) = 1: |HPQ1| = max{|HPQ1|,|HPQ2|,|HPQ3|}
C                 = 2: |HPQ2| = max{|HPQ1|,|HPQ2|,|HPQ3|}
C                 = 3: |HPQ3| = max{|HPQ1|,|HPQ2|,|HPQ3|}
C
C
C

```

```

IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION mhing1(2,*),hing1(5,*),idofh1(*),NODBC(NTNODE,6)
DIMENSION atotl(*),thetaP(3),thetaQ(3),thetPQ(3)
DIMENSION ttrP(3,3),ttrQ(3,3),ttrPQ(3,3)

```

```

do nhing=1,nhing1
  IP=mhing1(1,nhing)
  IQ=mhing1(2,nhing)
  iching=idofh1(nhing)

  do i = 1,3
    thetaP(i)=0.
    thetaQ(i)=0.
  end do

  do i = 1,3
    IR=NODBC(IP,3+i)
    if(IR.ne.0) then
      thetaP(i)=atotl(IR)
    end if
  end do

  do i = 1,3
    IR=NODBC(IQ,3+i)
    if(IR.ne.0) then
      thetaQ(i)=atotl(IR)
    end if
  end do

```



```

        end do
c       write(6,*) 'thetP=',thetaP
c       write(6,*) 'thetQ=',thetaQ

        call thettt(thetaP,ttrP)
        call thettt(thetaQ,ttrQ)

c       write(6,*) 'ttrP=',ttrP
c       write(6,*) 'ttrQ=',ttrQ

        do i=1,3
            do j =1,3
                ttrPQ(i,j)=ttrQ(j,i)
            end do
        end do

        do i=1,3
            do j =1,3
                ttrQ(i,j)=ttrPQ(i,j)
            end do
        end do

        call matmat(ttrQ,ttrP,ttrPQ)
c       write(6,*) 'ttrPQ=',ttrPQ
        call ttthet(ttrPQ,thetPQ)
c       write(6,*) 'thetPQ=',thetPQ

        hing1(5,nhing) =thetPQ(1)*hing1(1,nhing)+
&                      thetPQ(2)*hing1(2,nhing)+
&                      thetPQ(3)*hing1(3,nhing)

    end do

    return
end

```

C analy3.f:

C

```
      SUBROUTINE LDSTEP(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&                      LM,MAXH,MHT,
&                      X0,XYZN,xnode1,melm,NODBC,
&                      nhing1,nhing3,
&                      mhing1,mhing3,
&                      hing1,idofh1,
&                      cncm,
&                      ptload,distgg,distnn,distpp,
&                      qext0,qint,da,dq,da0,dq0,
&                      sda,sdq,sxnode,
&                      ttrg,t6rg,omegag,om6gag,
&                      sttrg,st6rg,somegg,som6gg,
&                      AST,AST1,diag,IPVT,WORK,
&                      atotl,atotl0,
&                      maxnld,maxldn,maxitr,
&                      NLD,lldown,niter,
&                      toltld,tolda,toldq,toldadq,
&                      alambd,alamb0,det0,rdet)

      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION LM(*),MAXH(*),MHT(*)
      DIMENSION X0(*),XYZN(*),xnode1(*),MELM(*),NODBC(*)
      DIMENSION mhing1(2,*),mhing3(2,*),
      DIMENSION hing1(5,*),idofh1(*)
      DIMENSION CNCM(6,4,nelm)
      DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
      DIMENSION qext0(*),qint(*)
      DIMENSION da(*),dq(*),da0(*),dq0(*)
      DIMENSION sda(*),sdq(*),sxnode(*)
      DIMENSION ttrg(*),t6rg(*),omegag(*),om6gag(*)
      DIMENSION sttrg(*),st6rg(*),somegg(*),som6gg(*)
      DIMENSION AST(*),AST1(*),diag(*),IPVT(*),WORK(*)
      DIMENSION atotl(*),atotl0(*)
```

```
      Npr=1
      lldown=0
      niter=0
```

```
      IF(NLD.EQ.maxnld) then
        STOP 'MAXIMUM NUMBER OF LOAD STEPS EXCEEDED'
      END IF
```

```
      IF(alambd.GT.toltld) then
        stop 'MAXIMUM LOAD FACTOR EXCEEDED'
      END IF
      alinc=alambd-alamb0
```

```
      Npr=Npr+1
      NLD=NLD+1
```

18 continue

```
      c      write(6,*) '      '
      c      write(6,*) 'load level NLD=', nld, ' alambd =',alambd
      c      write(6,*) '      '
```

```

CALL CLVEC(AST,NWK)
CALL CLVEC(dq0,MAX)

MAXEQN=MAX-1

C
C ASSEMBLY THE TANGENT STIFFNESS MATRIX: AST
C
C
C
CALL CLVEC(AST,NWK)
CALL CLVEC(dq0,MAX)

LCODE = 1
CALL ASSEMB(NTNODE,NELM,melm,x0,xnode1,cncm,NODBC,MAXEQN,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          distgg,distnn,distpp,ptload,alambd,
&          ttrg,t6rg,omegag,om6gag,da,
&          LM,MAXH,MHT,NWK,MK,
&          AST,qint,qext0,LCODE)

DO IASSIGN = 1, NWK
  AST1(IASSIGN)=AST(IASSIGN)
END DO

C
C ASSEMBLY THE INTITIAL UNBALANCED LOAD,
C          AT CURRENT LOAD LEVEL
C
CALL CLVEC(dq,MAX)
LCODE = 2
CALL ASSEMB(NTNODE,NELM,melm,x0,xnode1,cncm,NODBC,MAXEQN,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          distgg,distnn,distpp,ptload,alambd,
&          ttrg,t6rg,omegag,om6gag,da,
&          LM,MAXH,MHT,NWK,MK,
&          AST,qint,qext0,LCODE)
CALL RESIDL(qext0,qint,maxeqn,alambd,dq)

c
c write(6,*) 'intitial unbalanced LOAD'
c write(6,*) 'dq=',(dq(iii),iii=1,maxeqn)

if(nld.eq.1) then
  DO ING=1,MAXEQN
    dq0(ING)=dq(ING) ! get the intitial unbalanced LOAD
  END DO
end if

C
C DECOMPOSITION OF TANGENT STIFFNESS MATRIX
C
CALL DECOMP1(MAXEQN,MAXEQN,AST,COND,IPVT,NCHG,WORK)
CALL LUDCMP(AST,MAXEQN,MAXEQN,IPVT,D)
CALL COLSOL(AST,da,MAXH,MAXEQN,NWK,MAX,1)

```

```

64      CONTINUE

      DO ING=1,MAXEQN
        da(ING)=dq(ING) ! get the unbalanced load
      END DO

      ITR=1      ! number of iterations inside the incremental load
loop
C-----SOLVE THE MATRIX EQUATIONS TO GET DISPLACEMENTS
C      DUE TO INTITIAL UNBALANCED LOAD

c      CALL LUBKSB(AST,MAXEQN,MAXEQN,IPVT,da)
      CALL SOLVE1(MAXEQN,MAXEQN,AST,da,IPVT)
C      CALL COLSOL(AST,da,MAXH,MAXEQN,NWK,MAX,2)
C
C      RECOVER daQ
C
      call recdal(da,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)

      if(nld.eq.1) then
        DO ING=1,MAXEQN
          da0(ING)=da(ING) !store the displ due to intitial
unbalanced LOAD
        END DO
        CALL VECVEC(dq0,dq0,dq0dq0,MAXEQN) ! inner product dq0.dq0
        CALL VECVEC(da0,da0,da0da0,MAXEQN) ! inner product da0.da0
      end if

24      CONTINUE      ! iterations inside a load step start here

      ITR=ITR+1
      niter=niter+1
C      IF(ITR.GT.76) GO TO 28

C
C      UPDATE xnodel
C
      CALL upxnod(ntnode,nelm,MELM,NODBC,xnodel,da)
      CALL upatot(ntnode,NODBC,atot1,da)
      CALL thetah(atot1,nhing1,mhing1,hing1,idofh1,NODBC,NTNODE)
C
C      UPDATE ttrg & omegag
C
      CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
&      x0,da,ttrg,omegag)
      CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
&      x0,da,t6rg,om6gag)
C
C      ASSEMBLY THE UPDATED TANGENT STIFFNESS MATRIX: AST
C
C      CALL CLVEC(AST,NWK)
      LCODE = 1
      CALL ASSEMB(NTNODE,NELM,melm,x0,xnodel,cncm,NODBC,MAXEQN,
&      nhing1,nhing3,

```

```

&             mhing1,mhing3,
&             hing1,idofh1,
&             distgg,distnn,distpp,ptload,alambd,
&             ttrg,t6rg,omegag,om6gag,da,
&             LM,MAXH,MHT,NWK,MK,
&             AST,qint,qext0,LCODE)
DO IASSIGN = 1, NWK
  AST1(IASSIGN)=AST(IASSIGN)
END DO

C
C DECOMPOSITION OF TANGENT STIFFNESS MATRIX
C
  CALL DECOMP1(MAXEQN,MAXEQN,AST,COND,IPVT,NCHG,WORK)
C   CALL LUDCMP(AST,MAXEQN,MAXEQN,IPVT,D)
C   CALL COLSOL(AST,da,MAXH,MAXEQN,NWK,MAX,1)

C
C ASSEMBLY THE unbalanced LOAD for updated configuration
C   at current load level
C
  CALL CLVEC(dq,MAX)
  LCODE = 2
  CALL ASSEMB(NTNODE,NELM,melm,x0,xnode1,cncm,NODBC,MAXEQN,
&            nhing1,nhing3,
&            mhing1,mhing3,
&            hing1,idofh1,
&            distgg,distnn,distpp,ptload,alambd,
&            ttrg,t6rg,omegag,om6gag,da,
&            LM,MAXH,MHT,NWK,MK,
&            AST,qint,qext0,LCODE)
  CALL RESIDL(qext0,qint,maxeqn,alambd,dq)

  CALL VECVEC(dq,dq,dqdq,MAXEQN) ! inner product dq.dq

c
c   store current unbalanced force into da temporarily.
c
  do ing = 1,maxeqn
    da(ing) = dq(ing)
  end do

c
c-----SOLVE THE MATRIX EQUATIONS TO GET DISPLACEMENTS under unbalanced
force
c
  CALL LUBKSB(AST,MAXEQN,MAXEQN,IPVT,da)
  CALL SOLVE1(MAXEQN,MAXEQN,AST,da,IPVT)
c   CALL COLSOL(AST,da,MAXH,MAXEQN,NWK,MAX,2)
C
C RECOVER daQ
C
  call recdal(da,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)

  CALL VECVEC(da,da,dada,MAXEQN) ! inner product da.da

```

```

dq0dq0=1.d0
da0da0=1.d0

RATIOI=DSQRT(dqdq/dq0dq0)
RATIOD=DSQRT(dada/da0da0)
RATLD=DSQRT(dqdq/dada)
WRITE (16,*) itr,RATIOI
C   WRITE (6,*) 'RATLD=',RATLD

C
C   CONVERGENCE CHECKING FOR LOAD RESIDUAL
C
      IF(RATIOI.LE.toldq) GO TO 28      !CONVERGENCE

C
C   CONVERGENCE CHECKING FOR DISPLACEMENT INCREMENT
C
      IF(RATIOD.LE.tolda) GO TO 28      !CONVERGENCE

C
C   CONVERGENCE CHECKING FOR LOAD RESIDUAL AND DISPLACEMENT INCREMENT
C
      IF(RATIOD.LE.toldadq.AND.RATIOI.LE.toldadq) GO TO 28
!CONVERGENCE

C
C   CHECK MAXIMUM NUMBER OF ITERATIONS INSIDE OF CURRENT LOAD LOOP
C
      IF(ITR.GT.MAXITR) GO TO 66          ! EXCEED

      GO TO 24                          ! GO TO ANOTHER ITR LOOP

28      CONTINUE                          ! GET HERE IF CONVERGENCE
ACHIEVES.

      WRITE(6,*) 'The number of iteration itr =',itr
      WRITE(6,*) 'dada=',dada, 'dqdq=',dqdq
      WRITE (6,20003) RATIOD,RATIOI
20003  FORMAT(/2X,'RATIOS: RATIOD =',1PE20.10,'RATIOI =',1PE20.10)
c      WRITE (6,*) 'RATLD=',RATLD

      WRITE(10,*) '      '
      WRITE(10,*) '===== '
      write(10,*) 'load level NLD=', nld, '      alambd=',alambd

C
C
C
      CALL DIADET(AST,IPVT,DIAG,MAXEQN,NCHG,NNEGA,NNEGD,DET)
      idet = IPVT(MAXEQN)*(-1)**NNEGA
      if(nld.eq.1) then
          det0=det
      else if (nld.gt.1) then
          rdet=dfloat(idet)*dexp(det-det0)

```

```

        write(10,*) 'Relative Det =', rdet
    end if

    call tempsv(maxeqn,nelm,
&                xnode1,ttrg,omegag,da,dq,
&                sxnode,sttrg,somegg,sda,sdq)

    call tempsv(maxeqn,nelm,
&                xnode1,t6rg,om6gag,da,dq,
&                sxnode,st6rg,som6gg,sda,sdq)

    return        ! go to new load loop
C
C
C
66    CONTINUE
C
C    CONVERGENCE NOT ACHIEVED. REDUCE THE LOAD STEP BY HALF.
C
    write(6,*) 'Load increment is reduced by half'

    ldown=ldown+1
    ALINC=ALINC*0.5D0
    alambd=alamb0+ALINC

    write(6,*) 'Load level is reduced to alambd= ', alambd

    call tempgt(maxeqn,nelm,
&                xnode1,ttrg,omegag,da,dq,
&                sxnode,sttrg,somegg,sda,sdq)
    call tempgt(maxeqn,nelm,
&                xnode1,t6rg,om6gag,da,dq,
&                sxnode,st6rg,som6gg,sda,sdq)

    IF(ldown.GT.maxldn) then
        return
    end if

    GO TO 18

    END

```

```

C  analy4.f
C
C
c   this subroutine is used to locate bifurcation point by bisection
method
c
c   when the relative determinant of tangential stiffness matrix rdet
changes
c   sign (from positive to negative). this method will fail to locate
the exact
c   bifurcation point if the noise is large.
c
c
c

```

```

      SUBROUTINE BIFUR2(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&
&          LM,MAXH,MHT,
&          X0,XYZN,xnode1,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet,
&          aaa,bbb,faaa,fbbb)
      IMPLICIT REAL*8(A-H,O-Z)
      DIMENSION X(1),Y(1),S(1)
      DIMENSION LM(*),MAXH(*),MHT(*)
      DIMENSION X0(*),XYZN(*),xnode1(*),MELM(*),NODBC(*)
      DIMENSION mhing1(2,*),mhing3(2,*)
      DIMENSION hing1(5,*),idofh1(*)
      DIMENSION CNCM(6,4,*)
      DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
      DIMENSION qext0(*),qint(*)
      DIMENSION da(*),dq(*),da0(*),dq0(*)
      DIMENSION sda(*),sdq(*),sxnode(*)
      DIMENSION ttrg(*),t6rg(*),omegag(*),om6gag(*)
      DIMENSION sttrg(*),st6rg(*),somegg(*),som6gg(*)
      DIMENSION AST(*),AST1(*),diag(*),IPVT(*),WORK(*)
      DIMENSION atotl(*),atotl0(*)

      do ii = 1,50
         ccc=0.5*(aaa+bbb)
         X(1)=ccc
         CALL OBJ(X,F,
&
&          NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&          LM,MAXH,MHT,

```



```

&          X0,XYZN,xnode1,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)

ccc=alambd
fccc=rdet

if(faaa*fccc.lt.0.) then
  bbb=ccc
  fbbb=fccc
else
  aaa=ccc
  faaa=fccc
end if
if(dabs(bbb-aaa)/dabs(ccc).le.1.d-5
& .and.dabs(fccc).le.1.d-5)then
  return
end if
end do

return
end

c
c  this subroutine is used to locate bifurcation point by bisection
c  method
c
c  when the relative determinant of tangential stiffness matrix rdet
c  starts
c  to increase after it decreases and when rdet is small enough (to
c  make sure
c  that there is a point inside at which rdet = 0. it is
c  especially useful when rdet >= 0. when rdet is not smooth enough,
c  it may
c  fail to locate the exact bifurcation point.
c
c  if rdet changes sign from positive to negative, it is better use
c  bisection
c  method.
c
c  it is not used currently as there is some bugs inside.
c
c
c
c
SUBROUTINE BISEC2(NTNODE,NELM,NWK,MK,MAX,MAXEQN,

```

```

&          LM,MAXH,MHT,
&          X0,XYZN,xnode1,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet,
&          aaa,bbb,ccc,faaa,fbbb,fccc)

IMPLICIT REAL*8(A-H,O-Z)
DIMENSION X(1),Y(1),S(1)
DIMENSION LM(*),MAXH(*),MHT(*)
DIMENSION X0(*),XYZN(*),xnode1(*),MELM(*),NODBC(*)
DIMENSION mhing1(2,*),mhing3(2,*)
DIMENSION hing1(5,*),idofh1(*)
DIMENSION CNCM(6,4,*)
DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
DIMENSION qext0(*),qint(*)
DIMENSION da(*),dq(*),da0(*),dq0(*)
DIMENSION sda(*),sdq(*),sxnode(*)
DIMENSION ttrg(*),t6rg(*),omegag(*),om6gag(*)
DIMENSION sttrg(*),st6rg(*),somegg(*),som6gg(*)
DIMENSION AST(*),AST1(*),diag(*),IPVT(*),WORK(*)
DIMENSION atotl(*),atotl0(*)

do ii = 1,50

  aaal=0.5*(aaa+ccc)
  X(1)=aaal
  CALL OBJ(X,faaal,
&          NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&          LM,MAXH,MHT,
&          X0,XYZN,xnode1,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,

```

```

&                                alambd,alamb0,det0,rdet)

if(faaa1.ge.fccc) then
  aaa = aaal
  faaa = faaal

  bbb1 = 0.5*(ccc+bbb)
  X(1)=bbb1
  CALL OBJ(X,fbbb1,
&                                NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&                                LM,MAXH,MHT,
&                                X0,XYZN,xnode1,melm,NODBC,
&                                nhing1,nhing3,
&                                mhing1,mhing3,
&                                hing1,idofh1,
&                                cncm,
&                                ptload,distgg,distnn,distpp,
&                                qext0,qint,da,dq,da0,dq0,
&                                sda,sdq,sxnode,
&                                ttrg,t6rg,omegag,om6gag,
&                                sttrg,st6rg,somegg,som6gg,
&                                AST,AST1,diag,IPVT,WORK,
&                                atotl,atotl0,
&                                maxnld,maxldn,maxitr,
&                                NLD,ldown,niter,
&                                toltld,tolda,toldq,toldadq,
&                                alambd,alamb0,det0,rdet)

  bbb1 = alambd
  if(fbbb1.ge.fccc) then
    bbb = bbb1
    fbbb = fbbb1
  else if(fbbb1.lt.fccc) then
    ccc = bbb1
    fccc = fbbb1
  end if
else if(faaa1.lt.fccc) then
  ccc = aaal
  fccc = faaal

  bbb1 = 0.5*(ccc+bbb)
  X(1)=bbb1
  CALL OBJ(X,fbbb1,
&                                NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&                                LM,MAXH,MHT,
&                                X0,XYZN,xnode1,melm,NODBC,
&                                nhing1,nhing3,
&                                mhing1,mhing3,
&                                hing1,idofh1,
&                                cncm,
&                                ptload,distgg,distnn,distpp,
&                                qext0,qint,da,dq,da0,dq0,
&                                sda,sdq,sxnode,
&                                ttrg,t6rg,omegag,om6gag,
&                                sttrg,st6rg,somegg,som6gg,
&                                AST,AST1,diag,IPVT,WORK,
&                                atotl,atotl0,
&                                maxnld,maxldn,maxitr,

```



```

&          NLD,ldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION X(1),Y(1),S(1)
DIMENSION LM(*),MAXH(*),MHT(*)
DIMENSION X0(*),XYZN(*),xnodel(*),MELM(*),NODBC(*)
DIMENSION mhing1(2,*),mhing3(2,*),
DIMENSION hing1(5,*),idofh1(*)
DIMENSION CNCM(6,4,*)
DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
DIMENSION qext0(*),qint(*)
DIMENSION da(*),dq(*),da0(*),dq0(*)
DIMENSION sda(*),sdq(*),sxnode(*)
DIMENSION ttrg(*),t6rg(*),omegag(*),om6gag(*)
DIMENSION sttrg(*),st6rg(*),somegg(*),som6gg(*)
DIMENSION AST(*),AST1(*),diag(*),IPVT(*),WORK(*)
DIMENSION atotl(*),atotl0(*)

X(1) = alambd
FX=1.
S(1) = alambd-alamb0
ITERR = 0

CALL DSCPOW(X,FX,Y,FY,S,S,1,2,0.00001,
&          1,ITERR,IEX,
&          NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&          LM,MAXH,MHT,
&          X0,XYZN,xnodel,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,ldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)

return
end

SUBROUTINE OBJ(X,FX,
&          NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&          LM,MAXH,MHT,
&          X0,XYZN,xnodel,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,

```

```

&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)

IMPLICIT REAL*8(A-H,O-Z)
DIMENSION X(1)
DIMENSION LM(*),MAXH(*),MHT(*)
DIMENSION X0(*),XYZN(*),xnodel(*),MELM(*),NODBC(*)
DIMENSION mHING1(2,*),mHING3(2,*),
DIMENSION HING1(5,*),IDOFH1(*)
DIMENSION CNCM(6,4,*)
DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
DIMENSION qEXT0(*),QINT(*)
DIMENSION DA(*),DQ(*),DA0(*),DQ0(*)
DIMENSION SDA(*),SDQ(*),SXNODE(*)
DIMENSION TTRG(*),T6RG(*),OMEGAG(*),OM6GAG(*)
DIMENSION STTRG(*),ST6RG(*),SOMEGG(*),SOM6GG(*)
DIMENSION AST(*),AST1(*),DIAG(*),IPVT(*),WORK(*)
DIMENSION ATOTL(*),ATOTL0(*)

alambd= X(1)

CALL LDSTEP(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&          LM,MAXH,MHT,
&          X0,XYZN,xnodel,melm,NODBC,
&          nHING1,nHING3,
&          mHING1,mHING3,
&          HING1,IDOFH1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)

X(1)=alambd
FX=dsqrt(dsqrt(rdet*rdet))

RETURN
END
SUBROUTINE DSCPOW(X,FX,Y,FY,S,DELX,N,INDIC,TOL,
&          ITOL,ITERR,IEX,
&          NTNODE,NELM,NWK,MK,MAX,MAXEQN,

```

```

&             LM,MAXH,MHT,
&             X0,XYZN,xnode1,melm,NODBC,
&             nhing1,nhing3,
&             mhing1,mhing3,
&             hing1,idofh1,
&             cncm,
&             ptload,distgg,distnn,distpp,
&             qext0,qint,da,dq,da0,dq0,
&             sda,sdq,sxnode,
&             ttrg,t6rg,omegag,om6gag,
&             sttrg,st6rg,somegg,som6gg,
&             AST,AST1,diag,IPVT,WORK,
&             atotl,atotl0,
&             maxnld,maxldn,maxitr,
&             NLD,lldown,niter,
&             toltld,tolda,toldq,toldadq,
&             alambd,alamb0,det0,rdet)

IMPLICIT REAL*8(A-H,O-Z)

DIMENSION X(N),Y(N),S(N),DELX(N)

DIMENSION LM(*),MAXH(*),MHT(*)
DIMENSION X0(*),XYZN(*),xnode1(*),MELM(*),NODBC(*)
DIMENSION mhing1(2,*),mhing3(2,*)
DIMENSION hing1(5,*),idofh1(*)
DIMENSION CNCM(4,6,*)
DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
DIMENSION qext0(*),qint(*)
DIMENSION da(*),dq(*),da0(*),dq0(*)
DIMENSION sda(*),sdq(*),sxnode(*)
DIMENSION ttrg(*),t6rg(*),omegag(*),om6gag(*)
DIMENSION sttrg(*),st6rg(*),somegg(*),som6gg(*)
DIMENSION AST(*),AST1(*),diag(*),IPVT(*),WORK(*)
DIMENSION atotl(*),atotl0(*)

IF(ITOL.EQ.1) GO TO 110
TOL = .001
110 FTOL2 = TOL/1000.
NTOL = 0
IEX = 1
K = -2
M = 0
FA = FX
DAA = 0.
STEP = 1.
D = STEP
IF(INDIC.EQ.2) GO TO 1
IF(ITERR.EQ.0) GO TO 1
DXNORM = 0.
SNORM = 0.
DO 102 I = 1,N
DXNORM = DXNORM + DELX(I)*DELX(I)
102 SNORM = SNORM + S(I)*S(I)
IF(INDIC.NE.1) GO TO 103
IF(DXNORM.GE.SNORM) GO TO 1
103 RATIO = DXNORM/SNORM

```

```

STEP = DSQRT(RATIO)
D = STEP
1 DO 2 I =1,N
2 Y(I) = X(I) + D*S(I)
CALL OBJ(Y,F,
& NTNODE ,NELM ,NWK ,MK ,MAX ,MAXEQN ,
& LM ,MAXH ,MHT ,
& X0 ,XYZN ,xnodel ,melm ,NODBC ,
& nhing1 ,nhing3 ,
& mhing1 ,mhing3 ,
& hing1 ,idofhl ,
& cncm ,
& ptload ,distgg ,distnn ,distpp ,
& qext0 ,qint ,da ,dq ,da0 ,dq0 ,
& sda ,sdq ,sxnodel ,
& ttrg ,t6rg ,omegag ,om6gag ,
& sttrg ,st6rg ,somegg ,som6gg ,
& AST ,AST1 ,diag ,IPVT ,WORK ,
& atotl ,atotl0 ,
& maxnld ,maxldn ,maxitr ,
& NLD ,ldown ,niter ,
& toltld ,tolda ,toldq ,toldadq ,
& alambd ,alamb0 ,det0 ,rdet )
c if(dabs(f).lt.ftol2) return
K = K + 1
IF(F-FA) 5,3,6
3 D = 0.5*(DAA+D)
DO 4 I = 1,N
4 Y(I) = X(I) + D*S(I)
CALL OBJ(Y,F,
& NTNODE ,NELM ,NWK ,MK ,MAX ,MAXEQN ,
& LM ,MAXH ,MHT ,
& X0 ,XYZN ,xnodel ,melm ,NODBC ,
& nhing1 ,nhing3 ,
& mhing1 ,mhing3 ,
& hing1 ,idofhl ,
& cncm ,
& ptload ,distgg ,distnn ,distpp ,
& qext0 ,qint ,da ,dq ,da0 ,dq0 ,
& sda ,sdq ,sxnodel ,
& ttrg ,t6rg ,omegag ,om6gag ,
& sttrg ,st6rg ,somegg ,som6gg ,
& AST ,AST1 ,diag ,IPVT ,WORK ,
& atotl ,atotl0 ,
& maxnld ,maxldn ,maxitr ,
& NLD ,ldown ,niter ,
& toltld ,tolda ,toldq ,toldadq ,
& alambd ,alamb0 ,det0 ,rdet )
c if(dabs(f).lt.ftol2) return
IF(F-FA) 204,202,205
202 DO 203 I = 1,N
203 Y(I) = X(I) + DAA*S(I)
FY = FA
GO TO 326
204 FC = FA
DC = DAA + 2.*(D-DAA)
FB = F

```



```

      DB = D
      GO TO 21
205  STEP = 0.5*STEP
      IF(K) 7,8,206
206  DC = D
      D = DAA
      DAA = DB
      DB = D
      FC = F
      F = FA
      FA = FB
      FB = F
      GO TO 10
5    FC = FB
      FB = FA
      FA = F
      DC = DB
      DB = DAA
      DAA = D
      D = 2.*D + STEP
      GO TO 1
6    IF(K)7,8,9
7    FB = F
      DB = D
      D = -D
      STEP = -STEP
      GO TO 1
8    FC = FB
      FB = FA
      FA = F
      DC = DB
      DB = DAA
      DAA = D
      GO TO 21
9    DC = DB
      DB = DAA
      DAA = D
      FC = FB
      FB = FA
      FA = F
10   D = 0.5*(DAA+DB)
      DO 11 I = 1,N
11   Y(I) = X(I) + D*S(I)
      CALL OBJ(Y,F,
&          NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&          LM,MAXH,MHT,
&          X0,XYZN,xnode1,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,

```

```

&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)

c          if(dabs(f).lt.ftol2) return
12         IF((DC-D)*(D-DB)) 15,13,18
13         DO 14 I = 1,N
14         Y(I) =X(I) + DB*S(I)
           FY = FB
           IF(IEX.EQ.0) GO TO 32
           GO TO 326
15         IF(F-FB) 16,13,17
16         FC = FB
           FB = F
           DC = DB
           DB = D
           GO TO 21
17         FA = F
           DAA = D
           GO TO 21
18         IF(F-FB) 19,13,20
19         FA = FB
           FB = F
           DAA = DB
           DB = D
           GO TO 21
20         FC = F
           DC = D
21         IF(DABS(DAA-DB).GE.DABS(DC-DB)) GO TO 50
           D = DAA
           DAA = DC
           DC = D
           F = FA
           FA = FC
           FC = F
50         IF(DABS((DC-DB)/(DAA-DB)).GT.0.25) GO TO 26
           D = 0.5*(DAA+DB)
           DO 51 I = 1,N
51         Y(I) = X(I) + D*S(I)
           CALL OBJ(Y,F,
&          NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&          LM,MAXH,MHT,
&          X0,XYZN,xnode1,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,

```

```

&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)
c      if(dabs(f).lt.ftol2) return
      IF(F-FB) 52,13,53
52     FC = FB
      FB = F
      DC = DB
      DB = D
      GO TO 26
53     FA = F
      DAA = D
26     A = FA*(DB-DC) + FB*(DC-DAA) + FC*(DAA-DB)
      IF(A) 22,30,22
22     D = 0.5*((DB*DB-DC*DC)*FA + (DC*DC-DAA*DAA)*FB +
&          (DAA*DAA-DB*DB)*FC)/A
      IF((DAA-D)*(D-DC)) 13,13,23
23     DO 24 I = 1,N
24     Y(I) = X(I) + D*S(I)
      CALL OBJ(Y,F,
&          NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&          LM,MAXH,MHT,
&          X0,XYZN,xnode1,me1m,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atot1,atotl0,
&          maxnld,maxlnd,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)
c      if(dabs(f).lt.ftol2) return
      IF(DABS(FB-F)-(DABS(FB)+FTOL2)*TOL) 28,28,12
28     IEX = 0
      IF(F-FB) 29,13,13
29     FY = F
      GO TO 32
30     IF(M) 31,31,13
31     M = M + 1
      GO TO 10
32     DO 99 I = 1,N
      IF(Y(I).NE.X(I)) GO TO 326
99     CONTINUE
33     IF(NTOL.EQ.5) GO TO 34
      IEX = 1
      NTOL = NTOL + 1
      TOL = TOL/10.
      GO TO 12
34     IEX = 1
      GO TO 35

```

```

326    IF(FY.LT.FX) GO TO 36
      IEX = 1
      GO TO 35
36     IF(IEX.EQ.0) GO TO 35
      IEX = 2
35     RETURN
      END

c
c
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
cccc
c
c
c   this subroutine is used to perform linearized bifurcation buckling
c   analysis by solving eigenproblem:
c
c             (Kl + lambda Knl) u = 0                               (1)
c
c   According to the theory of stability, a bifurcation point
c   corresponds
c   to the condition that the first variation of the virtual work
c   vanishes:
c
c             ddW = 0                                             (2)
c
c   or
c
c             det(Kt) = 0                                         (3)
c
c   or
c
c             Kt u = 0                                             (4)
c
c   or
c
c             (Kt + dlambda I) u = 0   with dlambda = 0          (5)
c
c   where Kt is the tangential stiffness matrix at load level lambda
c
c
c   assume that we start from a reference load level lambda0 and
c   increase
c   load by dlambda to lambda:
c
c             lambda = lambda0 + dlambda                           (6)
c
c   now we expand Kt using Talor series and negelect higher order of
c   terms:
c
c             Kt = Kt(lambda)
c                = Kt(lambda0 + dlambda)
c                = Kt(lambda0) + [dKt(lambda0)/dlambda] dlambda
c
c   or
c
c             Kt = Kl + dlambda Knl                               (7)
c
c   where
c
c             Kl = Kt(lambda0)                                    (7a)

```

$$K_{n1} = dK_t(\lambda_0)/d\lambda \quad (7b)$$

using eqns (7) and (4), we obtain eigen problem:

$$(K_1 + d\lambda K_{n1}) u = 0 \quad (8)$$

considering the case

$$\lambda_0 = 0 \quad (9)$$

which means that

$$\lambda = d\lambda \quad (10)$$

we obtain linearized bifurcation buckling problem (1)

now we examine the K_t in detail. we can separate K_t as

$$K_t = K_m + K_g + K_p \quad (11)$$

where

K_m is the material part of K_t

K_g is the geometric part of K_t

K_p is the pressure part of K_t

K_m is a function of material properties and rotation matrix Λ

$$K_m = K_m(C_n, C_m, \Lambda) \quad (12)$$

K_g is a function of stress resultants and couples

$$K_g = K_g(n, m) \quad (13)$$

K_p is a function of load factor λ and rotation matrix Λ

$$K_p = K_p(\lambda, \Lambda) \quad (14)$$

Simply pushing it, K_m , K_g , and K_p all are function of load factor λ .

so we can linearize them as before:

$$K_m = K_{m1} + d\lambda K_{mn1} \quad (15)$$

with

$$K_{m1} = K_m(\lambda_0) \quad (15a)$$

$$K_{mn1} = dK_m(\lambda_0)/d\lambda \quad (15b)$$

$$K_g = K_{g1} + d\lambda K_{gn1} \quad (16)$$

with

$$K_{g1} = K_g(\lambda_0) \quad (16a)$$

$$K_{gn1} = dK_g(\lambda_0)/d\lambda \quad (16b)$$

and

c $K_p = K_{pl} + d\lambda K_{pnl}$ (17)

c with

c $K_{pl} = K_p(\lambda_0)$ (17a)

c $K_{pnl} = dK_p(\lambda_0)/d\lambda$ (17b)

c

c

c now substitute eqns (15-17) into eqn (11), we get:

c

c $K_t = (K_{ml} + K_{gl} + K_{pl}) + d\lambda (K_{mnl} + K_{gnl} + K_{pnl})$ (18)

c

c by comparison of eqn (18) with eqn (7), we obtain:

c

c $K_l = K_{ml} + K_{gl} + K_{pl}$ (18a)

c

c $K_{nl} = K_{mnl} + K_{gnl} + K_{pnl}$ (18b)

c

c assume that $\lambda_0 = 0$, (i.e., $\lambda = d\lambda$) and that there is
c no pre-stresses, it is obvious that

c

c $K_{gl} = 0$ and $K_{pl} = 0$

c

c but usually $K_{mnl} \neq 0$.

c

c therefore, eqn (18a) can be rewritten as

c

c $K_l = K_{ml}$ (18c)

c

c in classical theory, however, the assumption is made of use that
c either

c rotation is zero or neglected, hence

c

c $K_{mnl} = 0$ (18d)

c

c

c therefore, eqn (18b) can be rewritten as

c

c $K_{nl} = K_{gnl} + K_{pnl}$ (18e)

c

c furthermore, if there is no pressure load, i.e.,

c

c $K_{pnl} = 0$ (18f)

c we have

c $K_{nl} = K_{gnl}$ (18g)

c

c

c

c now we consider how to calculate K_l and K_{nl} .

c

c in principle, we can linearize K_t as eqn (7) by the assumption that
c rotation is small. therefore, rotation matrix is simplified to

c

c $\Lambda = \Lambda(\theta)$

c

c $= I + \text{skew}(\theta)$ (19)

c

```

c  substitute into the exact formula for Km, we get
c
c          Km = Km1(0) + Kmnl.t.theta          (20)
c
c  in which Kmnl is independent of theta and therefore indepent of
load
c  factor lambda while rotation vector theta is proportational to
lambda.
c  if we represent theta0 corresponds to reference load, then
c
c          Kmnl.t.theta = lambda Kmnl.(theta0)
c                      = lambda Kmnl          (21)
c  where
c
c          Kmnl = Kmnl.t.theta0          (21a)
c
c  for classical solution, we can simply set Kmnl = 0.
c
c
c  similarly, using linearized formular (19), we can get linearized
formulae
c  for stress resultants and couples:
c
c          n = lambda n_nlt.theta_0 = lambda n_nlt (22)
c
c          m = lambda m_nlt.theta_0 = lambda m_nlt (23)
c
c  where
c
c          n_nl = n_nlt.theta_0          (22a)
c
c          m_nl = m_nlt.theta_0          (23a)
c
c  which are linearized stress resultants and couples associated
with reference load.
c
c  substitute eqns (22a) and (23a) into the formula for Kg, we can get
Kgnl.
c
c
c  the same to Kpnl.
c
c
c  as it is messy to get Knl via above approach, here quasi forward
finite
c  difference method is used to get Knl.
c
c
c
c  -----
c  ---
c
c
c  two different approaches to choose K1 and Knl:
c
c  1: K1 is the tangential stiffness matrix at lambda equals to
zero.
c
c

```

```

c      Knl is geometric and pressure parts of tangential stiffness
matrix
c      out of first iteration of the first load step. this approach
is
c      classical approach in the sense that rotation is assumed to be
zero
c      or negligible.
c
c      Knl = [Kg(lambda_eps)+Kp(lambda_eps)]/lambda_eps
c
c      lambda_eps is set to 0.0001
c
c
c      2: K1 is the tangential stiffness matrix at lambda equals to
zero.
c
c      Knl includes all terms of material, geometric and pressure
parts of
c      tangential stiffness matrix out of first iteration of the
first load
c      step.
c
c      Knl = [Km(lambda_eps)+Kg(lambda_eps)+Kp(lambda_eps)-
K1]/lambda_eps
c
c      lambda_eps is set to 0.001
c
c
c
c
c
c
c
c
c
c
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
cccccc

```

```

      SUBROUTINE BIFURL(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&                      LM,MAXH,MHT,
&                      X0,XYZN,xnode1,melm,NODBC,
&                      nhing1,nhing3,
&                      mhing1,mhing3,
&                      hing1,idofh1,
&                      cncm,
&                      ptload,distgg,distnn,distpp,
&                      qext0,qint,da,dq,da0,dq0,
&                      sda,sdq,sxnode,
&                      ttrg,t6rg,omegag,om6gag,
&                      sttrg,st6rg,somegg,som6gg,
&                      AST,AST1,diag,IPVT,WORK,
&                      atot1,atotl0,
&                      wr,wi,z,loc,fv1,iv1,
&                      maxnld,maxldn,maxitr,
&                      NLD,lldown,niter,
&                      toltld,tolda,toldq,toldadq,
&                      alambd,alamb0,det0,rdet,ip2)

```



```

IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION LM(*),MAXH(*),MHT(*)
DIMENSION X0(*),XYZN(*),xnodel(*),MELM(*),NODBC(*)
DIMENSION mHING1(2,*),mHING3(2,*),
DIMENSION HING1(5,*),idofH1(*)
DIMENSION CNCM(6,4,*)
DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
DIMENSION qext0(*),qint(*)
DIMENSION da(*),dq(*),da0(*),dq0(*)
DIMENSION sda(*),sdq(*),sxnode(*)
DIMENSION ttrg(*),t6rg(*),omegag(*),om6gag(*)
DIMENSION sttrg(*),st6rg(*),somegg(*),som6gg(*)
DIMENSION AST(*),AST1(*),diag(*),IPVT(*),WORK(*)
DIMENSION atot1(*),atot10(*)
DIMENSION WR(*),WI(*),Z(MAXEQN,*),LOC(*)
DIMENSION FV1(*),IV1(*)
MAXEQN=MAX-1

```

```

CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)

```

C
C
C
C

```

ASSEMBLY THE TANGENT STIFFNESS MATRIX: AST

```

```

CALL CLVEC(AST,NWK)

```

```

if(ip2.eq.1) then
  almde = 0.000001
end if
if(ip2.eq.2) then
  almde = 0.000001
end if

```

```

LCODE = 1

```

```

CALL ASSEMB(NTNODE,NELM,melm,x0,xnodel,cncm,NODBC,MAXEQN,
&          nhing1,nhing3,
&          mHING1,mHING3,
&          HING1,idofH1,
&          distgg,distnn,distpp,ptload,alambd,
&          ttrg,t6rg,omegag,om6gag,da,
&          LM,MAXH,MHT,NWK,MK,
&          AST,qint,qext0,LCODE)

```

C
C
C
C

```

ASSEMBLY THE INTITIAL UNBALANCED LOAD,
      AT CURRENT LOAD LEVEL

```

```

CALL CLVEC(dq,MAX)

```

```

LCODE = 2

```

```

CALL ASSEMB(NTNODE,NELM,melm,x0,xnodel,cncm,NODBC,MAXEQN,
&          nhing1,nhing3,
&          mHING1,mHING3,
&          HING1,idofH1,
&          distgg,distnn,distpp,ptload,alambd,
&          ttrg,t6rg,omegag,om6gag,da,
&          LM,MAXH,MHT,NWK,MK,
&          AST,qint,qext0,LCODE)

```

```

write(6,*) 'external load'
qq=0.
do ii=1, maxeqn
  qq=qq+qext0(ii)
end do

write(6,*) qq

alambd=almde
CALL RESIDL(qext0,qint,maxeqn,alambd,dq)

do ii = 1,nwk

  AST1(ii) = AST(ii)

end do

C
C DECOMPOSITION OF TANGENT STIFFNESS MATRIX
C
  CALL DECOMP1(MAXEQN,MAXEQN,AST,COND,IPVT,NCHG,WORK)

DO ING=1,MAXEQN
  da(ING)=dq(ING) ! get the unbalanced load
END DO

C-----SOLVE THE MATRIX EQUATIONS TO GET DISPLACEMENTS
C DUE TO INTITIAL UNBALANCED LOAD

  CALL SOLVE1(MAXEQN,MAXEQN,AST,da,IPVT)

C
C RECOVER daQ
C
  call recdal(da,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
C
C ASSEMBLY THE TANGENT STIFFNESS MATRIX: AST
C
C
C CALL CLVEC(AST,NWK)

c
c LCODE = 1
c CALL ASSEMB(NTNODE,NELM,me1m,x0,xnode1,cncm,NODBC,MAXEQN,
c & nhing1,nhing3,
c & mhing1,mhing3,
c & hing1,idofh1,
c & distgg,distnn,distpp,ptload,alambd,
c & ttrg,t6rg,omegag,om6gag,da,
c & LM,MAXH,MHT,NWK,MK,
c & AST,qint,qext0,LCODE)
do ii = 1,nwk

  AST(ii) = AST1(ii)

```

```

        end do

C
C   UPDATE xnod1
C
        CALL upxnod(ntnode,nelm,MELM,NODBC,xnod1,da)
        CALL upatot(ntnode,NODBC,atot1,da)
        CALL thetah(atot1,nhing1,mhing1,hing1,idofh1,NODBC,NTNODE)
C
C   UPDATE ttrg & omegag
C
        CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
&                 x0,da,ttrg,omegag)
        CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
&                 x0,da,t6rg,om6gag)
C
C   ASSEMBLY THE Nonlinear TANGENT STIFFNESS MATRIX: AST1
C
C
        CALL CLVEC(AST1,NWK)
        LCODE = 1
        alambd=almde
        if(ip2.eq.1) then
            CALL ASSEMN(NTNODE,NELM,melm,x0,xnod1,cncm,NODBC,MAXEQN,
&                    nhing1,nhing3,
&                    mhing1,mhing3,
&                    hing1,idofh1,
&                    distgg,distnn,distpp,ptload,alambd,
&                    ttrg,t6rg,omegag,om6gag,da,
&                    LM,MAXH,MHT,NWK,MK,
&                    AST1,qint,qext0,LCODE)
        end if
        if(ip2.eq.2) then
            CALL ASSEMB(NTNODE,NELM,melm,x0,xnod1,cncm,NODBC,MAXEQN,
&                    nhing1,nhing3,
&                    mhing1,mhing3,
&                    hing1,idofh1,
&                    distgg,distnn,distpp,ptload,alambd,
&                    ttrg,t6rg,omegag,om6gag,da,
&                    LM,MAXH,MHT,NWK,MK,
&                    AST1,qint,qext0,LCODE)
        end if

        do ii = 1,nwk
            if(ip2.eq.1) then
                AST1(ii) = -AST1(ii)/almde
            end if
            if(ip2.eq.2) then
                AST1(ii) = (AST(ii)- AST1(ii))/almde
            end if
        end do

        MATZ=2
        call EIGRGG(maxeqn,maxeqn,AST,AST1,WR,WI,FV1,
&               MATZ,Z,LOC,Nmin,IERR)

```

```

DO II=1,Nmin
  imag=0
  imag0=0
  LOCAT=LOC(II)
  do i = 1, locat-1
    if(dabs(WI(i)).gt.0.) then
      imag=imag+1
    end if
  end do

  if(dabs(WI(locat)).gt.0.) then
    imag0=1
  end if
  WRITE(10,*) ' '
  WRITE(10,*) ' '
  WRITE(10,4) II,WR(locat)
4  FORMAT(2X,'Bifurcation buckling load factor',I2, '
=' ,E20.10/)
  WRITE(10,*) ' '
  WRITE(10,*) ' '
  WRITE(10,5)
5  FORMAT(2X,'Buckling Shape=')

  do icol=1, 1+imag0
    do i=1,maxeqn
      FV1(i) = Z(i,locat+imag-icol+1)
    end do
    call recdal(FV1,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
    CALL upxnod(ntnode,nelm,MELM,NODBC,xnodel,FV1)
    CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)
    do i=1,maxeqn
      Z(i,locat+imag-icol+1)=FV1(i)
    end do
  end do
  do icol=1, 1+imag0
    WRITE(10,6) (Z(i,locat+imag-icol+1), i=1,maxeqn)
  end do

6  FORMAT(2X,5E20.10)
  WRITE(10,7) IERR
7  FORMAT(2X,'IERR = ',I8/)
  WRITE(10,*) ' '
  WRITE(10,*) 'Note: only if IERR = 0, '
  WRITE(10,*) '          is eigen analysis successful!!'
END DO

return

END

SUBROUTINE EIGRG(NM,N,A,WR,WI,MATZ,Z,IV1,FV1,IERR,
&              da,CADD,LOC,NZERO)
  IMPLICIT REAL*8(A-H,O-Z)
  DIMENSION A(NM,N),WR(N),WI(N),Z(NM,N),FV1(N),IV1(N),da(*)

```

```

DIMENSION LOC(*)
CALL RG(NM,N,A,WR,WI,MATZ,Z,IV1,FV1,IERR)

small= 10000000000.
locat=0
imag=0
imag0=0

do i = 1, n
  small1=dsqrt(wr(i)*wr(i)+wi(i)*wi(i))
  if(small1.lt.small) then
    locat=i
    small=small1
  end if
end do
LOC(1) = LOCAT
NZERO=1
DO I = 1,N
  IF(I.NE.LOCAT) THEN
    small1=dsqrt(wr(i)*wr(i)+wi(i)*wi(i))
    IF(DABS(SMALL1-SMALL).LT.0.01) THEN
      NZERO=NZERO+1
      LOC(NZERO)=I
    END IF
  END IF
END DO

DO II=1,NZERO
  imag=0
  imag0=0
  LOCAT=LOC(II)
  do i = 1, locat-1
    if(dabs(wi(i)).gt.0.) then
      imag=imag+1
    end if
  end do

  if(dabs(wi(locat)).gt.0.) then
    imag0=1
  end if

  WRITE(10,4) II,WR(locat)
4  FORMAT(2X,'Eigenvalue',I2, ' =',E20.10/)

  WRITE(10,5)
5  FORMAT(2X,'Eigenvector =')

  do icol=1, 1+imag0
    WRITE(10,6) (Z(i,locat+imag-icol+1), i=1,n)
  end do
6  FORMAT(2X,5E20.10)
  WRITE(10,7) IERR
7  FORMAT(2X,'IERR = ',I8/)
c  CALL VECVEC(da,da,dada,N) ! inner product da.da
END DO
c  do iadd=1,N
c  da(iadd)=CADD*Z(iadd,locat+imag-1+1)

```

```

c      end do

return
END

SUBROUTINE EIGRGG(NM,N,A,B,ALFR,ALFI,BETA,MATZ,Z,LOC,Nmin,IERR)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION A(NM,N),B(NM,N),ALFR(N),ALFI(N),BETA(N),Z(NM,N)
dimension LOC(*)

CALL RGG(NM,N,A,B,ALFR,ALFI,BETA,MATZ,Z,IERR)
c      write(10,*) 'alfr',(ALFR(ii),ii=1,n)
c      write(10,*) 'beta',(beta(ii),ii=1,n)
small= 1000000000000000.
locat=0
imag=0
imag0=0
do i = 1,n
    ALFR(i)=ALFR(i)/BETA(i)
    ALFI(i)=ALFI(i)/BETA(i)
end do

write(10,*) 'eigenvalues = '
write(10,*) ' '
write(10,*) (ALFR(ii),ii=1,n)
do i = 1, n
    small1=ALFR(i)
    if(small1.lt.small.and.small1.gt.0.) then
        locat=i
        small=small1
    end if
end do
LOC(1) = LOCAT
Nmin=1
DO I = 1,N
    IF(I.NE.LOCAT) THEN
        small1=ALFR(i)
        IF(DABS(SMALL1-SMALL).LT.0.01) THEN
            Nmin=Nmin+1
            LOC(Nmin)=I
        END IF
    END IF
END DO

return
END

SUBROUTINE DIADET(AST,IPVT,DIAG,MAXEQN,NCHG,NNEGA,NNEGD,DET)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION AST(MAXEQN,MAXEQN),IPVT(MAXEQN),DIAG(MAXEQN)
NNEGA=0
DET = 1.
DO II=1,MAXEQN
    DIAG(II) = AST(II,II)

```

```

      IF(DIAG(II).LT.0.) THEN
        NNEGA= NNEGA+1
      END IF
      DET=DET + DLOG(DABS(DIAG(II)))
    END DO
    idet = IPVT(MAXEQN)*(-1)**NNEGA
    NNEGD=NNEGA-NCHG
    WRITE(10, 5) NNEGA, NCHG, NNEGD
5    FORMAT(2X,'NNEGA =',I3,3X,'NCHG=',I3,3X,'NNEGD=',I3/)

    WRITE(10, *) 'log |DET| =', det, 'the sign of det ', idet

    RETURN
  END

```

C analy5.f:
C

```
      SUBROUTINE ARCSTP(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&
&          LM,MAXH,MHT,
&          X0,XYZN,xnode1,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,aext0,aint,
&          wr,wi,z,loc,fv1,iv1,
&          maxnld,maxldn,maxitr,
&          NLD,ldown,ilast,
&          toltld,tolda,toldq,toldadq,cadd,
&          alamb0,alinc0,det0,rdet,nodepr,ip)

      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION LM(*),MAXH(*),MHT(*)
      DIMENSION X0(*),XYZN(*),xnode1(*),MELM(*),NODBC(NTNODE,*)
      DIMENSION mhing1(2,*),mhing3(2,*)
      DIMENSION hing1(5,*),idofh1(*)
      DIMENSION CNCM(6,4,*)
      DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
      DIMENSION qext0(*),qint(*)
      DIMENSION da(*),dq(*),da0(*),dq0(*)
      DIMENSION sda(*),sdq(*),sxnode(*)
      DIMENSION ttrg(*),t6rg(*),omegag(*),om6gag(*)
      DIMENSION sttrg(*),st6rg(*),somegg(*),som6gg(*)
      DIMENSION AST(*),AST1(*),diag(*),IPVT(*),WORK(*)
      DIMENSION ATOTL(*),ATOTL0(*),aext0(*),aint(*)
      DIMENSION aexw0(900),dar(900),ATOTL1(900),ATOTL2(900)
      DIMENSION darw(900), detdat(3),aaccbb(3)
      DIMENSION WR(*),WI(*),Z(MAXEQN,*),LOC(*)
      DIMENSION FV1(*),IV1(*)

      do ii = 1,3
         detdat(ii)=0.
         aaccbb(ii)=0.
      end do

      Npr=1
      nldc=0
      ldown=0
      niter=0
      idesir=6
      if(ip.eq.2) then
         idesir=7
      end if
      idet0 = 1
      alambd=alamb0
```



```

CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)

18   Npr=Npr+1
      niter=0
      NLD=NLD+1

      IF(NLD.EQ.maxnld) then
        STOP 'MAXIMUM NUMBER OF LOAD STEPS EXCEEDED'
      END IF

      IF(dabs(alamb0).GT.toltld) then
        stop 'MAXIMUM LOAD FACTOR EXCEEDED'
      END IF

c     write(6,*) '      '
c     write(6,*) 'load level NLD=', nld, '  alambd =',alambd
c     write(6,*) '      '

      CALL CLVEC(AST,NWK)
      CALL CLVEC(dq0,MAX)

      MAXEQN=MAX-1

C
C   ASSEMBLY THE TANGENT STIFFNESS MATRIX: AST
C
C
63   continue

      CALL CLVEC(AST,NWK)
      CALL CLVEC(dq0,MAX)

      LCODE = 1
      CALL ASSEMB(NTNODE,NELM,melm,x0,xnodel,cncm,NODBC,MAXEQN,
&              nhing1,nhing3,
&              mhing1,mhing3,
&              hing1,idofh1,
&              distgg,distnn,distpp,ptload,alambd,
&              ttrg,t6rg,omegag,om6gag,da,
&              LM,MAXH,MHT,NWK,MK,
&              AST,qint,qext0,LCODE)

      DO IASSIGN = 1, NWK
        AST1(IASSIGN)=AST(IASSIGN)
      END DO

C
C   DECOMPOSITION OF TANGENT STIFFNESS MATRIX
C
      CALL DECOMP1(MAXEQN,MAXEQN,AST,COND,IPVT,NCHG,WORK)

C
C   ASSEMBLY THE INTITIAL UNBALANCED LOAD,
C           AT CURRENT LOAD LEVEL
C

```

```

CALL CLVEC(dq,MAX)
LCODE = 2
CALL ASSEMB(NTNODE,NELM,melm,x0,xnode1,cncm,NODBC,MAXEQN,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          distgg,distnn,distpp,ptload,alambd,
&          ttrg,t6rg,omegag,om6gag,da,
&          LM,MAXH,MHT,NWK,MK,
&          AST,qint,qext0,LCODE)

CALL RESIDL(qext0,qint,maxeqn,alambd,dq)

c      write(6,*) 'intitial unbalanced load, external load'
c      write(6,*) 'dq=',(dq(iii),iii=1,maxeqn)
c      write(6,*) 'qext0=',(qext0(iii),iii=1,maxeqn)
c      write(6,*) 'atotl=',(atotl(iii),iii=1,maxeqn)

      if(nld.eq.1) then
        DO ING=1,MAXEQN
          dq0(ING)=dq(ING) ! get the intitial unbalanced LOAD
        END DO
      end if

64      CONTINUE

      ITR=1      ! number of iterations inside the incremental load
loop

C-----SOLVE THE MATRIX EQUATIONS TO GET DISPLACEMENTS
C      DUE TO INTITIAL UNBALANCED LOAD

      DO ING=1,MAXEQN
        aext0(ING)=qext0(ING)
        dar(ING)=dq(ING)
      END DO

      CALL SOLVE1(MAXEQN,MAXEQN,AST,aext0,IPVT)
      CALL SOLVE1(MAXEQN,MAXEQN,AST,dar,IPVT)

C
C      RECOVER daQ
C

      call recdal(aext0,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
      call recdal(dar,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)

c      write(6,*) 'aext0=',(aext0(iii),iii=1,maxeqn)
      DO ING=1,MAXEQN
        aexw0(ING)=aext0(ING)
      END DO

      call aarotv(ntnode,NODBC,atotl,aexw0)
      CALL VECVEC(aexw0,aexw0,aexaex,MAXEQN)

      IF(NLD.EQ.1) THEN
        c3 = 2.

```

```

        if(ip.eq.1) then
            c3=1.01
        end if
        arclen = alinc0*dsqrt(c3*aexaex)
        c2=(c3-1.)*aexaex
        write(6,*) 'CCCCCCC = ', c2

ELSE IF(NLD.GT.1) THEN
    faclim=1.1
    if(ip.eq.2) then
        faclim=1.001
    end if
    fac=dsqrt(dfloat(idesir)/dfloat(ILAST))
    if(fac.ge.faclim) then
        fac = faclim
    end if
    if(fac.lt.0.5) then
        fac = 0.5
    end if
    arclen = fac*arcln0
    if(idet.ne.idet0) then
        idet0 = -idet0
    end if
    alinc0 = dfloat(idet0)*arclen/dsqrt(c3*aexaex)
    c2=(c3-1.)*aexaex
END IF
write(6,*) 'CCCCCCC = ', c2
alinc=alinc0

write(6,*) 'alinc, arclen',alinc, arclen

do ii = 1,maxeqn
    da(ii) = dar(ii)+alinc*aext0(ii)
end do

alambd = alambd+alinc

IF(NLD.EQ.1) THEN
    DO ING=1,MAXEQN
        da0(ING)=da(ING) !store the displ due to intitial
unbalanced LOAD
    END DO
    CALL VECVEC(dq0,dq0,dq0dq0,MAXEQN) ! inner product dq0.dq0
    CALL VECVEC(da0,da0,da0da0,MAXEQN) ! inner product da0.da0
END IF

ITR=1      ! number of iterations inside the incremental load
loop
24      CONTINUE      ! iterations inside a load step start here

        ITR=ITR+1
        niter=niter+1
C      IF(ITR.GT.76) GO TO 28
C
C

```

```

C      UPDATE xnode1, atot1
C
      CALL upxnod(ntnode,nelm,MELM,NODBC,xnode1,da)
      CALL upatot(ntnode,NODBC,atot1,da)
      call thetah(atot1,nhing1,mhing1,hing1,idofh1,NODBC,NTNODE)

      do ii = 1,maxeqn
        atot11(ii)=atot1(ii)
        atot12(ii)=atot1(ii)
      end do
C
C      UPDATE ttrg & omegag
C
      CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
&              x0,da,ttrg,omegag)
      CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
&              x0,da,t6rg,om6gag)
C
C      ASSEMBLY THE UPDATED TANGENT STIFFNESS MATRIX: AST
C
C
      CALL CLVEC(AST,NWK)
      LCODE = 1
      CALL ASSEMB(NTNODE,NELM,melm,x0,xnode1,cncm,NODBC,MAXEQN,
&              nhing1,nhing3,
&              mhing1,mhing3,
&              hing1,idofh1,
&              distgg,distnn,distpp,ptload,alambd,
&              ttrg,t6rg,omegag,om6gag,da,
&              LM,MAXH,MHT,NWK,MK,
&              AST,qint,qext0,LCODE)
      DO IASSIGN = 1, NWK
        AST1(IASSIGN)=AST(IASSIGN)
      END DO
C
C      DECOMPOSITION OF TANGENT STIFFNESS MATRIX
C
      CALL DECOMP1(MAXEQN,MAXEQN,AST,COND,IPVT,NCHG,WORK)
C
C      ASSEMBLY THE unbalanced LOAD for updated configuration
C      at current load level
C
      CALL CLVEC(dq,MAX)
      LCODE = 2
      CALL ASSEMB(NTNODE,NELM,melm,x0,xnode1,cncm,NODBC,MAXEQN,
&              nhing1,nhing3,
&              mhing1,mhing3,
&              hing1,idofh1,
&              distgg,distnn,distpp,ptload,alambd,
&              ttrg,t6rg,omegag,om6gag,da,
&              LM,MAXH,MHT,NWK,MK,
&              AST,qint,qext0,LCODE)
      CALL RESIDL(qext0,qint,maxeqn,alambd,dq)

      DO ING=1,MAXEQN

```

```

        aext0(ING)=qext0(ING)
        dar(ING)=dq(ING)
    END DO

C
C-----SOLVE THE MATRIX EQUATIONS TO GET DISPLACEMENTS under unbalanced
force
C
        CALL SOLVE1(MAXEQN,MAXEQN,AST,aext0,IPVT)
        CALL SOLVE1(MAXEQN,MAXEQN,AST,dar,IPVT)
C
C    RECOVER daQ
C
        call recda1(aext0,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
        call recda1(dar,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
C
C    GET NEW LOADING FACTOR BASED ON ARC LENGTH
C
        do ii = 1, maxeqn
            aexw0(ii) = aext0(ii)
            darw(ii) = dar(ii)
        end do

        call aarotv(ntnode,NODBC,atotl,aexw0)
        call aarotv(ntnode,NODBC,atotl,darw)

        aexaex=0.
        aedain=0.
        dain2=0.

        do ii =1,maxeqn
            aexaex=aexaex+aexw0(ii)*aexw0(ii)
            aedain=aedain+aexw0(ii)
&            *(atotl(ii)-atotl0(ii)+darw(ii))
            dain2=dain2+
&            (atotl(ii)-atotl0(ii)+darw(ii))
&            *(atotl(ii)-atotl0(ii)+darw(ii))
        end do

        aa=aexaex+c2
        bb=2.*aedain+2.*(alambd-alamb0)*c2
        cc=dain2-arclen*arclen+(alambd-alamb0)*(alambd-alamb0)*c2

        crit=bb*bb-4.*aa*cc
        write(6,*) 'crit =', crit

        if(crit.lt.0.) then
            ilast = 100
            write(6,*) 'itr =itr =itr =', itr
            go to 66
        end if

        dalmd1=(-bb+dsqrt(crit))/(2.*aa)

```

```

dalmd2=(-bb-dsqrt(crit))/(2.*aa)

c   write(6,*) 'aa =',aa , 'bb=', bb, 'cc=', cc

   write(6,*) 'dalmd1 =',dalmd1 , 'dalmd2=', dalmd2

   dalda=0.
   da2da=0.

   do ii = 1,maxeqn
     da(ii) = dar(ii)+dalmd1*aext0(ii)
   end do
   call upatot(ntnode,NODBC,atotl1,da)
   do ii = 1,maxeqn
     da(ii) = dar(ii)+dalmd2*aext0(ii)
   end do
   call upatot(ntnode,NODBC,atotl2,da)

   do ii = 1, maxeqn
     dalda = dalda+(atotl1(ii)-atotl0(ii))*(atotl(ii)-atotl0(ii))
     da2da = da2da+(atotl2(ii)-atotl0(ii))*(atotl(ii)-atotl0(ii))
   end do

c   write(6,*) 'dalda  =',dalda , 'da2da  =',da2da

   if(dalda.ge.0.0.and.da2da.ge.0.0) then
     alinc =dalmd2
     if(dabs(dalmd1+cc/bb).lt.dabs(dalmd2+cc/bb)) then
       alinc =dalmd1
     end if
   else if(dalda.gt.0.0.and.da2da.le.0.0) then
     alinc = dalmd1
   else if(da2da.gt.0.0.and.dalda.le.0.0) then
     alinc = dalmd2
   else
     alinc = almd1
   end if

   do ii = 1,maxeqn
     da(ii) = dar(ii)+alinc*aext0(ii)
   end do

   alambd =alambd+alinc
   write(6,*) 'ARC =', arclen, 'alambd=', alambd
   CALL RESIDL(qext0,qint,maxeqn,alambd,dq)

   CALL VECVEC(dq,dq,dqdq,MAXEQN) ! inner product dq.dq

   CALL VECVEC(da,da,dada,MAXEQN) ! inner product da.da

   dq0dq0=1.d0
   da0da0=1.d0

   RATIOL=DSQRT(dqdq/dq0dq0)
   RATIOD=DSQRT(dada/da0da0)

```

```

RATLD=DSQRT(dqdg/dada)
WRITE (16,*) itr,RATIOL
C   WRITE (6,*) 'RATLD=',RATLD

C
C   CONVERGENCE CHECKING FOR LOAD RESIDUAL
C
      IF(RATIOL.LE.toldq) GO TO 28      !CONVERGENCE

C
C   CONVERGENCE CHECKING FOR DISPLACEMENT INCREMENT
C
      IF(RATIOD.LE.tolda) GO TO 28      !CONVERGENCE
c     IF(arclen.LE.tolda) GO TO 28      !CONVERGENCE

C
C   CONVERGENCE CHECKING FOR LOAD RESIDUAL AND DISPLACEMENT INCREMENT
C
C     IF(RATIOD.LE.toldadq.AND.RATIOL.LE.toldadq) GO TO 28
!CONVERGENCE

C
C   CHECK MAXIMUM NUMBER OF ITERATIONS INSIDE OF CURRENT LOAD LOOP
C
      IF(ITR.GT.MAXITR) GO TO 66          ! EXCEED

      GO TO 24                          ! GO TO ANOTHER ITR LOOP

28    CONTINUE                          ! GET HERE IF CONVERGENCE
ACHIEVES.
      nldc=nldc+1
      write(6,*) 'nldc =', nldc
      WRITE(6,*) 'The number of iteration itr =',itr
c     WRITE(6,*) 'dada=',dada, 'dqdg=',dqdg
c     WRITE (6,20003) RATIOD,RATIOL
20003  FORMAT(//2X,'RATIOS: RATIOD =',1PE20.10,'RATIOL =',1PE20.10)
c     WRITE (6,*) 'RATLD=',RATLD

      WRITE(10,*) '
      WRITE(10,*) '=====
      write(10,*) 'load level NLD=', nld, '      alambd=',alambd

C
C
C
      CALL DIADET(AST,IPVT,DIAG,MAXEQN,NCHG,NNEGA,NNEGD,DET)
      idet = IPVT(MAXEQN)*(-1)**NNEGA

      if(nld.eq.1) then
        det0=det
        bbb=alambd
        fbbb=1.
      else if (nld.gt.1) then

        rdet=dfloat(idet)*dexp(det-det0)

```

```

write(10,*) 'Relative Det =', rdet
write(12,*) alambd, rdet
if(ip.eq.2) then
  aaa=bbb
  faaa=fbbb
  bbb=alambd
  fbbb=rdet
  detdat(1) = detdat(2)
  detdat(2) = detdat(3)
  detdat(3) = dsqrt(dsqrt(rdet*rdet))
  aaccbb(1) = aaccbb(2)
  aaccbb(2) = aaccbb(3)
  aaccbb(3) = alambd
  if(nldc.gt.2) then
    if(faaa*fbbb.le.0.) then
      write(10,*) '==== Using Bisection Approach ===== '
      write(10,*) ' '
      write(10,*) '==== Using Bisection Approach ===== '

      call BIFUR2(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&                LM,MAXH,MHT,
&                X0,XYZN,xnode1,melm,NODBC,
&                nhing1,nhing3,
&                mhing1,mhing3,
&                hing1,idofh1,
&                cncm,
&                ptload,distgg,distnn,distpp,
&                qext0,qint,da,dq,da0,dq0,
&                sda,sdq,sxnode,
&                ttrg,t6rg,omegag,om6gag,
&                sttrg,st6rg,somegg,som6gg,
&                AST,AST1,diag,IPVT,WORK,
&                atotl,atotl0,
&                maxnld,maxlnd,maxitr,
&                NLD,lldown,niter,
&                toltld,tolda,toldq,toldadq,
&                alambd,alamb0,det0,rdet,
&                aaa,bbb,faaa,fbbb)

      write(10,*) ' '
      write(10,*) ' '
      write(10,*) '==== Using Bisection Approach ===== '
      write(10,*) ' '
      write(10,*) ' '
      write(10,*) ' bifurcation buclking load factor:',alambd
      nldc =0
      write(12,*) alambd, rdet
      MATZ=2
      CALL EIGRG(MAXEQN,MAXEQN,AST1,WR,WI,MATZ,Z,IV1,FV1,IERR,
&              da,CADD,LOC,Nmin)

DO II=1,Nmin
  imag=0
  imag0=0
  LOCAT=LOC(II)
  do i = 1, locat-1
    if(dabs(WI(i)).gt.0.) then

```



```

        imag=imag+1
    end if
end do

if(dabs(WI(locat)).gt.0.) then
    imag0=1
end if
write(10,*) '      '
write(10,*) '      '
WRITE(10,500)
500  FORMAT(2X,'Buckling Shape=')
write(10,*) '      '
do icol=1, 1+imag0
    do i=1,maxeqn
        FV1(i) = Z(i,locat+imag-icol+1)
    end do
    call recda1(FV1,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
    CALL upxnod(ntnode,nelm,MELM,NODBC,xnode1,FV1)
    CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)
    do i=1,maxeqn
        Z(i,locat+imag-icol+1)=FV1(i)
    end do
end do
do icol=1, 1+imag0
    WRITE(10,600) (Z(i,locat+imag-icol+1), i=1,maxeqn)
end do

600  FORMAT(2X,5E20.10)
WRITE(10,*) '      '
WRITE(10,700) IERR
700  FORMAT(2X,'IERR = ',I8/)
WRITE(10,*) '      '
WRITE(10,*) 'Note: only if IERR = 0, '
WRITE(10,*) '          is eigen analysis successful!!!'
END DO

    return
end if
end if

if(nldc.gt.3) then
    if((detdat(3)-detdat(2)).gt.0.
&      .and.(detdat(1)-detdat(2)).ge.0.
&      .and.dabs(detdat(2)).le.0.5) then
alamb0 = alambd
alambd = alamb0 - 0.25*alinc0
write(10,*) '==== Using 3 Points Interplotion ===== '
write(10,*) '      '
write(10,*) '      '
write(10,*) '==== Using 3 Points Interplotion ===== '
CALL BIFUR3(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&           LM,MAXH,MHT,
&           X0,XYZN,xnode1,melm,NODBC,
&           nhing1,nhing3,
&           mhing1,mhing3,
&           hing1,idofh1,
&           cncm,
&           ptload,distgg,distnn,distpp,

```

```

&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,niter,
&          toltld,tolda,toldq,toldadq,
&          alambd,alamb0,det0,rdet)
write(10,*) '          '
write(10,*) '          '
write(10,*) '==== Using 3 Points Interplotion ===== '
write(10,*) '          '
write(10,*) '          '
write(10,*) 'Bifurcation point is : alambd = ', alambd
          nldc = 0
c          write(12,*) alambd, rdet
MATZ=2
call EIGRG(MAXEQN,MAXEQN,AST1,WR,WI,MATZ,Z,IV1,FV1,IERR,
&          da,CADD,LOC,Nmin)

DO II=1,Nmin
  imag=0
  imag0=0
  LOCAT=LOC(II)
  do i = 1, locat-1
    if(dabs(WI(i)).gt.0.) then
      imag=imag+1
    end if
  end do

  if(dabs(WI(locat)).gt.0.) then
    imag0=1
  end if
  write(10,*) '          '
  write(10,*) '          '
  WRITE(10,5)
5  FORMAT(2X,'Buckling Shape =')
  write(10,*) '          '
  do icol=1, 1+imag0
    do i=1,maxeqn
      FV1(i) = Z(i,locat+imag-icol+1)
    end do
    call recdal(FV1,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
    CALL upxnod(ntnode,nelm,MELM,NODBC,xnodel,FV1)
    CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)
    do i=1,maxeqn
      Z(i,locat+imag-icol+1)=FV1(i)
    end do
  end do
  do icol=1, 1+imag0
    WRITE(10,6) (Z(i,locat+imag-icol+1), i=1,maxeqn)
  end do

6  FORMAT(2X,5E20.10)
  WRITE(10,*) '          '

```

```

WRITE(10,7) IERR
7  FORMAT(2X,'IERR = ',I8/)
WRITE(10,*) ' '
WRITE(10,*) 'Note: only if IERR = 0, '
WRITE(10,*) '      is eigen analysis successful!!'
END DO

return

C
C RECOVER daQ
C
c      call recdal(da,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
C
C UPDATE xnodel, atotl
C
c      CALL upxnod(ntnode,nelm,MELM,NODBC,xnodel,da)
c      CALL upatot(ntnode,NODBC,atotl,da)
c      call thetah(atotl,nhing1,mhing1,hing1,idofh1,NODBC,NTNODE)
c
c      do ii = 1,maxeqn
c          atotl1(ii)=atotl(ii)
c          atotl2(ii)=atotl(ii)
c      end do
C
C UPDATE ttrg & omegag
C
c      CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
c &          x0,da,ttrg,omegag)
c      CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
c &          x0,da,t6rg,om6gag)
c
c          end if
c          end if
c          end if

end if

C
C GET AND PRINT THE NEW NODAL POSITION VECTOR XYZN
C
c      if(npr.eq.1) then
c          write(6,*) 'load level NLD=', nld, '      alambd=',alambd
c          CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)
c          CALL PRDISP(NTNODE,NODBC,atotl,nodepr,alambd)
c
c          npr=0
c      end if

do ii = 1, maxeqn
    atotl0(ii) = atotl(ii)
end do

call tempsv(maxeqn,nelm,
&          xnodel,ttrg,omegag,da,dq,

```

```

&          sxnode, sttrg, somegg, sda, sdq)

call tempsv(maxeqn, nelm,
&          xnodel, t6rg, om6gag, da, dq,
&          sxnode, st6rg, som6gg, sda, sdq)

alamb0 = alambd
arcln0 = arclen
ilast=niter
ldown=0
go to 18          ! go to new load loop

C
C
C
66  CONTINUE
C
C  CONVERGENCE NOT ACHIEVED. REDUCE THE LOAD STEP BY HALF.
C
c    write(6,*) 'Load increment is reduced by half'

    ilast = 100
    ldown=ldown+1
    alambd=alamb0
    arcln0 = arclen
    idet = idet0
    write(6,*) 'Load level is reduced to alambd= ', alambd

    call tempgt(maxeqn, nelm,
&          xnodel, ttrg, omegag, da, dq,
&          sxnode, sttrg, somegg, sda, sdq)
    call tempgt(maxeqn, nelm,
&          xnodel, t6rg, om6gag, da, dq,
&          sxnode, st6rg, som6gg, sda, sdq)
    do ii = 1, maxeqn
      atotl(ii) = atotl0(ii)
    end do
    call thetah(atotl, nhing1, mhing1, hing1, idofh1, NODBC, NTNODE)

    IF(ldown.GT.maxldn) then
      write(6,*) 'ldown = ', ldown
      stop 'no convergence can be achieved'
    end if

    GO TO 18

    END

cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc
c
c  transfer rotational components of aext0 n aint into rotation vector
form
c
c
cccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
ccc
      subroutine aarotv(ntnode, NODBC, atotl, daw)

```

```

IMPLICIT REAL*8 (A-H,O-Z)
dimension atotl(*),daw(*)
dimension dwvec(3),theta(3),dthet(3)
dimension NODBC(ntnode,6)

do ii=1,ntnode

    dwvec(1)=0.
    dwvec(2)=0.
    dwvec(3)=0.

    theta(1)=0.
    theta(2)=0.
    theta(3)=0.

    do jj=1,3
        ndof=NODBC(II,JJ+3)
        if(ndof.ne.0) then
            theta(jj)=atotl(ndof)
            dwvec(jj)= daw(ndof)
        end if
    end do

    call dwthet(dwvec,theta,dthet)

    do jj=1,3
        ndof=NODBC(II,JJ+3)
        if(ndof.ne.0) then
            daw(ndof)=dthet(jj)
        end if
    end do

end do

return
end

```

```

C  analy6.f:
C
      SUBROUTINE LODSTP(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&
&          LM,MAXH,MHT,
&          X0,XYZN,xnode1,melm,NODBC,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          cncm,
&          ptload,distgg,distnn,distpp,
&          qext0,qint,da,dq,da0,dq0,
&          sda,sdq,sxnode,
&          ttrg,t6rg,omegag,om6gag,
&          sttrg,st6rg,somegg,som6gg,
&          AST,AST1,diag,IPVT,WORK,
&          atotl,atotl0,aext0,aint,
&          wr,wi,z,loc,fv1,iv1,
&          maxnld,maxldn,maxitr,
&          NLD,lldown,ilast,
&          toltld,tolda,toldq,toldadq,
&          alamb0,alinc0,det0,rdet,nodepr,ip)
C
C  ip = 1, dont seek the range in which, bifurcation point is
C
C  ip = 2, seek the range in which, bifurcation point is
C

      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION LM(*),MAXH(*),MHT(*)
      DIMENSION X0(*),XYZN(*),xnode1(*),MELM(*),NODBC(NTNODE,*)
      DIMENSION nhing1(2,*),mhing3(2,*)
      DIMENSION hing1(5,*),idofh1(*)
      DIMENSION CNCM(6,4,*)
      DIMENSION PTLOAD(*),DISTGG(*),DISTNN(*),DISTPP(*)
      DIMENSION qext0(*),qint(*)
      DIMENSION da(*),dq(*),da0(*),dq0(*)
      DIMENSION sda(*),sdq(*),sxnode(*)
      DIMENSION ttrg(*),t6rg(*),omegag(*),om6gag(*)
      DIMENSION sttrg(*),st6rg(*),somegg(*),som6gg(*)
      DIMENSION AST(*),AST1(*),diag(*),IPVT(*),WORK(*)
      DIMENSION ATOTL(*),ATOTL0(*),aext0(*),aint(*)
      DIMENSION aexw0(900),dar(900),ATOTL1(900),ATOTL2(900)
      DIMENSION darw(900),detdat(3),aaccbb(3)
      DIMENSION WR(*),WI(*),Z(MAXEQN,*),LOC(*)
      DIMENSION FV1(*),IV1(*)

      do ii = 1,3
         detdat(ii)=0.
         aaccbb(ii)=0.
      end do

      Npr=1
      nldc=0
      lldown=0
      niter=0
      IDEDIR=10
      idet0 = 1

```

```

alambd=alamb0

CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)

18   Npr=Npr+1

      MAXEQN=MAX-1

C
C   ASSEMBLY THE TANGENT STIFFNESS MATRIX: AST
C
C
63   continue

      CALL CLVEC(AST,NWK)
      CALL CLVEC(dq0,MAX)

      LCODE = 1

      CALL ASSEMB(NTNODE,NELM,melm,x0,xnodel,cncm,NODBC,MAXEQN,
&               nhing1,nhing3,
&               mhing1,mhing3,
&               hing1,idofh1,
&               distgg,distnn,distpp,ptload,alambd,
&               ttrg,t6rg,omegag,om6gag,da,
&               LM,MAXH,MHT,NWK,MK,
&               AST,qint,qext0,LCODE)

      DO IASSIGN = 1, NWK
        AST1(IASSIGN)=AST(IASSIGN)
      END DO

C
C   DECOMPOSITION OF TANGENT STIFFNESS MATRIX
C
C
      CALL DECOMP1(MAXEQN,MAXEQN,AST,COND,IPVT,NCHG,WORK)
c     call praskd(ast,maxeqn)
C
C   ASSEMBLY THE INTITIAL UNBALANCED LOAD,
C           AT CURRENT LOAD LEVEL
C
      CALL CLVEC(dq,MAX)
      LCODE = 2
      CALL ASSEMB(NTNODE,NELM,melm,x0,xnodel,cncm,NODBC,MAXEQN,
&               nhing1,nhing3,
&               mhing1,mhing3,
&               hing1,idofh1,
&               distgg,distnn,distpp,ptload,alambd,
&               ttrg,t6rg,omegag,om6gag,da,
&               LM,MAXH,MHT,NWK,MK,
&               AST,qint,qext0,LCODE)

      CALL RESIDL(qext0,qint,maxeqn,alambd,dq)

```

```

c      write(6,*) 'intitial unbalanced load, external load'
c      write(6,*) 'dq=',(dq(iii),iii=1,maxeqn)
c      write(6,*) 'qext0=',(qext0(iii),iii=1,maxeqn)
c      write(6,*) 'atotl=',(atotl(iii),iii=1,maxeqn)

      if(nld.eq.1) then
        DO ING=1,MAXEQN
          dq0(ING)=dq(ING) ! get the intitial unbalanced LOAD
        END DO
      end if

64      CONTINUE

      niter=0
      NLD=NLD+1

      IF(NLD.EQ.maxnld) then
        STOP 'MAXIMUM NUMBER OF LOAD STEPS EXCEEDED'
      END IF

      IF(dabs(alamb0).GT.toltld) then
        stop 'MAXIMUM LOAD FACTOR EXCEEDED'
      END IF

c      write(6,*) ' '
c      write(6,*) 'load level NLD=', nld, ' alambd =',alambd
c      write(6,*) ' '

      ITR=1 ! number of iterations inside the incremental load
loop

C-----SOLVE THE MATRIX EQUATIONS TO GET DISPLACEMENTS
C      DUE TO INTITIAL UNBALANCED LOAD

      DO ING=1,MAXEQN
        aext0(ING)=qext0(ING)
        dar(ING)=dq(ING)
      END DO
c      write(6,*) 'qext0=',(qext0(iii),iii=1,maxeqn)
      CALL SOLVE1(MAXEQN,MAXEQN,AST,aext0,IPVT)
      CALL SOLVE1(MAXEQN,MAXEQN,AST,dar,IPVT)
c      write(6,*) 'aext0=',(aext0(iii),iii=1,maxeqn)
c      write(6,*) 'dar1=',(dar(iii),iii=1,maxeqn)
C
C      RECOVER daQ
C

      call recda1(aext0,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
      call recda1(dar,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
c      write(6,*) 'aext2=',(aext0(iii),iii=1,maxeqn)
c      write(6,*) 'dar2=',(dar(iii),iii=1,maxeqn)
c      IF(NLD.GT.1) THEN

c      fac=dsqrt(dfloat(IDEDIR)/dfloat(ILAST))
c      if(fac.ge.1.0) then

```



```

c          fac = 1.0
c          end if
c          if(fac.lt.0.5) then
c              fac = 0.5
c          end if

c          alinc0 = fac*alinc0

c          END IF

c          alinc=alinc0
c          write(6,*) 'alinc0= ', alinc0
c          do ii = 1,maxeqn
c              da(ii) = dar(ii)+alinc*aext0(ii)
c          end do
c          write(6,*) 'da=',(da(iii),iii=1,maxeqn)
c          alambd = alambd+alinc
c          write(6,*) 'alambd = ',alambd

c          IF(NLD.EQ.1) THEN
c              DO ING=1,MAXEQN
c                  da0(ING)=da(ING) !store the displ due to intitial
unbalanced LOAD
c              END DO
c              CALL VECVEC(dq0,dq0,dq0dq0,MAXEQN) ! inner product dq0.dq0
c              CALL VECVEC(da0,da0,da0da0,MAXEQN) ! inner product da0.da0
c          END IF

c          ITR=1      ! number of iterations inside the incremental load
loop
c          24      CONTINUE      ! iterations inside a load step start here

c              ITR=ITR+1
c              niter=niter+1
c              IF(ITR.GT.76) GO TO 28
c          C
c          C      UPDATE xnodel, atotl
c          C
c              CALL upxnod(ntnode,nelm,MELM,NODBC,xnodel,da)
c              CALL upatot(ntnode,NODBC,atotl,da)

c              call thetah(atotl,nhing1,mhing1,hing1,idofh1,NODBC,NTNODE)
c              write(6,*) 'atotl=',(atotl(iii),iii=1,maxeqn)
c          C
c          C      UPDATE ttrg & omegag
c          C
c              CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
&                  x0,da,ttrg,omegag)
c              CALL ttromg(NTNODE,NELM,MELM,NODBC,max,
&                  x0,da,t6rg,om6gag)
c          C
c          C      ASSEMBLY THE UPDATED TANGENT STIFFNESS MATRIX: AST
c          C
c          C

```

```

CALL CLVEC(AST,NWK)
LCODE = 1
CALL ASSEMB(NTNODE,NELM,melm,x0,xnode1,cncm,NODBC,MAXEQN,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          distgg,distnn,distpp,ptload,alambd,
&          ttrg,t6rg,omegag,om6gag,da,
&          LM,MAXH,MHT,NWK,MK,
&          AST,qint,qext0,LCODE)

DO IASSIGN = 1, NWK
  AST1(IASSIGN)=AST(IASSIGN)
END DO
c   write(6,*) 'qext0=',(qext0(iii),iii=1,maxeqn)
C
C   DECOMPOSITION OF TANGENT STIFFNESS MATRIX
C
      CALL DECOMP1(MAXEQN,MAXEQN,AST,COND,IPVT,NCHG,WORK)

C
C   ASSEMBLY THE unbalanced LOAD for updated configuration
C   at current load level
C
      CALL CLVEC(dq,MAX)
      LCODE = 2
      CALL ASSEMB(NTNODE,NELM,melm,x0,xnode1,cncm,NODBC,MAXEQN,
&          nhing1,nhing3,
&          mhing1,mhing3,
&          hing1,idofh1,
&          distgg,distnn,distpp,ptload,alambd,
&          ttrg,t6rg,omegag,om6gag,da,
&          LM,MAXH,MHT,NWK,MK,
&          AST,qint,qext0,LCODE)
      CALL RESIDL(qext0,qint,maxeqn,alambd,dq)

      DO ING=1,MAXEQN
        dar(ING)=dq(ING)
      END DO
c   write(6,*) 'qext3=',(qext0(iii),iii=1,maxeqn)
c   write(6,*) 'qint3=',(qint(iii),iii=1,maxeqn)
c   write(6,*) 'dq3=',(dar(iii),iii=1,maxeqn)
c
c-----SOLVE THE MATRIX EQUATIONS TO GET DISPLACEMENTS under unbalanced
force
c
      CALL SOLVE1(MAXEQN,MAXEQN,AST,dar,IPVT)

c   write(6,*) 'dar3=',(dar(iii),iii=1,maxeqn)
C
C   RECOVER daQ
C
      call recdal(dar,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)

```

```

do ii = 1,maxeqn
  da(ii) = dar(ii)
end do

CALL VECVEC(dq,dq,dqdq,MAXEQN) ! inner product dq.dq

CALL VECVEC(da,da,dada,MAXEQN) ! inner product da.da

c    dq0dq0=1.d0
c    da0da0=1.d0

RATIOI=DSQRT(dqdq/dq0dq0)
RATIOD=DSQRT(dada/da0da0)
RATLD=DSQRT(dqdq/dada)
WRITE (16,*) itr,RATIOI
C   WRITE (6,*) 'RATLD=',RATLD

C
C   CONVERGENCE CHECKING FOR LOAD RESIDUAL
C
      IF(RATIOI.LE.toldq) GO TO 28      !CONVERGENCE

C
C   CONVERGENCE CHECKING FOR DISPLACEMENT INCREMENT
C
      IF(RATIOD.LE.tolda) GO TO 28      !CONVERGENCE

C
C   CONVERGENCE CHECKING FOR LOAD RESIDUAL AND DISPLACEMENT INCREMENT
C
      IF(RATIOD.LE.toldadq.AND.RATIOI.LE.toldadq) GO TO 28
!CONVERGENCE

C
C   CHECK MAXIMUM NUMBER OF ITERATIONS INSIDE OF CURRENT LOAD LOOP
C
      IF(ITR.GT.MAXITR) GO TO 66          ! EXCEED

      GO TO 24                          ! GO TO ANOTHER ITR LOOP

28    CONTINUE                          ! GET HERE IF CONVERGENCE
ACHIEVES.

      nldc=nldc+1
      WRITE(6,*) 'The number of iteration itr =',itr
c      WRITE(6,*) 'dada=',dada, 'dqdq=',dqdq
c      WRITE (6,20003) RATIOD,RATIOI
20003  FORMAT(//2X,'RATIOS: RATIOD =',1PE20.10,'RATIOI =',1PE20.10)
c      WRITE (6,*) 'RATLD=',RATLD

      WRITE(10,*) ' '
      WRITE(10,*) '===== '
      write(10,*) 'load level NLD=', nld, '      alambd=',alambd

C

```

C
C

```
CALL DIADET(AST,IPVT,DIAG,MAXEQN,NCHG,NNEGA,NNEGD,DET)
idet = IPVT(MAXEQN)*(-1)**NNEGA

if(nld.eq.1) then
  det0=det
  bbb=alambd
  fbbb=1.
else if (nld.gt.1) then
  rdet=dfloat(idet)*dexp(det-det0)

  write(10,*) 'Relative Det =', rdet
  write(12,*) alambd, rdet
  if(ip.eq.2) then
    aaa=bbb
    faaa=fbbb
    bbb=alambd
    fbbb=rdet
    detdat(1) = detdat(2)
    detdat(2) = detdat(3)
    detdat(3) =dsqrt(dsqrt(rdet*rdet))
    if(nldc.gt.2) then
      if(faaa*fbbb.le.0.) then
        write(10,*) '==== Using Bisection Approach ===== '
        write(10,*) '      '
        write(10,*) '==== Using Bisection Approach ===== '
        write(10,*) '      '
        call BIFUR2(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&                LM,MAXH,MHT,
&                X0,XYZN,xnode1,melm,NODBC,
&                nhing1,nhing3,
&                mhing1,mhing3,
&                hing1,idofh1,
&                cncm,
&                ptload,distgg,distnn,distpp,
&                qext0,qint,da,dq,da0,dq0,
&                sda,sdq,sxnode,
&                ttrg,t6rg,omegag,om6gag,
&                sttrg,st6rg,somegg,som6gg,
&                AST,AST1,diag,IPVT,WORK,
&                atotl,atotl0,
&                maxnld,maxldn,maxitr,
&                NLD,lldown,niter,
&                toltld,tolda,toldq,toldadq,
&                alambd,alamb0,det0,rdet,
&                aaa,bbb,faaa,fbbb)

        write(10,*) '==== Using Bisection Approach ===== '
        write(10,*) '      '
        write(10,*) '      '
        write(10,*) ' bifurcation buclking load factor:',alambd
          nldc =0
          write(12,*) alambd, rdet
          MATZ=2
          CALL EIGRG(MAXEQN,MAXEQN,AST1,WR,WI,MATZ,Z,IV1,FV1,IERR,
&                da,CADD,LOC,Nmin)
```

```

DO II=1,Nmin
  imag=0
  imag0=0
  LOCAT=LOC(II)
  do i = 1, locat-1
    if(dabs(WI(i)).gt.0.) then
      imag=imag+1
    end if
  end do

  if(dabs(WI(locat)).gt.0.) then
    imag0=1
  end if
  write(10,*) '      '
  write(10,*) '      '
  WRITE(10,500)
500  FORMAT(2X,'Buckling Shape =')

  do icol=1, 1+imag0
    do i=1,maxeqn
      FV1(i) = Z(i,locat+imag-icol+1)
    end do
    call recda1(FV1,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
    CALL upxnod(ntnode,nelm,MELM,NODBC,xnode1,FV1)
    CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)
    do i=1,maxeqn
      Z(i,locat+imag-icol+1)=FV1(i)
    end do
  end do
  do icol=1, 1+imag0
    WRITE(10,600) (Z(i,locat+imag-icol+1), i=1,maxeqn)
  end do

600  FORMAT(2X,5E20.10)
  write(10,*) '      '
  WRITE(10,700) IERR
700  FORMAT(2X,'IERR = ',I8/)
  WRITE(10,*) '      '
  WRITE(10,*) 'Note: only if IERR = 0, '
  WRITE(10,*) '      is eigen analysis successful!!'
END DO

  return
end if
end if
if(nldc.gt.3) then
  if((detdat(3)-detdat(2)).gt.0.
&    .and.(detdat(1)-detdat(2)).ge.0.
&    .and.dabs(detdat(2)).le.0.1) then
  alamb0 = alambd
  alambd = alamb0 - 0.2*alinc
  write(10,*) '==== Using 3 Points Interplotion ===== '
  CALL BIFUR3(NTNODE,NELM,NWK,MK,MAX,MAXEQN,
&            LM,MAXH,MHT,
&            X0,XYZN,xnode1,melm,NODBC,
&            nhing1,nhing3,

```

```

&             mhing1,mhing3,
&             hing1,idofh1,
&             cncm,
&             ptload,distgg,distnn,distpp,
&             qext0,qint,da,dq,da0,dq0,
&             sda,sdq,sxnode,
&             ttrg,t6rg,omegag,om6gag,
&             sttrg,st6rg,somegg,som6gg,
&             AST,AST1,diag,IPVT,WORK,
&             atotl,atotl0,
&             maxnld,maxlnd,maxitr,
&             NLD,lldown,niter,
&             toltld,tolda,toldq,toldadq,
&             alambd,alamb0,det0,rdet)

write(10,*) '==== Using 3 Points Interplotion ===== '
write(10,*) '      '
write(10,*) '      '
write(10,*) ' bifurcation buclking load factor:',alambd
      nldc =0
      write(12,*) alambd, rdet
      MATZ=2
      CALL EIGRG(MAXEQN,MAXEQN,AST1,WR,WI,MATZ,Z,IV1,FV1,IERR,
&             da,CADD,LOC,Nmin)

DO II=1,Nmin
  imag=0
  imag0=0
  LOCAT=LOC(II)
  do i = 1, locat-1
    if(dabs(WI(i)).gt.0.) then
      imag=imag+1
    end if
  end do

  if(dabs(WI(locat)).gt.0.) then
    imag0=1
  end if
  write(10,*) '      '
  write(10,*) '      '
  WRITE(10,5)
5  FORMAT(2X,'Buckling Shape =')

  do icol=1, 1+imag0
    do i=1,maxeqn
      FV1(i) = Z(i,locat+imag-icol+1)
    end do
    call recdal(FV1,nhing1,mhing1,hing1,idofh1,NODBC,ntnode)
    CALL upxnod(ntnode,nelm,MELM,NODBC,xnode1,FV1)
    CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)
    do i=1,maxeqn
      Z(i,locat+imag-icol+1)=FV1(i)
    end do
  end do
  do icol=1, 1+imag0
    WRITE(10,6) (Z(i,locat+imag-icol+1), i=1,maxeqn)

```

```

        end do

6       FORMAT(2X,5E20.10)
        write(10,*) '      '
        WRITE(10,7) IERR
7       FORMAT(2X,'IERR = ',I8/)
        WRITE(10,*) '      '
        WRITE(10,*) 'Note: only if IERR = 0, '
        WRITE(10,*) '      is eigen analysis successful!!'
        END DO

        return
        end if
        end if
        end if

        end if

C
C   GET AND PRINT THE NEW NODAL POSITION VECTOR XYZN
C
C   if(npr.eq.1) then
        write(6,*) 'load level NLD=', nld, '      alambd=',alambd
        CALL NXYZ(NTNODE,NELM,MELM,XNODEL,XYZN,alambd,itr)
        CALL PRDISP(NTNODE,NODBC,atotl,nodepr,alambd)

        npr=0
c       end if

        do ii = 1, maxeqn
            atotl0(ii) = atotl(ii)
        end do

        call tempsv(maxeqn,nelm,
&                xnode1,ttrg,omegag,da,dq,
&                sxnode,sttrg,somegg,sda,sdq)

        call tempsv(maxeqn,nelm,
&                xnode1,t6rg,om6gag,da,dq,
&                sxnode,st6rg,som6gg,sda,sdq)

        alamb0 = alambd
        ilast=niter
        ldown=0
        go to 64          ! go to new load loop

C
C
C
66      CONTINUE
C
C   CONVERGENCE NOT ACHIEVED. REDUCE THE LOAD STEP BY HALF.
C
        ldown=ldown+1

```

```

alambd=alamb0
alinc0=0.5*alinc0

idet = idet0
write(6,*) 'Load level is reduced to alambd= ', alambd

call tempgt(maxeqn,nelm,
&          xnode1,ttrg,omegag,da,dq,
&          sxnode,sttrg,somegg,sda,sdq)
call tempgt(maxeqn,nelm,
&          xnode1,t6rg,om6gag,da,dq,
&          sxnode,st6rg,som6gg,sda,sdq)
do ii = 1, maxeqn
  atotl(ii) = atotl0(ii)
end do
call thetah(atotl,nhing1,mhing1,hing1,idofh1,NODBC,NTNODE)

IF(ldown.GT.maxldn) then
  write(6,*) 'idown = ', ldown
  stop 'too small a load step!! Limit point maybe here.'
end if

GO TO 18

END

subroutine praskd(ast,maxeqn)
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION Ast(maxeqn,maxeqn)

write(22,*) 'ast2=',(ast(ii,ii), ii=1, maxeqn)

return
end

```



```

C eigenrgrgg.f:
C
C   SUBROUTINE RG(NM,N,A,WR,WI,MATZ,Z,IV1,FV1,IERR)
C
C   INTEGER N,NM,IS1,IS2,IERR,MATZ
C   DOUBLE PRECISION A(NM,N),WR(N),WI(N),Z(NM,N),FV1(N)
C   INTEGER IV1(N)
C
C   THIS SUBROUTINE CALLS THE RECOMMENDED SEQUENCE OF
C   SUBROUTINES FROM THE EIGENSYSTEM SUBROUTINE PACKAGE (EISPACK)
C   TO FIND THE EIGENVALUES AND EIGENVECTORS (IF DESIRED)
C   OF A REAL GENERAL MATRIX.
C
C   ON INPUT
C
C   NM MUST BE SET TO THE ROW DIMENSION OF THE TWO-DIMENSIONAL
C   ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C   DIMENSION STATEMENT.
C
C   N IS THE ORDER OF THE MATRIX A.
C
C   A CONTAINS THE REAL GENERAL MATRIX.
C
C   MATZ IS AN INTEGER VARIABLE SET EQUAL TO ZERO IF
C   ONLY EIGENVALUES ARE DESIRED. OTHERWISE IT IS SET TO
C   ANY NON-ZERO INTEGER FOR BOTH EIGENVALUES AND EIGENVECTORS.
C
C   ON OUTPUT
C
C   WR AND WI CONTAIN THE REAL AND IMAGINARY PARTS,
C   RESPECTIVELY, OF THE EIGENVALUES. COMPLEX CONJUGATE
C   PAIRS OF EIGENVALUES APPEAR CONSECUTIVELY WITH THE
C   EIGENVALUE HAVING THE POSITIVE IMAGINARY PART FIRST.
C
C   Z CONTAINS THE REAL AND IMAGINARY PARTS OF THE EIGENVECTORS
C   IF MATZ IS NOT ZERO. IF THE J-TH EIGENVALUE IS REAL, THE
C   J-TH COLUMN OF Z CONTAINS ITS EIGENVECTOR. IF THE J-TH
C   EIGENVALUE IS COMPLEX WITH POSITIVE IMAGINARY PART, THE
C   J-TH AND (J+1)-TH COLUMNS OF Z CONTAIN THE REAL AND
C   IMAGINARY PARTS OF ITS EIGENVECTOR. THE CONJUGATE OF THIS
C   VECTOR IS THE EIGENVECTOR FOR THE CONJUGATE EIGENVALUE.
C
C   IERR IS AN INTEGER OUTPUT VARIABLE SET EQUAL TO AN ERROR
C   COMPLETION CODE DESCRIBED IN THE DOCUMENTATION FOR HQR
C   AND HQR2. THE NORMAL COMPLETION CODE IS ZERO.
C
C   IV1 AND FV1 ARE TEMPORARY STORAGE ARRAYS.
C
C   QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARROW,
C   MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C   THIS VERSION DATED AUGUST 1983.
C
C   -----
C
C   IF (N .LE. NM) GO TO 10

```

```

IERR = 10 * N
GO TO 50
C
10 CALL BALANC(NM,N,A,IS1,IS2,FV1)
CALL ELMHES(NM,N,IS1,IS2,A,IV1)
IF (MATZ .NE. 0) GO TO 20
C
..... FIND EIGENVALUES ONLY .....
CALL HQR(NM,N,IS1,IS2,A,WR,WI,IERR)
GO TO 50
C
..... FIND BOTH EIGENVALUES AND EIGENVECTORS .....
20 CALL ELTRAN(NM,N,IS1,IS2,A,IV1,Z)
CALL HQR2(NM,N,IS1,IS2,A,WR,WI,Z,IERR)
IF (IERR .NE. 0) GO TO 50
CALL BALBAK(NM,N,IS1,IS2,FV1,N,Z)
50 RETURN
END

SUBROUTINE BALANC(NM,N,A,LOW,IGH,SCALE)
C
INTEGER I,J,K,L,M,N,JJ,NM,IGH,LOW,IEXC
DOUBLE PRECISION A(NM,N),SCALE(N)
DOUBLE PRECISION C,F,G,R,S,B2,RADIX
LOGICAL NOCONV
C
THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE BALANCE,
C
NUM. MATH. 13, 293-304(1969) BY PARLETT AND REINSCH.
C
HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 315-326(1971).
C
C
THIS SUBROUTINE BALANCES A REAL MATRIX AND ISOLATES
C
EIGENVALUES WHENEVER POSSIBLE.
C
C
ON INPUT
C
C
NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C
ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C
DIMENSION STATEMENT.
C
C
N IS THE ORDER OF THE MATRIX.
C
C
A CONTAINS THE INPUT MATRIX TO BE BALANCED.
C
C
ON OUTPUT
C
C
A CONTAINS THE BALANCED MATRIX.
C
C
LOW AND IGH ARE TWO INTEGERS SUCH THAT A(I,J)
C
IS EQUAL TO ZERO IF
C
(1) I IS GREATER THAN J AND
C
(2) J=1,...,LOW-1 OR I=IGH+1,...,N.
C
C
SCALE CONTAINS INFORMATION DETERMINING THE
C
PERMUTATIONS AND SCALING FACTORS USED.
C
C
SUPPOSE THAT THE PRINCIPAL SUBMATRIX IN ROWS LOW THROUGH IGH
C
HAS BEEN BALANCED, THAT P(J) DENOTES THE INDEX INTERCHANGED
C
WITH J DURING THE PERMUTATION STEP, AND THAT THE ELEMENTS
C
OF THE DIAGONAL MATRIX USED ARE DENOTED BY D(I,J). THEN

```

```

C      SCALE(J) = P(J),      FOR J = 1,...,LOW-1
C                = D(J,J),      J = LOW,...,IGH
C                = P(J)        J = IGH+1,...,N.
C      THE ORDER IN WHICH THE INTERCHANGES ARE MADE IS N TO IGH+1,
C      THEN 1 TO LOW-1.
C
C      NOTE THAT 1 IS RETURNED FOR IGH IF IGH IS ZERO FORMALLY.
C
C      THE ALGOL PROCEDURE EXC CONTAINED IN BALANCE APPEARS IN
C      BALANC IN LINE. (NOTE THAT THE ALGOL ROLES OF IDENTIFIERS
C      K,L HAVE BEEN REVERSED.)
C
C      QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C      MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C      THIS VERSION DATED AUGUST 1983.
C
C      -----
C
C      RADIX = 16.0D0
C
C      B2 = RADIX * RADIX
C      K = 1
C      L = N
C      GO TO 100
C
C      ..... IN-LINE PROCEDURE FOR ROW AND
C      COLUMN EXCHANGE .....
100 SCALE(M) = J
    IF (J .EQ. M) GO TO 50
C
    DO 30 I = 1, L
      F = A(I,J)
      A(I,J) = A(I,M)
      A(I,M) = F
30 CONTINUE
C
    DO 40 I = K, N
      F = A(J,I)
      A(J,I) = A(M,I)
      A(M,I) = F
40 CONTINUE
C
50 GO TO (80,130), IEXC
C      ..... SEARCH FOR ROWS ISOLATING AN EIGENVALUE
C      AND PUSH THEM DOWN .....
80 IF (L .EQ. 1) GO TO 280
    L = L - 1
C      ..... FOR J=L STEP -1 UNTIL 1 DO -- .....
100 DO 120 JJ = 1, L
      J = L + 1 - JJ
C
      DO 110 I = 1, L
        IF (I .EQ. J) GO TO 110
        IF (A(J,I) .NE. 0.0D0) GO TO 120
110 CONTINUE
C

```

```

        M = L
        IEXC = 1
        GO TO 20
120 CONTINUE
C
        GO TO 140
C      ..... SEARCH FOR COLUMNS ISOLATING AN EIGENVALUE
C      AND PUSH THEM LEFT .....
130 K = K + 1
C
140 DO 170 J = K, L
C
        DO 150 I = K, L
            IF (I .EQ. J) GO TO 150
            IF (A(I,J) .NE. 0.0D0) GO TO 170
150 CONTINUE
C
        M = K
        IEXC = 2
        GO TO 20
170 CONTINUE
C      ..... NOW BALANCE THE SUBMATRIX IN ROWS K TO L .....
        DO 180 I = K, L
180 SCALE(I) = 1.0D0
C      ..... ITERATIVE LOOP FOR NORM REDUCTION .....
190 NOCONV = .FALSE.
C
        DO 270 I = K, L
            C = 0.0D0
            R = 0.0D0
C
            DO 200 J = K, L
                IF (J .EQ. I) GO TO 200
                C = C + DABS(A(J,I))
                R = R + DABS(A(I,J))
200 CONTINUE
C      ..... GUARD AGAINST ZERO C OR R DUE TO UNDERFLOW .....
            IF (C .EQ. 0.0D0 .OR. R .EQ. 0.0D0) GO TO 270
            G = R / RADIX
            F = 1.0D0
            S = C + R
210 IF (C .GE. G) GO TO 220
            F = F * RADIX
            C = C * B2
            GO TO 210
220 G = R * RADIX
230 IF (C .LT. G) GO TO 240
            F = F / RADIX
            C = C / B2
            GO TO 230
C      ..... NOW BALANCE .....
240 IF ((C + R) / F .GE. 0.95D0 * S) GO TO 270
            G = 1.0D0 / F
            SCALE(I) = SCALE(I) * F
            NOCONV = .TRUE.
C
            DO 250 J = K, N

```

```

250   A(I,J) = A(I,J) * G
C
      DO 260 J = 1, L
260   A(J,I) = A(J,I) * F
C
270 CONTINUE
C
      IF (NOCONV) GO TO 190
C
280 LOW = K
      IGH = L
      RETURN
      END

      SUBROUTINE ELMHES(NM,N,LOW,IGH,A,INT)
C
      INTEGER I,J,M,N,LA,NM,IGH,KP1,LOW,MM1,MP1
      DOUBLE PRECISION A(NM,N)
      DOUBLE PRECISION X,Y
      INTEGER INT(IGH)
C
      THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE ELMHES,
      NUM. MATH. 12, 349-368(1968) BY MARTIN AND WILKINSON.
      HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 339-358(1971).
C
      GIVEN A REAL GENERAL MATRIX, THIS SUBROUTINE
      REDUCES A SUBMATRIX SITUATED IN ROWS AND COLUMNS
      LOW THROUGH IGH TO UPPER HESSENBERG FORM BY
      STABILIZED ELEMENTARY SIMILARITY TRANSFORMATIONS.
C
      ON INPUT
C
      NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
      ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
      DIMENSION STATEMENT.
C
      N IS THE ORDER OF THE MATRIX.
C
      LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING
      SUBROUTINE BALANC. IF BALANC HAS NOT BEEN USED,
      SET LOW=1, IGH=N.
C
      A CONTAINS THE INPUT MATRIX.
C
      ON OUTPUT
C
      A CONTAINS THE HESSENBERG MATRIX. THE MULTIPLIERS
      WHICH WERE USED IN THE REDUCTION ARE STORED IN THE
      REMAINING TRIANGLE UNDER THE HESSENBERG MATRIX.
C
      INT CONTAINS INFORMATION ON THE ROWS AND COLUMNS
      INTERCHANGED IN THE REDUCTION.
      ONLY ELEMENTS LOW THROUGH IGH ARE USED.
C
      QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
      MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C

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C      THIS VERSION DATED AUGUST 1983.
C
C      -----
C
C      LA = IGH - 1
C      KP1 = LOW + 1
C      IF (LA .LT. KP1) GO TO 200
C
C      DO 180 M = KP1, LA
C          MM1 = M - 1
C          X = 0.0D0
C          I = M
C
C          DO 100 J = M, IGH
C              IF (DABS(A(J,MM1)) .LE. DABS(X)) GO TO 100
C              X = A(J,MM1)
C              I = J
100      CONTINUE
C
C          INT(M) = I
C          IF (I .EQ. M) GO TO 130
C      ..... INTERCHANGE ROWS AND COLUMNS OF A .....
C          DO 110 J = MM1, N
C              Y = A(I,J)
C              A(I,J) = A(M,J)
C              A(M,J) = Y
110      CONTINUE
C
C          DO 120 J = 1, IGH
C              Y = A(J,I)
C              A(J,I) = A(J,M)
C              A(J,M) = Y
120      CONTINUE
C      ..... END INTERCHANGE .....
130      IF (X .EQ. 0.0D0) GO TO 180
C          MP1 = M + 1
C
C          DO 160 I = MP1, IGH
C              Y = A(I,MM1)
C              IF (Y .EQ. 0.0D0) GO TO 160
C              Y = Y / X
C              A(I,MM1) = Y
C
C          DO 140 J = M, N
140          A(I,J) = A(I,J) - Y * A(M,J)
C
C          DO 150 J = 1, IGH
150          A(J,M) = A(J,M) + Y * A(J,I)
C
160      CONTINUE
C
180      CONTINUE
C
200      RETURN
C          END
C          SUBROUTINE ELTRAN(NM,N,LOW,IGH,A,INT,Z)

```

```

C
INTEGER I,J,N,KL,MM,MP,NM,IGH,LOW,MP1
DOUBLE PRECISION A(NM,IGH),Z(NM,N)
INTEGER INT(IGH)

C
C THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE ELMTRANS,
C NUM. MATH. 16, 181-204(1970) BY PETERS AND WILKINSON.
C HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 372-395(1971).
C
C THIS SUBROUTINE ACCUMULATES THE STABILIZED ELEMENTARY
C SIMILARITY TRANSFORMATIONS USED IN THE REDUCTION OF A
C REAL GENERAL MATRIX TO UPPER HESSENBERG FORM BY ELMHES.
C
C ON INPUT
C
C NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C DIMENSION STATEMENT.
C
C N IS THE ORDER OF THE MATRIX.
C
C LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING
C SUBROUTINE BALANC. IF BALANC HAS NOT BEEN USED,
C SET LOW=1, IGH=N.
C
C A CONTAINS THE MULTIPLIERS WHICH WERE USED IN THE
C REDUCTION BY ELMHES IN ITS LOWER TRIANGLE
C BELOW THE SUBDIAGONAL.
C
C INT CONTAINS INFORMATION ON THE ROWS AND COLUMNS
C INTERCHANGED IN THE REDUCTION BY ELMHES.
C ONLY ELEMENTS LOW THROUGH IGH ARE USED.
C
C ON OUTPUT
C
C Z CONTAINS THE TRANSFORMATION MATRIX PRODUCED IN THE
C REDUCTION BY ELMHES.
C
C QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C THIS VERSION DATED AUGUST 1983.
C
C -----
C
C ..... INITIALIZE Z TO IDENTITY MATRIX .....
C DO 80 J = 1, N
C
C DO 60 I = 1, N
60 Z(I,J) = 0.0D0
C
C Z(J,J) = 1.0D0
80 CONTINUE
C
C KL = IGH - LOW - 1
C IF (KL .LT. 1) GO TO 200

```

```

C      ..... FOR MP=IGH-1 STEP -1 UNTIL LOW+1 DO -- .....
DO 140 MM = 1, KL
      MP = IGH - MM
      MP1 = MP + 1
C
      DO 100 I = MP1, IGH
100     Z(I,MP) = A(I,MP-1)
C
      I = INT(MP)
      IF (I .EQ. MP) GO TO 140
C
      DO 130 J = MP, IGH
          Z(MP,J) = Z(I,J)
          Z(I,J) = 0.0D0
130     CONTINUE
C
          Z(I,MP) = 1.0D0
140     CONTINUE
C
200     RETURN
      END

SUBROUTINE HQR(NM,N,LOW,IGH,H,WR,WI,IERR)
C
INTEGER I,J,K,L,M,N,EN,LL,MM,NA,NM,IGH,ITN,ITS,LOW,MP2,ENM2,IERR
DOUBLE PRECISION H(NM,N),WR(N),WI(N)
DOUBLE PRECISION P,Q,R,S,T,W,X,Y,ZZ,NORM,TST1,TST2
LOGICAL NOTLAS
C
THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE HQR,
C NUM. MATH. 14, 219-231(1970) BY MARTIN, PETERS, AND WILKINSON.
C HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 359-371(1971).
C
THIS SUBROUTINE FINDS THE EIGENVALUES OF A REAL
C UPPER HESSENBERG MATRIX BY THE QR METHOD.
C
ON INPUT
C
      NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C      ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C      DIMENSION STATEMENT.
C
      N IS THE ORDER OF THE MATRIX.
C
      LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING
C      SUBROUTINE BALANC. IF BALANC HAS NOT BEEN USED,
C      SET LOW=1, IGH=N.
C
      H CONTAINS THE UPPER HESSENBERG MATRIX. INFORMATION ABOUT
C      THE TRANSFORMATIONS USED IN THE REDUCTION TO HESSENBERG
C      FORM BY ELMHES OR ORTHES, IF PERFORMED, IS STORED
C      IN THE REMAINING TRIANGLE UNDER THE HESSENBERG MATRIX.
C
ON OUTPUT
C
      H HAS BEEN DESTROYED. THEREFORE, IT MUST BE SAVED
C      BEFORE CALLING HQR IF SUBSEQUENT CALCULATION AND

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C           BACK TRANSFORMATION OF EIGENVECTORS IS TO BE PERFORMED.
C
C           WR AND WI CONTAIN THE REAL AND IMAGINARY PARTS,
C           RESPECTIVELY, OF THE EIGENVALUES.  THE EIGENVALUES
C           ARE UNORDERED EXCEPT THAT COMPLEX CONJUGATE PAIRS
C           OF VALUES APPEAR CONSECUTIVELY WITH THE EIGENVALUE
C           HAVING THE POSITIVE IMAGINARY PART FIRST.  IF AN
C           ERROR EXIT IS MADE, THE EIGENVALUES SHOULD BE CORRECT
C           FOR INDICES IERR+1,...,N.
C
C           IERR IS SET TO
C           ZERO          FOR NORMAL RETURN,
C           J             IF THE LIMIT OF 30*N ITERATIONS IS EXHAUSTED
C                       WHILE THE J-TH EIGENVALUE IS BEING SOUGHT.
C
C           QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C           MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C           THIS VERSION DATED AUGUST 1983.
C
C           -----
C
C           IERR = 0
C           NORM = 0.0D0
C           K = 1
C           ..... STORE ROOTS ISOLATED BY BALANC
C                   AND COMPUTE MATRIX NORM .....
C           DO 50 I = 1, N
C
C               DO 40 J = K, N
40          NORM = NORM + DABS(H(I,J))
C
C               K = I
C               IF (I .GE. LOW .AND. I .LE. IGH) GO TO 50
C               WR(I) = H(I,I)
C               WI(I) = 0.0D0
50          CONTINUE
C
C           EN = IGH
C           T = 0.0D0
C           ITN = 30*N
C           ..... SEARCH FOR NEXT EIGENVALUES .....
60          IF (EN .LT. LOW) GO TO 1001
C           ITS = 0
C           NA = EN - 1
C           ENM2 = NA - 1
C           ..... LOOK FOR SINGLE SMALL SUB-DIAGONAL ELEMENT
C                   FOR L=EN STEP -1 UNTIL LOW DO -- .....
70          DO 80 LL = LOW, EN
C               L = EN + LOW - LL
C               IF (L .EQ. LOW) GO TO 100
C               S = DABS(H(L-1,L-1)) + DABS(H(L,L))
C               IF (S .EQ. 0.0D0) S = NORM
C               TST1 = S
C               TST2 = TST1 + DABS(H(L,L-1))
C               IF (TST2 .EQ. TST1) GO TO 100

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      80 CONTINUE
C      ..... FORM SHIFT .....
100 X = H(EN,EN)
      IF (L .EQ. EN) GO TO 270
      Y = H(NA,NA)
      W = H(EN,NA) * H(NA,EN)
      IF (L .EQ. NA) GO TO 280
      IF (ITN .EQ. 0) GO TO 1000
      IF (ITS .NE. 10 .AND. ITS .NE. 20) GO TO 130
C      ..... FORM EXCEPTIONAL SHIFT .....
      T = T + X
C
      DO 120 I = LOW, EN
120 H(I,I) = H(I,I) - X
C
      S = DABS(H(EN,NA)) + DABS(H(NA,ENM2))
      X = 0.75D0 * S
      Y = X
      W = -0.4375D0 * S * S
130 ITS = ITS + 1
      ITN = ITN - 1
C      ..... LOOK FOR TWO CONSECUTIVE SMALL
C      SUB-DIAGONAL ELEMENTS.
C      FOR M=EN-2 STEP -1 UNTIL L DO -- .....
      DO 140 MM = L, ENM2
      M = ENM2 + L - MM
      ZZ = H(M,M)
      R = X - ZZ
      S = Y - ZZ
      P = (R * S - W) / H(M+1,M) + H(M,M+1)
      Q = H(M+1,M+1) - ZZ - R - S
      R = H(M+2,M+1)
      S = DABS(P) + DABS(Q) + DABS(R)
      P = P / S
      Q = Q / S
      R = R / S
      IF (M .EQ. L) GO TO 150
      TST1 = DABS(P)*(DABS(H(M-1,M-1)) + DABS(ZZ) +
DABS(H(M+1,M+1)))
      TST2 = TST1 + DABS(H(M,M-1))*(DABS(Q) + DABS(R))
      IF (TST2 .EQ. TST1) GO TO 150
140 CONTINUE
C
150 MP2 = M + 2
C
      DO 160 I = MP2, EN
      H(I,I-2) = 0.0D0
      IF (I .EQ. MP2) GO TO 160
      H(I,I-3) = 0.0D0
160 CONTINUE
C      ..... DOUBLE QR STEP INVOLVING ROWS L TO EN AND
C      COLUMNS M TO EN .....
      DO 260 K = M, NA
      NOTLAS = K .NE. NA
      IF (K .EQ. M) GO TO 170
      P = H(K,K-1)
      Q = H(K+1,K-1)

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R = 0.0D0
IF (NOTLAS) R = H(K+2,K-1)
X = DABS(P) + DABS(Q) + DABS(R)
IF (X .EQ. 0.0D0) GO TO 260
P = P / X
Q = Q / X
R = R / X
170 S = DSIGN(DSQRT(P*P+Q*Q+R*R),P)
IF (K .EQ. M) GO TO 180
H(K,K-1) = -S * X
GO TO 190
180 IF (L .NE. M) H(K,K-1) = -H(K,K-1)
190 P = P + S
X = P / S
Y = Q / S
ZZ = R / S
Q = Q / P
R = R / P
IF (NOTLAS) GO TO 225
C ..... ROW MODIFICATION .....
DO 200 J = K, N
P = H(K,J) + Q * H(K+1,J)
H(K,J) = H(K,J) - P * X
H(K+1,J) = H(K+1,J) - P * Y
200 CONTINUE
C
J = MIN0(EN,K+3)
C ..... COLUMN MODIFICATION .....
DO 210 I = 1, J
P = X * H(I,K) + Y * H(I,K+1)
H(I,K) = H(I,K) - P
H(I,K+1) = H(I,K+1) - P * Q
210 CONTINUE
GO TO 255
225 CONTINUE
C ..... ROW MODIFICATION .....
DO 230 J = K, N
P = H(K,J) + Q * H(K+1,J) + R * H(K+2,J)
H(K,J) = H(K,J) - P * X
H(K+1,J) = H(K+1,J) - P * Y
H(K+2,J) = H(K+2,J) - P * ZZ
230 CONTINUE
C
J = MIN0(EN,K+3)
C ..... COLUMN MODIFICATION .....
DO 240 I = 1, J
P = X * H(I,K) + Y * H(I,K+1) + ZZ * H(I,K+2)
H(I,K) = H(I,K) - P
H(I,K+1) = H(I,K+1) - P * Q
H(I,K+2) = H(I,K+2) - P * R
240 CONTINUE
255 CONTINUE
C
260 CONTINUE
C
GO TO 70
C ..... ONE ROOT FOUND .....

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270 WR(EN) = X + T
    WI(EN) = 0.0D0
    EN = NA
    GO TO 60
C
    ..... TWO ROOTS FOUND .....
280 P = (Y - X) / 2.0D0
    Q = P * P + W
    ZZ = DSQRT(DABS(Q))
    X = X + T
    IF (Q .LT. 0.0D0) GO TO 320
C
    ..... REAL PAIR .....
    ZZ = P + DSIGN(ZZ,P)
    WR(NA) = X + ZZ
    WR(EN) = WR(NA)
    IF (ZZ .NE. 0.0D0) WR(EN) = X - W / ZZ
    WI(NA) = 0.0D0
    WI(EN) = 0.0D0
    GO TO 330
C
    ..... COMPLEX PAIR .....
320 WR(NA) = X + P
    WR(EN) = X + P
    WI(NA) = ZZ
    WI(EN) = -ZZ
330 EN = ENM2
    GO TO 60
C
    ..... SET ERROR -- ALL EIGENVALUES HAVE NOT
C
    CONVERGED AFTER 30*N ITERATIONS .....
1000 IERR = EN
1001 RETURN
    END
    SUBROUTINE HQR2(NM,N,LOW,IGH,H,WR,WI,Z,IERR)
C
    INTEGER I,J,K,L,M,N,EN,II,JJ,LL,MM,NA,NM,NN,
X      IGH,ITN,ITS,LOW,MP2,ENM2,IERR
    DOUBLE PRECISION H(NM,N),WR(N),WI(N),Z(NM,N)
    DOUBLE PRECISION P,Q,R,S,T,W,X,Y,RA,SA,VI,VR,ZZ,NORM,TST1,TST2
    LOGICAL NOTLAS
C
C   THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE HQR2,
C   NUM. MATH. 16, 181-204(1970) BY PETERS AND WILKINSON.
C   HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 372-395(1971).
C
C   THIS SUBROUTINE FINDS THE EIGENVALUES AND EIGENVECTORS
C   OF A REAL UPPER HESSENBERG MATRIX BY THE QR METHOD. THE
C   EIGENVECTORS OF A REAL GENERAL MATRIX CAN ALSO BE FOUND
C   IF ELMHES AND ELTRAN OR ORTHES AND ORTRAN HAVE
C   BEEN USED TO REDUCE THIS GENERAL MATRIX TO HESSENBERG FORM
C   AND TO ACCUMULATE THE SIMILARITY TRANSFORMATIONS.
C
C   ON INPUT
C
C   NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C   ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C   DIMENSION STATEMENT.
C
C   N IS THE ORDER OF THE MATRIX.
C

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```

C      LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING
C      SUBROUTINE BALANC.  IF BALANC HAS NOT BEEN USED,
C      SET LOW=1, IGH=N.
C
C      H CONTAINS THE UPPER HESSENBERG MATRIX.
C
C      Z CONTAINS THE TRANSFORMATION MATRIX PRODUCED BY ELTRAN
C      AFTER THE REDUCTION BY ELMHES, OR BY ORTRAN AFTER THE
C      REDUCTION BY ORTHES, IF PERFORMED.  IF THE EIGENVECTORS
C      OF THE HESSENBERG MATRIX ARE DESIRED, Z MUST CONTAIN THE
C      IDENTITY MATRIX.
C
C      ON OUTPUT
C
C      H HAS BEEN DESTROYED.
C
C      WR AND WI CONTAIN THE REAL AND IMAGINARY PARTS,
C      RESPECTIVELY, OF THE EIGENVALUES.  THE EIGENVALUES
C      ARE UNORDERED EXCEPT THAT COMPLEX CONJUGATE PAIRS
C      OF VALUES APPEAR CONSECUTIVELY WITH THE EIGENVALUE
C      HAVING THE POSITIVE IMAGINARY PART FIRST.  IF AN
C      ERROR EXIT IS MADE, THE EIGENVALUES SHOULD BE CORRECT
C      FOR INDICES IERR+1,...,N.
C
C      Z CONTAINS THE REAL AND IMAGINARY PARTS OF THE EIGENVECTORS.
C      IF THE I-TH EIGENVALUE IS REAL, THE I-TH COLUMN OF Z
C      CONTAINS ITS EIGENVECTOR.  IF THE I-TH EIGENVALUE IS COMPLEX
C      WITH POSITIVE IMAGINARY PART, THE I-TH AND (I+1)-TH
C      COLUMNS OF Z CONTAIN THE REAL AND IMAGINARY PARTS OF ITS
C      EIGENVECTOR.  THE EIGENVECTORS ARE UNNORMALIZED.  IF AN
C      ERROR EXIT IS MADE, NONE OF THE EIGENVECTORS HAS BEEN FOUND.
C
C      IERR IS SET TO
C      ZERO          FOR NORMAL RETURN,
C      J             IF THE LIMIT OF 30*N ITERATIONS IS EXHAUSTED
C                  WHILE THE J-TH EIGENVALUE IS BEING SOUGHT.
C
C      CALLS CDIV FOR COMPLEX DIVISION.
C
C      QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C      MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C      THIS VERSION DATED AUGUST 1983.
C
C      -----
C
C      IERR = 0
C      NORM = 0.0D0
C      K = 1
C      ..... STORE ROOTS ISOLATED BY BALANC
C              AND COMPUTE MATRIX NORM .....
C      DO 50 I = 1, N
C
C          DO 40 J = K, N
40      NORM = NORM + DABS(H(I,J))
C

```

```

        K = I
        IF (I .GE. LOW .AND. I .LE. IGH) GO TO 50
        WR(I) = H(I,I)
        WI(I) = 0.0D0
50 CONTINUE
C
        EN = IGH
        T = 0.0D0
        ITN = 30*N
C
        ..... SEARCH FOR NEXT EIGENVALUES .....
60 IF (EN .LT. LOW) GO TO 340
        ITS = 0
        NA = EN - 1
        ENM2 = NA - 1
C
        ..... LOOK FOR SINGLE SMALL SUB-DIAGONAL ELEMENT
C
        FOR L=EN STEP -1 UNTIL LOW DO -- .....
70 DO 80 LL = LOW, EN
        L = EN + LOW - LL
        IF (L .EQ. LOW) GO TO 100
        S = DABS(H(L-1,L-1)) + DABS(H(L,L))
        IF (S .EQ. 0.0D0) S = NORM
        TST1 = S
        TST2 = TST1 + DABS(H(L,L-1))
        IF (TST2 .EQ. TST1) GO TO 100
80 CONTINUE
C
        ..... FORM SHIFT .....
100 X = H(EN,EN)
        IF (L .EQ. EN) GO TO 270
        Y = H(NA,NA)
        W = H(EN,NA) * H(NA,EN)
        IF (L .EQ. NA) GO TO 280
        IF (ITN .EQ. 0) GO TO 1000
        IF (ITS .NE. 10 .AND. ITS .NE. 20) GO TO 130
C
        ..... FORM EXCEPTIONAL SHIFT .....
        T = T + X
C
        DO 120 I = LOW, EN
120 H(I,I) = H(I,I) - X
C
        S = DABS(H(EN,NA)) + DABS(H(NA,ENM2))
        X = 0.75D0 * S
        Y = X
        W = -0.4375D0 * S * S
130 ITS = ITS + 1
        ITN = ITN - 1
C
        ..... LOOK FOR TWO CONSECUTIVE SMALL
C
        SUB-DIAGONAL ELEMENTS.
C
        FOR M=EN-2 STEP -1 UNTIL L DO -- .....
DO 140 MM = L, ENM2
        M = ENM2 + L - MM
        ZZ = H(M,M)
        R = X - ZZ
        S = Y - ZZ
        P = (R * S - W) / H(M+1,M) + H(M,M+1)
        Q = H(M+1,M+1) - ZZ - R - S
        R = H(M+2,M+1)
        S = DABS(P) + DABS(Q) + DABS(R)

```

```

        P = P / S
        Q = Q / S
        R = R / S
        IF (M .EQ. L) GO TO 150
        TST1 = DABS(P)*(DABS(H(M-1,M-1)) + DABS(ZZ) +
DABS(H(M+1,M+1)))
        TST2 = TST1 + DABS(H(M,M-1))*(DABS(Q) + DABS(R))
        IF (TST2 .EQ. TST1) GO TO 150
140 CONTINUE
C
150 MP2 = M + 2
C
        DO 160 I = MP2, EN
            H(I,I-2) = 0.0D0
            IF (I .EQ. MP2) GO TO 160
            H(I,I-3) = 0.0D0
160 CONTINUE
C
        ..... DOUBLE QR STEP INVOLVING ROWS L TO EN AND
C
        ..... COLUMNS M TO EN .....
        DO 260 K = M, NA
            NOTLAS = K .NE. NA
            IF (K .EQ. M) GO TO 170
            P = H(K,K-1)
            Q = H(K+1,K-1)
            R = 0.0D0
            IF (NOTLAS) R = H(K+2,K-1)
            X = DABS(P) + DABS(Q) + DABS(R)
            IF (X .EQ. 0.0D0) GO TO 260
            P = P / X
            Q = Q / X
            R = R / X
170 S = DSIGN(DSQRT(P*P+Q*Q+R*R),P)
            IF (K .EQ. M) GO TO 180
            H(K,K-1) = -S * X
            GO TO 190
180 IF (L .NE. M) H(K,K-1) = -H(K,K-1)
190 P = P + S
            X = P / S
            Y = Q / S
            ZZ = R / S
            Q = Q / P
            R = R / P
            IF (NOTLAS) GO TO 225
C
        ..... ROW MODIFICATION .....
        DO 200 J = K, N
            P = H(K,J) + Q * H(K+1,J)
            H(K,J) = H(K,J) - P * X
            H(K+1,J) = H(K+1,J) - P * Y
200 CONTINUE
C
            J = MIN0(EN,K+3)
C
        ..... COLUMN MODIFICATION .....
        DO 210 I = 1, J
            P = X * H(I,K) + Y * H(I,K+1)
            H(I,K) = H(I,K) - P
            H(I,K+1) = H(I,K+1) - P * Q
210 CONTINUE

```

```

C      ..... ACCUMULATE TRANSFORMATIONS .....
      DO 220 I = LOW, IGH
        P = X * Z(I,K) + Y * Z(I,K+1)
        Z(I,K) = Z(I,K) - P
        Z(I,K+1) = Z(I,K+1) - P * Q
220    CONTINUE
      GO TO 255
225    CONTINUE
C      ..... ROW MODIFICATION .....
      DO 230 J = K, N
        P = H(K,J) + Q * H(K+1,J) + R * H(K+2,J)
        H(K,J) = H(K,J) - P * X
        H(K+1,J) = H(K+1,J) - P * Y
        H(K+2,J) = H(K+2,J) - P * ZZ
230    CONTINUE
C
      J = MIN0(EN,K+3)
C      ..... COLUMN MODIFICATION .....
      DO 240 I = 1, J
        P = X * H(I,K) + Y * H(I,K+1) + ZZ * H(I,K+2)
        H(I,K) = H(I,K) - P
        H(I,K+1) = H(I,K+1) - P * Q
        H(I,K+2) = H(I,K+2) - P * R
240    CONTINUE
C      ..... ACCUMULATE TRANSFORMATIONS .....
      DO 250 I = LOW, IGH
        P = X * Z(I,K) + Y * Z(I,K+1) + ZZ * Z(I,K+2)
        Z(I,K) = Z(I,K) - P
        Z(I,K+1) = Z(I,K+1) - P * Q
        Z(I,K+2) = Z(I,K+2) - P * R
250    CONTINUE
255    CONTINUE
C
260 CONTINUE
C
      GO TO 70
C      ..... ONE ROOT FOUND .....
270 H(EN,EN) = X + T
      WR(EN) = H(EN,EN)
      WI(EN) = 0.0D0
      EN = NA
      GO TO 60
C      ..... TWO ROOTS FOUND .....
280 P = (Y - X) / 2.0D0
      Q = P * P + W
      ZZ = DSQRT(DABS(Q))
      H(EN,EN) = X + T
      X = H(EN,EN)
      H(NA,NA) = Y + T
      IF (Q .LT. 0.0D0) GO TO 320
C      ..... REAL PAIR .....
      ZZ = P + DSIGN(ZZ,P)
      WR(NA) = X + ZZ
      WR(EN) = WR(NA)
      IF (ZZ .NE. 0.0D0) WR(EN) = X - W / ZZ
      WI(NA) = 0.0D0
      WI(EN) = 0.0D0

```



```

X = H(EN,NA)
S = DABS(X) + DABS(ZZ)
P = X / S
Q = ZZ / S
R = DSQRT(P*P+Q*Q)
P = P / R
Q = Q / R
C ..... ROW MODIFICATION .....
DO 290 J = NA, N
  ZZ = H(NA,J)
  H(NA,J) = Q * ZZ + P * H(EN,J)
  H(EN,J) = Q * H(EN,J) - P * ZZ
290 CONTINUE
C ..... COLUMN MODIFICATION .....
DO 300 I = 1, EN
  ZZ = H(I,NA)
  H(I,NA) = Q * ZZ + P * H(I,EN)
  H(I,EN) = Q * H(I,EN) - P * ZZ
300 CONTINUE
C ..... ACCUMULATE TRANSFORMATIONS .....
DO 310 I = LOW, IGH
  ZZ = Z(I,NA)
  Z(I,NA) = Q * ZZ + P * Z(I,EN)
  Z(I,EN) = Q * Z(I,EN) - P * ZZ
310 CONTINUE
C
GO TO 330
C ..... COMPLEX PAIR .....
320 WR(NA) = X + P
  WR(EN) = X + P
  WI(NA) = ZZ
  WI(EN) = -ZZ
330 EN = ENM2
GO TO 60
C ..... ALL ROOTS FOUND. BACKSUBSTITUTE TO FIND
C ..... VECTORS OF UPPER TRIANGULAR FORM .....
340 IF (NORM .EQ. 0.0D0) GO TO 1001
C ..... FOR EN=N STEP -1 UNTIL 1 DO -- .....
DO 800 NN = 1, N
  EN = N + 1 - NN
  P = WR(EN)
  Q = WI(EN)
  NA = EN - 1
  IF (Q) 710, 600, 800
C ..... REAL VECTOR .....
600 M = EN
  H(EN,EN) = 1.0D0
  IF (NA .EQ. 0) GO TO 800
C ..... FOR I=EN-1 STEP -1 UNTIL 1 DO -- .....
DO 700 II = 1, NA
  I = EN - II
  W = H(I,I) - P
  R = 0.0D0
C
DO 610 J = M, EN
610 R = R + H(I,J) * H(J,EN)
C

```

```

        IF (WI(I) .GE. 0.0D0) GO TO 630
        ZZ = W
        S = R
        GO TO 700
630      M = I
        IF (WI(I) .NE. 0.0D0) GO TO 640
        T = W
        IF (T .NE. 0.0D0) GO TO 635
            TST1 = NORM
            T = TST1
632      T = 0.01D0 * T
            TST2 = NORM + T
            IF (TST2 .GT. TST1) GO TO 632
635      H(I,EN) = -R / T
        GO TO 680
C      ..... SOLVE REAL EQUATIONS .....
640      X = H(I,I+1)
        Y = H(I+1,I)
        Q = (WR(I) - P) * (WR(I) - P) + WI(I) * WI(I)
        T = (X * S - ZZ * R) / Q
        H(I,EN) = T
        IF (DABS(X) .LE. DABS(ZZ)) GO TO 650
        H(I+1,EN) = (-R - W * T) / X
        GO TO 680
650      H(I+1,EN) = (-S - Y * T) / ZZ
C
C      ..... OVERFLOW CONTROL .....
680      T = DABS(H(I,EN))
        IF (T .EQ. 0.0D0) GO TO 700
        TST1 = T
        TST2 = TST1 + 1.0D0/TST1
        IF (TST2 .GT. TST1) GO TO 700
        DO 690 J = I, EN
            H(J,EN) = H(J,EN)/T
690      CONTINUE
C
700      CONTINUE
C      ..... END REAL VECTOR .....
        GO TO 800
C      ..... COMPLEX VECTOR .....
710      M = NA
C      ..... LAST VECTOR COMPONENT CHOSEN IMAGINARY SO THAT
C      EIGENVECTOR MATRIX IS TRIANGULAR .....
        IF (DABS(H(EN,NA)) .LE. DABS(H(NA,EN))) GO TO 720
        H(NA,NA) = Q / H(EN,NA)
        H(NA,EN) = -(H(EN,EN) - P) / H(EN,NA)
        GO TO 730
720      CALL CDIV(0.0D0, -H(NA,EN), H(NA,NA)-P, Q, H(NA,NA), H(NA,EN))
730      H(EN,NA) = 0.0D0
        H(EN,EN) = 1.0D0
        ENM2 = NA - 1
        IF (ENM2 .EQ. 0) GO TO 800
C      ..... FOR I=EN-2 STEP -1 UNTIL 1 DO -- .....
        DO 795 II = 1, ENM2
            I = NA - II
            W = H(I,I) - P
            RA = 0.0D0

```

```

      SA = 0.0D0
C
      DO 760 J = M, EN
          RA = RA + H(I,J) * H(J,NA)
          SA = SA + H(I,J) * H(J,EN)
760      CONTINUE
C
      IF (WI(I) .GE. 0.0D0) GO TO 770
      ZZ = W
      R = RA
      S = SA
      GO TO 795
770      M = I
      IF (WI(I) .NE. 0.0D0) GO TO 780
      CALL CDIV(-RA,-SA,W,Q,H(I,NA),H(I,EN))
      GO TO 790
C
      ..... SOLVE COMPLEX EQUATIONS .....
780      X = H(I,I+1)
          Y = H(I+1,I)
          VR = (WR(I) - P) * (WR(I) - P) + WI(I) * WI(I) - Q * Q
          VI = (WR(I) - P) * 2.0D0 * Q
          IF (VR .NE. 0.0D0 .OR. VI .NE. 0.0D0) GO TO 784
          TST1 = NORM * (DABS(W) + DABS(Q) + DABS(X)
X              + DABS(Y) + DABS(ZZ))
          VR = TST1
783          VR = 0.01D0 * VR
          TST2 = TST1 + VR
          IF (TST2 .GT. TST1) GO TO 783
784      CALL CDIV(X*R-ZZ*RA+Q*SA,X*S-ZZ*SA-Q*RA,VR,VI,
X              H(I,NA),H(I,EN))
          IF (DABS(X) .LE. DABS(ZZ) + DABS(Q)) GO TO 785
          H(I+1,NA) = (-RA - W * H(I,NA) + Q * H(I,EN)) / X
          H(I+1,EN) = (-SA - W * H(I,EN) - Q * H(I,NA)) / X
          GO TO 790
785      CALL CDIV(-R-Y*H(I,NA),-S-Y*H(I,EN),ZZ,Q,
X              H(I+1,NA),H(I+1,EN))
C
C
      ..... OVERFLOW CONTROL .....
790      T = DMAX1(DABS(H(I,NA)), DABS(H(I,EN)))
          IF (T .EQ. 0.0D0) GO TO 795
          TST1 = T
          TST2 = TST1 + 1.0D0/TST1
          IF (TST2 .GT. TST1) GO TO 795
          DO 792 J = I, EN
              H(J,NA) = H(J,NA)/T
              H(J,EN) = H(J,EN)/T
792      CONTINUE
C
795      CONTINUE
C
      ..... END COMPLEX VECTOR .....
800      CONTINUE
C
      ..... END BACK SUBSTITUTION.
C              VECTORS OF ISOLATED ROOTS .....
      DO 840 I = 1, N
          IF (I .GE. LOW .AND. I .LE. IGH) GO TO 840
C
      DO 820 J = I, N

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```

      820      Z(I,J) = H(I,J)
C
      840 CONTINUE
C
      ..... MULTIPLY BY TRANSFORMATION MATRIX TO GIVE
C
      VECTORS OF ORIGINAL FULL MATRIX.
C
      FOR J=N STEP -1 UNTIL LOW DO -- .....
      DO 880 JJ = LOW, N
        J = N + LOW - JJ
        M = MIN0(J,IGH)
C
        DO 880 I = LOW, IGH
          ZZ = 0.0D0
C
          DO 860 K = LOW, M
            860      ZZ = ZZ + Z(I,K) * H(K,J)
C
          Z(I,J) = ZZ
      880 CONTINUE
C
      GO TO 1001
C
      ..... SET ERROR -- ALL EIGENVALUES HAVE NOT
C
      CONVERGED AFTER 30*N ITERATIONS .....
      1000 IERR = EN
      1001 RETURN
      END

      SUBROUTINE BALBAK(NM,N,LOW,IGH,SCALE,M,Z)
C
      INTEGER I,J,K,M,N,II,NM,IGH,LOW
      DOUBLE PRECISION SCALE(N),Z(NM,M)
      DOUBLE PRECISION S
C
      THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE BALBAK,
C
      NUM. MATH. 13, 293-304(1969) BY PARLETT AND REINSCH.
C
      HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 315-326(1971).
C
      THIS SUBROUTINE FORMS THE EIGENVECTORS OF A REAL GENERAL
C
      MATRIX BY BACK TRANSFORMING THOSE OF THE CORRESPONDING
C
      BALANCED MATRIX DETERMINED BY BALANC.
C
      ON INPUT
C
      NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C
      ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C
      DIMENSION STATEMENT.
C
      N IS THE ORDER OF THE MATRIX.
C
      LOW AND IGH ARE INTEGERS DETERMINED BY BALANC.
C
      SCALE CONTAINS INFORMATION DETERMINING THE PERMUTATIONS
C
      AND SCALING FACTORS USED BY BALANC.
C
      M IS THE NUMBER OF COLUMNS OF Z TO BE BACK TRANSFORMED.
C
      Z CONTAINS THE REAL AND IMAGINARY PARTS OF THE EIGEN-
C
      VECTORS TO BE BACK TRANSFORMED IN ITS FIRST M COLUMNS.

```

```

C
C   ON OUTPUT
C
C       Z CONTAINS THE REAL AND IMAGINARY PARTS OF THE
C       TRANSFORMED EIGENVECTORS IN ITS FIRST M COLUMNS.
C
C   QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C   MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C   THIS VERSION DATED AUGUST 1983.
C
C   -----
C
C
C   IF (M .EQ. 0) GO TO 200
C   IF (IGH .EQ. LOW) GO TO 120
C
C   DO 110 I = LOW, IGH
C       S = SCALE(I)
C   ..... LEFT HAND EIGENVECTORS ARE BACK TRANSFORMED
C           IF THE FOREGOING STATEMENT IS REPLACED BY
C           S=1.0D0/SCALE(I). .....
C       DO 100 J = 1, M
100     Z(I,J) = Z(I,J) * S
C
C   110 CONTINUE
C   ..... FOR I=LOW-1 STEP -1 UNTIL 1,
C           IGH+1 STEP 1 UNTIL N DO -- .....
C   120 DO 140 II = 1, N
C       I = II
C       IF (I .GE. LOW .AND. I .LE. IGH) GO TO 140
C       IF (I .LT. LOW) I = LOW - II
C       K = SCALE(I)
C       IF (K .EQ. I) GO TO 140
C
C       DO 130 J = 1, M
C           S = Z(I,J)
C           Z(I,J) = Z(K,J)
C           Z(K,J) = S
130     CONTINUE
C
C   140 CONTINUE
C
C   200 RETURN
C       END
C       SUBROUTINE CDIV(AR,AI,BR,BI,CR,CI)
C       DOUBLE PRECISION AR,AI,BR,BI,CR,CI
C
C       COMPLEX DIVISION, (CR,CI) = (AR,AI)/(BR,BI)
C
C       DOUBLE PRECISION S,ARS,AIS,BRS,BIS
C       S = DABS(BR) + DABS(BI)
C       ARS = AR/S
C       AIS = AI/S
C       BRS = BR/S
C       BIS = BI/S
C       S = BRS**2 + BIS**2

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```

CR = (ARS*BRS + AIS*BIS)/S
CI = (AIS*BRS - ARS*BIS)/S
RETURN
END
SUBROUTINE CSROOT(XR,XI,YR,YI)
DOUBLE PRECISION XR,XI,YR,YI
C
C (YR,YI) = COMPLEX DSQRT(XR,XI)
C BRANCH CHOSEN SO THAT YR .GE. 0.0 AND SIGN(YI) .EQ. SIGN(XI)
C
DOUBLE PRECISION S,TR,TI,PYTHAG
TR = XR
TI = XI
S = DSQRT(0.5D0*(PYTHAG(TR,TI) + DABS(TR)))
IF (TR .GE. 0.0D0) YR = S
IF (TI .LT. 0.0D0) S = -S
IF (TR .LE. 0.0D0) YI = S
IF (TR .LT. 0.0D0) YR = 0.5D0*(TI/YI)
IF (TR .GT. 0.0D0) YI = 0.5D0*(TI/YR)
RETURN
END
DOUBLE PRECISION FUNCTION EPSLON (X)
DOUBLE PRECISION X
C
C ESTIMATE UNIT ROUNDOFF IN QUANTITIES OF SIZE X.
C
DOUBLE PRECISION A,B,C,EPS
C
C THIS PROGRAM SHOULD FUNCTION PROPERLY ON ALL SYSTEMS
C SATISFYING THE FOLLOWING TWO ASSUMPTIONS,
C 1. THE BASE USED IN REPRESENTING FLOATING POINT
C NUMBERS IS NOT A POWER OF THREE.
C 2. THE QUANTITY A IN STATEMENT 10 IS REPRESENTED TO
C THE ACCURACY USED IN FLOATING POINT VARIABLES
C THAT ARE STORED IN MEMORY.
C THE STATEMENT NUMBER 10 AND THE GO TO 10 ARE INTENDED TO
C FORCE OPTIMIZING COMPILERS TO GENERATE CODE SATISFYING
C ASSUMPTION 2.
C UNDER THESE ASSUMPTIONS, IT SHOULD BE TRUE THAT,
C A IS NOT EXACTLY EQUAL TO FOUR-THIRDS,
C B HAS A ZERO FOR ITS LAST BIT OR DIGIT,
C C IS NOT EXACTLY EQUAL TO ONE,
C EPS MEASURES THE SEPARATION OF 1.0 FROM
C THE NEXT LARGER FLOATING POINT NUMBER.
C THE DEVELOPERS OF EISPACK WOULD APPRECIATE BEING INFORMED
C ABOUT ANY SYSTEMS WHERE THESE ASSUMPTIONS DO NOT HOLD.
C
C THIS VERSION DATED 4/6/83.
C
A = 4.0D0/3.0D0
10 B = A - 1.0D0
C = B + B + B
EPS = DABS(C-1.0D0)
IF (EPS .EQ. 0.0D0) GO TO 10
EPSLON = EPS*DABS(X)
RETURN
END

```

```

DOUBLE PRECISION FUNCTION PYTHAG(A,B)
DOUBLE PRECISION A,B
C
C FINDS DSQRT(A**2+B**2) WITHOUT OVERFLOW OR DESTRUCTIVE UNDERFLOW
C
DOUBLE PRECISION P,R,S,T,U
P = DMAX1(DABS(A),DABS(B))
IF (P .EQ. 0.0D0) GO TO 20
R = (DMIN1(DABS(A),DABS(B))/P)**2
10 CONTINUE
    T = 4.0D0 + R
    IF (T .EQ. 4.0D0) GO TO 20
    S = R/T
    U = 1.0D0 + 2.0D0*S
    P = U*P
    R = (S/U)**2 * R
GO TO 10
20 PYTHAG = P
RETURN
END
SUBROUTINE RGG(NM,N,A,B,ALFR,ALFI,BETA,MATZ,Z,IERR)
C
INTEGER NM,NM,IERR,MATZ
DOUBLE PRECISION A(NM,N),B(NM,N),ALFR(N),ALFI(N),BETA(N),Z(NM,N)
LOGICAL TF
C
C THIS SUBROUTINE CALLS THE RECOMMENDED SEQUENCE OF
C SUBROUTINES FROM THE EIGENSYSTEM SUBROUTINE PACKAGE (EISPACK)
C TO FIND THE EIGENVALUES AND EIGENVECTORS (IF DESIRED)
C FOR THE REAL GENERAL GENERALIZED EIGENPROBLEM  $AX = (\text{LAMBDA})BX$ .
C
C ON INPUT
C
C NM MUST BE SET TO THE ROW DIMENSION OF THE TWO-DIMENSIONAL
C ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C DIMENSION STATEMENT.
C
C N IS THE ORDER OF THE MATRICES A AND B.
C
C A CONTAINS A REAL GENERAL MATRIX.
C
C B CONTAINS A REAL GENERAL MATRIX.
C
C MATZ IS AN INTEGER VARIABLE SET EQUAL TO ZERO IF
C ONLY EIGENVALUES ARE DESIRED. OTHERWISE IT IS SET TO
C ANY NON-ZERO INTEGER FOR BOTH EIGENVALUES AND EIGENVECTORS.
C
C ON OUTPUT
C
C ALFR AND ALFI CONTAIN THE REAL AND IMAGINARY PARTS,
C RESPECTIVELY, OF THE NUMERATORS OF THE EIGENVALUES.
C
C BETA CONTAINS THE DENOMINATORS OF THE EIGENVALUES,
C WHICH ARE THUS GIVEN BY THE RATIOS  $(\text{ALFR}+I*\text{ALFI})/\text{BETA}$ .
C COMPLEX CONJUGATE PAIRS OF EIGENVALUES APPEAR CONSECUTIVELY
C WITH THE EIGENVALUE HAVING THE POSITIVE IMAGINARY PART FIRST.
C

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C      Z  CONTAINS THE REAL AND IMAGINARY PARTS OF THE EIGENVECTORS
C      IF MATZ IS NOT ZERO.  IF THE J-TH EIGENVALUE IS REAL, THE
C      J-TH COLUMN OF Z  CONTAINS ITS EIGENVECTOR.  IF THE J-TH
C      EIGENVALUE IS COMPLEX WITH POSITIVE IMAGINARY PART, THE
C      J-TH AND (J+1)-TH COLUMNS OF Z  CONTAIN THE REAL AND
C      IMAGINARY PARTS OF ITS EIGENVECTOR.  THE CONJUGATE OF THIS
C      VECTOR IS THE EIGENVECTOR FOR THE CONJUGATE EIGENVALUE.
C
C      IERR  IS AN INTEGER OUTPUT VARIABLE SET EQUAL TO AN ERROR
C      COMPLETION CODE DESCRIBED IN THE DOCUMENTATION FOR QZIT.
C      THE NORMAL COMPLETION CODE IS ZERO.
C
C      QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C      MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C      THIS VERSION DATED AUGUST 1983.
C
C      -----
C
C      IF (N .LE. NM) GO TO 10
C      IERR = 10 * N
C      GO TO 50
C
C 10 IF (MATZ .NE. 0) GO TO 20
C      ..... FIND EIGENVALUES ONLY .....
C      TF = .FALSE.
C      CALL QZHES(NM,N,A,B,TF,Z)
C      CALL QZIT(NM,N,A,B,0.0D0,TF,Z,IERR)
C      CALL QZVAL(NM,N,A,B,ALFR,ALFI,BETA,TF,Z)
C      GO TO 50
C      ..... FIND BOTH EIGENVALUES AND EIGENVECTORS .....
C 20 TF = .TRUE.
C      CALL QZHES(NM,N,A,B,TF,Z)
C      CALL QZIT(NM,N,A,B,0.0D0,TF,Z,IERR)
C      CALL QZVAL(NM,N,A,B,ALFR,ALFI,BETA,TF,Z)
C      IF (IERR .NE. 0) GO TO 50
C      CALL QZVEC(NM,N,A,B,ALFR,ALFI,BETA,Z)
C 50 RETURN
C      END
C
C      SUBROUTINE QZHES(NM,N,A,B,MATZ,Z)
C
C      INTEGER I,J,K,L,N,LB,L1,NM,NK1,NM1,NM2
C      DOUBLE PRECISION A(NM,N),B(NM,N),Z(NM,N)
C      DOUBLE PRECISION R,S,T,U1,U2,V1,V2,RHO
C      LOGICAL MATZ
C
C      THIS SUBROUTINE IS THE FIRST STEP OF THE QZ ALGORITHM
C      FOR SOLVING GENERALIZED MATRIX EIGENVALUE PROBLEMS,
C      SIAM J. NUMER. ANAL. 10, 241-256(1973) BY MOLER AND STEWART.
C
C      THIS SUBROUTINE ACCEPTS A PAIR OF REAL GENERAL MATRICES AND
C      REDUCES ONE OF THEM TO UPPER HESSENBERG FORM AND THE OTHER
C      TO UPPER TRIANGULAR FORM USING ORTHOGONAL TRANSFORMATIONS.
C      IT IS USUALLY FOLLOWED BY QZIT, QZVAL AND, POSSIBLY, QZVEC.
C

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```

C      ON INPUT
C
C      NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C      ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C      DIMENSION STATEMENT.
C
C      N IS THE ORDER OF THE MATRICES.
C
C      A CONTAINS A REAL GENERAL MATRIX.
C
C      B CONTAINS A REAL GENERAL MATRIX.
C
C      MATZ SHOULD BE SET TO .TRUE. IF THE RIGHT HAND TRANSFORMATIONS
C      ARE TO BE ACCUMULATED FOR LATER USE IN COMPUTING
C      EIGENVECTORS, AND TO .FALSE. OTHERWISE.
C
C      ON OUTPUT
C
C      A HAS BEEN REDUCED TO UPPER HESSENBERG FORM.  THE ELEMENTS
C      BELOW THE FIRST SUBDIAGONAL HAVE BEEN SET TO ZERO.
C
C      B HAS BEEN REDUCED TO UPPER TRIANGULAR FORM.  THE ELEMENTS
C      BELOW THE MAIN DIAGONAL HAVE BEEN SET TO ZERO.
C
C      Z CONTAINS THE PRODUCT OF THE RIGHT HAND TRANSFORMATIONS IF
C      MATZ HAS BEEN SET TO .TRUE.  OTHERWISE, Z IS NOT REFERENCED.
C
C      QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C      MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C      THIS VERSION DATED AUGUST 1983.
C
C      -----
C
C      ..... INITIALIZE Z .....
C      IF (.NOT. MATZ) GO TO 10
C
C      DO 3 J = 1, N
C
C          DO 2 I = 1, N
C              Z(I,J) = 0.0D0
C      2  CONTINUE
C
C          Z(J,J) = 1.0D0
C      3  CONTINUE
C
C      ..... REDUCE B TO UPPER TRIANGULAR FORM .....
C      10 IF (N .LE. 1) GO TO 170
C          NM1 = N - 1
C
C          DO 100 L = 1, NM1
C              L1 = L + 1
C              S = 0.0D0
C
C              DO 20 I = L1, N
C                  S = S + DABS(B(I,L))
C      20  CONTINUE

```

```

C      IF (S .EQ. 0.0D0) GO TO 100
      S = S + DABS(B(L,L))
      R = 0.0D0
C
      DO 25 I = L, N
        B(I,L) = B(I,L) / S
        R = R + B(I,L)**2
25     CONTINUE
C
      R = DSIGN(DSQRT(R),B(L,L))
      B(L,L) = B(L,L) + R
      RHO = R * B(L,L)
C
      DO 50 J = L1, N
        T = 0.0D0
C
        DO 30 I = L, N
          T = T + B(I,L) * B(I,J)
30     CONTINUE
C
        T = -T / RHO
C
        DO 40 I = L, N
          B(I,J) = B(I,J) + T * B(I,L)
40     CONTINUE
C
50     CONTINUE
C
      DO 80 J = 1, N
        T = 0.0D0
C
        DO 60 I = L, N
          T = T + B(I,L) * A(I,J)
60     CONTINUE
C
        T = -T / RHO
C
        DO 70 I = L, N
          A(I,J) = A(I,J) + T * B(I,L)
70     CONTINUE
C
80     CONTINUE
C
      B(L,L) = -S * R
C
      DO 90 I = L1, N
        B(I,L) = 0.0D0
90     CONTINUE
C
100    CONTINUE
C      ..... REDUCE A TO UPPER HESSENBERG FORM, WHILE
C      ..... KEEPING B TRIANGULAR .....
      IF (N .EQ. 2) GO TO 170
      NM2 = N - 2
C
      DO 160 K = 1, NM2

```

```

      NK1 = NM1 - K
C     ..... FOR L=N-1 STEP -1 UNTIL K+1 DO -- .....
      DO 150 LB = 1, NK1
        L = N - LB
        L1 = L + 1
C     ..... ZERO A(L+1,K) .....
        S = DABS(A(L,K)) + DABS(A(L1,K))
        IF (S .EQ. 0.0D0) GO TO 150
        U1 = A(L,K) / S
        U2 = A(L1,K) / S
        R = DSIGN(DSQRT(U1*U1+U2*U2),U1)
        V1 = -(U1 + R) / R
        V2 = -U2 / R
        U2 = V2 / V1
C
        DO 110 J = K, N
          T = A(L,J) + U2 * A(L1,J)
          A(L,J) = A(L,J) + T * V1
          A(L1,J) = A(L1,J) + T * V2
110      CONTINUE
C
        A(L1,K) = 0.0D0
C
        DO 120 J = L, N
          T = B(L,J) + U2 * B(L1,J)
          B(L,J) = B(L,J) + T * V1
          B(L1,J) = B(L1,J) + T * V2
120      CONTINUE
C     ..... ZERO B(L+1,L) .....
        S = DABS(B(L1,L1)) + DABS(B(L1,L))
        IF (S .EQ. 0.0D0) GO TO 150
        U1 = B(L1,L1) / S
        U2 = B(L1,L) / S
        R = DSIGN(DSQRT(U1*U1+U2*U2),U1)
        V1 = -(U1 + R) / R
        V2 = -U2 / R
        U2 = V2 / V1
C
        DO 130 I = 1, L1
          T = B(I,L1) + U2 * B(I,L)
          B(I,L1) = B(I,L1) + T * V1
          B(I,L) = B(I,L) + T * V2
130      CONTINUE
C
        B(L1,L) = 0.0D0
C
        DO 140 I = 1, N
          T = A(I,L1) + U2 * A(I,L)
          A(I,L1) = A(I,L1) + T * V1
          A(I,L) = A(I,L) + T * V2
140      CONTINUE
C
        IF (.NOT. MATZ) GO TO 150
C
        DO 145 I = 1, N
          T = Z(I,L1) + U2 * Z(I,L)
          Z(I,L1) = Z(I,L1) + T * V1

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          Z(I,L) = Z(I,L) + T * V2
145      CONTINUE
C
150      CONTINUE
C
160 CONTINUE
C
170 RETURN
      END

      SUBROUTINE QZIT(NM,N,A,B,EPS1,MATZ,Z,IERR)
C
      INTEGER I,J,K,L,N,EN,K1,K2,LD,LL,L1,NA,NM,ISH,ITN,ITS,KM1,LM1,
X          ENM2,IERR,LOR1,ENORN
      DOUBLE PRECISION A(NM,N),B(NM,N),Z(NM,N)
      DOUBLE PRECISION R,S,T,A1,A2,A3,EP,SH,U1,U2,U3,V1,V2,V3,ANI,A11,
X          A12,A21,A22,A33,A34,A43,A44,BNI,B11,B12,B22,B33,B34,
X          B44,EPSA,EPSB,EPS1,ANORM,BNORM,EPSLON
      LOGICAL MATZ,NOTLAS
C
      THIS SUBROUTINE IS THE SECOND STEP OF THE QZ ALGORITHM
      FOR SOLVING GENERALIZED MATRIX EIGENVALUE PROBLEMS,
      SIAM J. NUMER. ANAL. 10, 241-256(1973) BY MOLER AND STEWART,
      AS MODIFIED IN TECHNICAL NOTE NASA TN D-7305(1973) BY WARD.
C
      THIS SUBROUTINE ACCEPTS A PAIR OF REAL MATRICES, ONE OF THEM
      IN UPPER HESSENBERG FORM AND THE OTHER IN UPPER TRIANGULAR FORM.
      IT REDUCES THE HESSENBERG MATRIX TO QUASI-TRIANGULAR FORM USING
      ORTHOGONAL TRANSFORMATIONS WHILE MAINTAINING THE TRIANGULAR FORM
      OF THE OTHER MATRIX. IT IS USUALLY PRECEDED BY QZHES AND
      FOLLOWED BY QZVAL AND, POSSIBLY, QZVEC.
C
      ON INPUT
C
      NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
      ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
      DIMENSION STATEMENT.
C
      N IS THE ORDER OF THE MATRICES.
C
      A CONTAINS A REAL UPPER HESSENBERG MATRIX.
C
      B CONTAINS A REAL UPPER TRIANGULAR MATRIX.
C
      EPS1 IS A TOLERANCE USED TO DETERMINE NEGLIGIBLE ELEMENTS.
      EPS1 = 0.0 (OR NEGATIVE) MAY BE INPUT, IN WHICH CASE AN
      ELEMENT WILL BE NEGLECTED ONLY IF IT IS LESS THAN ROUNDOFF
      ERROR TIMES THE NORM OF ITS MATRIX. IF THE INPUT EPS1 IS
      POSITIVE, THEN AN ELEMENT WILL BE CONSIDERED NEGLIGIBLE
      IF IT IS LESS THAN EPS1 TIMES THE NORM OF ITS MATRIX. A
      POSITIVE VALUE OF EPS1 MAY RESULT IN FASTER EXECUTION,
      BUT LESS ACCURATE RESULTS.
C
      MATZ SHOULD BE SET TO .TRUE. IF THE RIGHT HAND TRANSFORMATIONS
      ARE TO BE ACCUMULATED FOR LATER USE IN COMPUTING
      EIGENVECTORS, AND TO .FALSE. OTHERWISE.
C

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```

C      Z CONTAINS, IF MATZ HAS BEEN SET TO .TRUE., THE
C      TRANSFORMATION MATRIX PRODUCED IN THE REDUCTION
C      BY QZHES, IF PERFORMED, OR ELSE THE IDENTITY MATRIX.
C      IF MATZ HAS BEEN SET TO .FALSE., Z IS NOT REFERENCED.
C
C      ON OUTPUT
C
C      A HAS BEEN REDUCED TO QUASI-TRIANGULAR FORM.  THE ELEMENTS
C      BELOW THE FIRST SUBDIAGONAL ARE STILL ZERO AND NO TWO
C      CONSECUTIVE SUBDIAGONAL ELEMENTS ARE NONZERO.
C
C      B IS STILL IN UPPER TRIANGULAR FORM, ALTHOUGH ITS ELEMENTS
C      HAVE BEEN ALTERED.  THE LOCATION B(N,1) IS USED TO STORE
C      EPS1 TIMES THE NORM OF B FOR LATER USE BY QZVAL AND
C      QZVEC.
C
C      Z CONTAINS THE PRODUCT OF THE RIGHT HAND TRANSFORMATIONS
C      (FOR BOTH STEPS) IF MATZ HAS BEEN SET TO .TRUE..
C
C      IERR IS SET TO
C      ZERO      FOR NORMAL RETURN,
C      J        IF THE LIMIT OF 30*N ITERATIONS IS EXHAUSTED
C              WHILE THE J-TH EIGENVALUE IS BEING SOUGHT.
C
C      QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C      MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C      THIS VERSION DATED AUGUST 1983.
C
C      -----
C
C
C      IERR = 0
C      ..... COMPUTE EPSA,EPSB .....
C      ANORM = 0.0D0
C      BNORM = 0.0D0
C
C      DO 30 I = 1, N
C          ANI = 0.0D0
C          IF (I .NE. 1) ANI = DABS(A(I,I-1))
C          BNI = 0.0D0
C
C          DO 20 J = I, N
C              ANI = ANI + DABS(A(I,J))
C              BNI = BNI + DABS(B(I,J))
C      20  CONTINUE
C
C          IF (ANI .GT. ANORM) ANORM = ANI
C          IF (BNI .GT. BNORM) BNORM = BNI
C      30  CONTINUE
C
C      IF (ANORM .EQ. 0.0D0) ANORM = 1.0D0
C      IF (BNORM .EQ. 0.0D0) BNORM = 1.0D0
C      EP = EPS1
C      IF (EP .GT. 0.0D0) GO TO 50
C      ..... USE ROUND OFF LEVEL IF EPS1 IS ZERO .....
C      EP = EPSLON(1.0D0)

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50 EPSA = EP * ANORM
   EPSB = EP * BNORM
C     ..... REDUCE A TO QUASI-TRIANGULAR FORM, WHILE
C     ..... KEEPING B TRIANGULAR .....
   LOR1 = 1
   ENORN = N
   EN = N
   ITN = 30*N
C     ..... BEGIN QZ STEP .....
60 IF (EN .LE. 2) GO TO 1001
   IF (.NOT. MATZ) ENORN = EN
   ITS = 0
   NA = EN - 1
   ENM2 = NA - 1
70 ISH = 2
C     ..... CHECK FOR CONVERGENCE OR REDUCIBILITY.
C     ..... FOR L=EN STEP -1 UNTIL 1 DO -- .....
   DO 80 LL = 1, EN
     LM1 = EN - LL
     L = LM1 + 1
     IF (L .EQ. 1) GO TO 95
     IF (DABS(A(L,LM1)) .LE. EPSA) GO TO 90
80 CONTINUE
C
90 A(L,LM1) = 0.0D0
   IF (L .LT. NA) GO TO 95
C     ..... 1-BY-1 OR 2-BY-2 BLOCK ISOLATED .....
   EN = LM1
   GO TO 60
C     ..... CHECK FOR SMALL TOP OF B .....
95 LD = L
100 L1 = L + 1
   B11 = B(L,L)
   IF (DABS(B11) .GT. EPSB) GO TO 120
   B(L,L) = 0.0D0
   S = DABS(A(L,L)) + DABS(A(L1,L))
   U1 = A(L,L) / S
   U2 = A(L1,L) / S
   R = DSIGN(DSQRT(U1*U1+U2*U2),U1)
   V1 = -(U1 + R) / R
   V2 = -U2 / R
   U2 = V2 / V1
C
   DO 110 J = L, ENORN
     T = A(L,J) + U2 * A(L1,J)
     A(L,J) = A(L,J) + T * V1
     A(L1,J) = A(L1,J) + T * V2
     T = B(L,J) + U2 * B(L1,J)
     B(L,J) = B(L,J) + T * V1
     B(L1,J) = B(L1,J) + T * V2
110 CONTINUE
C
   IF (L .NE. 1) A(L,LM1) = -A(L,LM1)
   LM1 = L
   L = L1
   GO TO 90
120 A11 = A(L,L) / B11

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A21 = A(L1,L) / B11
IF (ISH .EQ. 1) GO TO 140
C ..... ITERATION STRATEGY .....
IF (ITN .EQ. 0) GO TO 1000
IF (ITS .EQ. 10) GO TO 155
C ..... DETERMINE TYPE OF SHIFT .....
B22 = B(L1,L1)
IF (DABS(B22) .LT. EPSB) B22 = EPSB
B33 = B(NA,NA)
IF (DABS(B33) .LT. EPSB) B33 = EPSB
B44 = B(EN,EN)
IF (DABS(B44) .LT. EPSB) B44 = EPSB
A33 = A(NA,NA) / B33
A34 = A(NA,EN) / B44
A43 = A(EN,NA) / B33
A44 = A(EN,EN) / B44
B34 = B(NA,EN) / B44
T = 0.5D0 * (A43 * B34 - A33 - A44)
R = T * T + A34 * A43 - A33 * A44
IF (R .LT. 0.0D0) GO TO 150
C ..... DETERMINE SINGLE SHIFT ZEROETH COLUMN OF A .....
ISH = 1
R = DSQRT(R)
SH = -T + R
S = -T - R
IF (DABS(S-A44) .LT. DABS(SH-A44)) SH = S
C ..... LOOK FOR TWO CONSECUTIVE SMALL
C SUB-DIAGONAL ELEMENTS OF A.
C FOR L=EN-2 STEP -1 UNTIL LD DO -- .....
DO 130 LL = LD, ENM2
L = ENM2 + LD - LL
IF (L .EQ. LD) GO TO 140
LM1 = L - 1
L1 = L + 1
T = A(L,L)
IF (DABS(B(L,L)) .GT. EPSB) T = T - SH * B(L,L)
IF (DABS(A(L,LM1)) .LE. DABS(T/A(L1,L)) * EPSA) GO TO 100
130 CONTINUE
C
140 A1 = A11 - SH
A2 = A21
IF (L .NE. LD) A(L,LM1) = -A(L,LM1)
GO TO 160
C ..... DETERMINE DOUBLE SHIFT ZEROETH COLUMN OF A .....
150 A12 = A(L,L1) / B22
A22 = A(L1,L1) / B22
B12 = B(L,L1) / B22
A1 = ((A33 - A11) * (A44 - A11) - A34 * A43 + A43 * B34 * A11)
X / A21 + A12 - A11 * B12
A2 = (A22 - A11) - A21 * B12 - (A33 - A11) - (A44 - A11)
X + A43 * B34
A3 = A(L1+1,L1) / B22
GO TO 160
C ..... AD HOC SHIFT .....
155 A1 = 0.0D0
A2 = 1.0D0
A3 = 1.1605D0

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160 ITS = ITS + 1
    ITN = ITN - 1
    IF (.NOT. MATZ) LOR1 = LD
C     ..... MAIN LOOP .....
    DO 260 K = L, NA
        NOTLAS = K .NE. NA .AND. ISH .EQ. 2
        K1 = K + 1
        K2 = K + 2
        KM1 = MAX0(K-1,L)
        LL = MIN0(EN,K1+ISH)
        IF (NOTLAS) GO TO 190
C     ..... ZERO A(K+1,K-1) .....
        IF (K .EQ. L) GO TO 170
        A1 = A(K,KM1)
        A2 = A(K1,KM1)
170    S = DABS(A1) + DABS(A2)
        IF (S .EQ. 0.0D0) GO TO 70
        U1 = A1 / S
        U2 = A2 / S
        R = DSIGN(DSQRT(U1*U1+U2*U2),U1)
        V1 = -(U1 + R) / R
        V2 = -U2 / R
        U2 = V2 / V1
C
        DO 180 J = KM1, ENORN
            T = A(K,J) + U2 * A(K1,J)
            A(K,J) = A(K,J) + T * V1
            A(K1,J) = A(K1,J) + T * V2
            T = B(K,J) + U2 * B(K1,J)
            B(K,J) = B(K,J) + T * V1
            B(K1,J) = B(K1,J) + T * V2
180    CONTINUE
C
        IF (K .NE. L) A(K1,KM1) = 0.0D0
        GO TO 240
C     ..... ZERO A(K+1,K-1) AND A(K+2,K-1) .....
190    IF (K .EQ. L) GO TO 200
        A1 = A(K,KM1)
        A2 = A(K1,KM1)
        A3 = A(K2,KM1)
200    S = DABS(A1) + DABS(A2) + DABS(A3)
        IF (S .EQ. 0.0D0) GO TO 260
        U1 = A1 / S
        U2 = A2 / S
        U3 = A3 / S
        R = DSIGN(DSQRT(U1*U1+U2*U2+U3*U3),U1)
        V1 = -(U1 + R) / R
        V2 = -U2 / R
        V3 = -U3 / R
        U2 = V2 / V1
        U3 = V3 / V1
C
        DO 210 J = KM1, ENORN
            T = A(K,J) + U2 * A(K1,J) + U3 * A(K2,J)
            A(K,J) = A(K,J) + T * V1
            A(K1,J) = A(K1,J) + T * V2
            A(K2,J) = A(K2,J) + T * V3

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```

        T = B(K,J) + U2 * B(K1,J) + U3 * B(K2,J)
        B(K,J) = B(K,J) + T * V1
        B(K1,J) = B(K1,J) + T * V2
        B(K2,J) = B(K2,J) + T * V3
210    CONTINUE
C
        IF (K .EQ. L) GO TO 220
        A(K1,KM1) = 0.0D0
        A(K2,KM1) = 0.0D0
C
        ..... ZERO B(K+2,K+1) AND B(K+2,K) .....
220    S = DABS(B(K2,K2)) + DABS(B(K2,K1)) + DABS(B(K2,K))
        IF (S .EQ. 0.0D0) GO TO 240
        U1 = B(K2,K2) / S
        U2 = B(K2,K1) / S
        U3 = B(K2,K) / S
        R = DSIGN(DSQRT(U1*U1+U2*U2+U3*U3),U1)
        V1 = -(U1 + R) / R
        V2 = -U2 / R
        V3 = -U3 / R
        U2 = V2 / V1
        U3 = V3 / V1
C
        DO 230 I = LOR1, LL
            T = A(I,K2) + U2 * A(I,K1) + U3 * A(I,K)
            A(I,K2) = A(I,K2) + T * V1
            A(I,K1) = A(I,K1) + T * V2
            A(I,K) = A(I,K) + T * V3
            T = B(I,K2) + U2 * B(I,K1) + U3 * B(I,K)
            B(I,K2) = B(I,K2) + T * V1
            B(I,K1) = B(I,K1) + T * V2
            B(I,K) = B(I,K) + T * V3
230    CONTINUE
C
        B(K2,K) = 0.0D0
        B(K2,K1) = 0.0D0
        IF (.NOT. MATZ) GO TO 240
C
        DO 235 I = 1, N
            T = Z(I,K2) + U2 * Z(I,K1) + U3 * Z(I,K)
            Z(I,K2) = Z(I,K2) + T * V1
            Z(I,K1) = Z(I,K1) + T * V2
            Z(I,K) = Z(I,K) + T * V3
235    CONTINUE
C
        ..... ZERO B(K+1,K) .....
240    S = DABS(B(K1,K1)) + DABS(B(K1,K))
        IF (S .EQ. 0.0D0) GO TO 260
        U1 = B(K1,K1) / S
        U2 = B(K1,K) / S
        R = DSIGN(DSQRT(U1*U1+U2*U2),U1)
        V1 = -(U1 + R) / R
        V2 = -U2 / R
        U2 = V2 / V1
C
        DO 250 I = LOR1, LL
            T = A(I,K1) + U2 * A(I,K)
            A(I,K1) = A(I,K1) + T * V1
            A(I,K) = A(I,K) + T * V2

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                T = B(I,K1) + U2 * B(I,K)
                B(I,K1) = B(I,K1) + T * V1
                B(I,K) = B(I,K) + T * V2
250    CONTINUE
C
                B(K1,K) = 0.0D0
                IF (.NOT. MATZ) GO TO 260
C
                DO 255 I = 1, N
                    T = Z(I,K1) + U2 * Z(I,K)
                    Z(I,K1) = Z(I,K1) + T * V1
                    Z(I,K) = Z(I,K) + T * V2
255    CONTINUE
C
260    CONTINUE
C
                ..... END QZ STEP .....
                GO TO 70
C
                ..... SET ERROR -- ALL EIGENVALUES HAVE NOT
C
                ..... CONVERGED AFTER 30*N ITERATIONS .....
1000   IERR = EN
C
                ..... SAVE EPSB FOR USE BY QZVAL AND QZVEC .....
1001   IF (N .GT. 1) B(N,1) = EPSB
                RETURN
                END
                SUBROUTINE QZVAL(NM,N,A,B,ALFR,ALFI,BETA,MATZ,Z)
C
                INTEGER I,J,N,EN,NA,NM,NN,ISW
                DOUBLE PRECISION A(NM,N),B(NM,N),ALFR(N),ALFI(N),BETA(N),Z(NM,N)
                DOUBLE PRECISION C,D,E,R,S,T,AN,A1,A2,BN,CQ,CZ,DI,DR,EI,TI,TR,U1,
X      U2,V1,V2,A1I,A11,A12,A2I,A21,A22,B11,B12,B22,SQI,SQR,
X      SSI,SSR,SZI,SZR,A11I,A11R,A12I,A12R,A22I,A22R,EPSB
                LOGICAL MATZ
C
                THIS SUBROUTINE IS THE THIRD STEP OF THE QZ ALGORITHM
C
                FOR SOLVING GENERALIZED MATRIX EIGENVALUE PROBLEMS,
C
                SIAM J. NUMER. ANAL. 10, 241-256(1973) BY MOLER AND STEWART.
C
                THIS SUBROUTINE ACCEPTS A PAIR OF REAL MATRICES, ONE OF THEM
C
                IN QUASI-TRIANGULAR FORM AND THE OTHER IN UPPER TRIANGULAR FORM.
C
                IT REDUCES THE QUASI-TRIANGULAR MATRIX FURTHER, SO THAT ANY
C
                REMAINING 2-BY-2 BLOCKS CORRESPOND TO PAIRS OF COMPLEX
C
                EIGENVALUES, AND RETURNS QUANTITIES WHOSE RATIOS GIVE THE
C
                GENERALIZED EIGENVALUES. IT IS USUALLY PRECEDED BY QZHES
C
                AND QZIT AND MAY BE FOLLOWED BY QZVEC.
C
                ON INPUT
C
                NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C
                ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C
                DIMENSION STATEMENT.
C
                N IS THE ORDER OF THE MATRICES.
C
                A CONTAINS A REAL UPPER QUASI-TRIANGULAR MATRIX.
C
                B CONTAINS A REAL UPPER TRIANGULAR MATRIX. IN ADDITION,
C
                LOCATION B(N,1) CONTAINS THE TOLERANCE QUANTITY (EPSB)

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C          COMPUTED AND SAVED IN QZIT.
C
C          MATZ SHOULD BE SET TO .TRUE. IF THE RIGHT HAND TRANSFORMATIONS
C          ARE TO BE ACCUMULATED FOR LATER USE IN COMPUTING
C          EIGENVECTORS, AND TO .FALSE. OTHERWISE.
C
C          Z CONTAINS, IF MATZ HAS BEEN SET TO .TRUE., THE
C          TRANSFORMATION MATRIX PRODUCED IN THE REDUCTIONS BY QZHES
C          AND QZIT, IF PERFORMED, OR ELSE THE IDENTITY MATRIX.
C          IF MATZ HAS BEEN SET TO .FALSE., Z IS NOT REFERENCED.
C
C          ON OUTPUT
C
C          A HAS BEEN REDUCED FURTHER TO A QUASI-TRIANGULAR MATRIX
C          IN WHICH ALL NONZERO SUBDIAGONAL ELEMENTS CORRESPOND TO
C          PAIRS OF COMPLEX EIGENVALUES.
C
C          B IS STILL IN UPPER TRIANGULAR FORM, ALTHOUGH ITS ELEMENTS
C          HAVE BEEN ALTERED. B(N,1) IS UNALTERED.
C
C          ALFR AND ALFI CONTAIN THE REAL AND IMAGINARY PARTS OF THE
C          DIAGONAL ELEMENTS OF THE TRIANGULAR MATRIX THAT WOULD BE
C          OBTAINED IF A WERE REDUCED COMPLETELY TO TRIANGULAR FORM
C          BY UNITARY TRANSFORMATIONS. NON-ZERO VALUES OF ALFI OCCUR
C          IN PAIRS, THE FIRST MEMBER POSITIVE AND THE SECOND NEGATIVE.
C
C          BETA CONTAINS THE DIAGONAL ELEMENTS OF THE CORRESPONDING B,
C          NORMALIZED TO BE REAL AND NON-NEGATIVE. THE GENERALIZED
C          EIGENVALUES ARE THEN THE RATIOS ((ALFR+I*ALFI)/BETA).
C
C          Z CONTAINS THE PRODUCT OF THE RIGHT HAND TRANSFORMATIONS
C          (FOR ALL THREE STEPS) IF MATZ HAS BEEN SET TO .TRUE.
C
C          QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C          MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C          THIS VERSION DATED AUGUST 1983.
C
C          -----
C
C          EPSB = B(N,1)
C          ISW = 1
C          ..... FIND EIGENVALUES OF QUASI-TRIANGULAR MATRICES.
C          FOR EN=N STEP -1 UNTIL 1 DO -- .....
C          DO 510 NN = 1, N
C             EN = N + 1 - NN
C             NA = EN - 1
C             IF (ISW .EQ. 2) GO TO 505
C             IF (EN .EQ. 1) GO TO 410
C             IF (A(EN,NA) .NE. 0.0D0) GO TO 420
C          ..... 1-BY-1 BLOCK, ONE REAL ROOT .....
410      ALFR(EN) = A(EN,EN)
          IF (B(EN,EN) .LT. 0.0D0) ALFR(EN) = -ALFR(EN)
          BETA(EN) = DABS(B(EN,EN))
          ALFI(EN) = 0.0D0
          GO TO 510

```

```

C ..... 2-BY-2 BLOCK .....
420 IF (DABS(B(NA,NA)) .LE. EPSB) GO TO 455
    IF (DABS(B(EN,EN)) .GT. EPSB) GO TO 430
    A1 = A(EN,EN)
    A2 = A(EN,NA)
    BN = 0.0D0
    GO TO 435
430 AN = DABS(A(NA,NA)) + DABS(A(NA,EN)) + DABS(A(EN,NA))
    X   + DABS(A(EN,EN))
    BN = DABS(B(NA,NA)) + DABS(B(NA,EN)) + DABS(B(EN,EN))
    A11 = A(NA,NA) / AN
    A12 = A(NA,EN) / AN
    A21 = A(EN,NA) / AN
    A22 = A(EN,EN) / AN
    B11 = B(NA,NA) / BN
    B12 = B(NA,EN) / BN
    B22 = B(EN,EN) / BN
    E = A11 / B11
    EI = A22 / B22
    S = A21 / (B11 * B22)
    T = (A22 - E * B22) / B22
    IF (DABS(E) .LE. DABS(EI)) GO TO 431
    E = EI
    T = (A11 - E * B11) / B11
431 C = 0.5D0 * (T - S * B12)
    D = C * C + S * (A12 - E * B12)
    IF (D .LT. 0.0D0) GO TO 480
C ..... TWO REAL ROOTS.
C           ZERO BOTH A(EN,NA) AND B(EN,NA) .....
    E = E + (C + DSIGN(DSQRT(D),C))
    A11 = A11 - E * B11
    A12 = A12 - E * B12
    A22 = A22 - E * B22
    IF (DABS(A11) + DABS(A12) .LT.
    X   DABS(A21) + DABS(A22)) GO TO 432
    A1 = A12
    A2 = A11
    GO TO 435
432 A1 = A22
    A2 = A21
C ..... CHOOSE AND APPLY REAL Z .....
435 S = DABS(A1) + DABS(A2)
    U1 = A1 / S
    U2 = A2 / S
    R = DSIGN(DSQRT(U1*U1+U2*U2),U1)
    V1 = -(U1 + R) / R
    V2 = -U2 / R
    U2 = V2 / V1
C
    DO 440 I = 1, EN
        T = A(I,EN) + U2 * A(I,NA)
        A(I,EN) = A(I,EN) + T * V1
        A(I,NA) = A(I,NA) + T * V2
        T = B(I,EN) + U2 * B(I,NA)
        B(I,EN) = B(I,EN) + T * V1
        B(I,NA) = B(I,NA) + T * V2
440 CONTINUE

```

```

C      IF (.NOT. MATZ) GO TO 450
C
      DO 445 I = 1, N
          T = Z(I,EN) + U2 * Z(I,NA)
          Z(I,EN) = Z(I,EN) + T * V1
          Z(I,NA) = Z(I,NA) + T * V2
445    CONTINUE
C
450    IF (BN .EQ. 0.0D0) GO TO 475
      IF (AN .LT. DABS(E) * BN) GO TO 455
      A1 = B(NA,NA)
      A2 = B(EN,NA)
      GO TO 460
455    A1 = A(NA,NA)
      A2 = A(EN,NA)
C      ..... CHOOSE AND APPLY REAL Q .....
460    S = DABS(A1) + DABS(A2)
      IF (S .EQ. 0.0D0) GO TO 475
      U1 = A1 / S
      U2 = A2 / S
      R = DSIGN(DSQRT(U1*U1+U2*U2), U1)
      V1 = -(U1 + R) / R
      V2 = -U2 / R
      U2 = V2 / V1
C
      DO 470 J = NA, N
          T = A(NA,J) + U2 * A(EN,J)
          A(NA,J) = A(NA,J) + T * V1
          A(EN,J) = A(EN,J) + T * V2
          T = B(NA,J) + U2 * B(EN,J)
          B(NA,J) = B(NA,J) + T * V1
          B(EN,J) = B(EN,J) + T * V2
470    CONTINUE
C
475    A(EN,NA) = 0.0D0
      B(EN,NA) = 0.0D0
      ALFR(NA) = A(NA,NA)
      ALFR(EN) = A(EN,EN)
      IF (B(NA,NA) .LT. 0.0D0) ALFR(NA) = -ALFR(NA)
      IF (B(EN,EN) .LT. 0.0D0) ALFR(EN) = -ALFR(EN)
      BETA(NA) = DABS(B(NA,NA))
      BETA(EN) = DABS(B(EN,EN))
      ALFI(EN) = 0.0D0
      ALFI(NA) = 0.0D0
      GO TO 505
C      ..... TWO COMPLEX ROOTS .....
480    E = E + C
      EI = DSQRT(-D)
      A11R = A11 - E * B11
      A11I = EI * B11
      A12R = A12 - E * B12
      A12I = EI * B12
      A22R = A22 - E * B22
      A22I = EI * B22
      IF (DABS(A11R) + DABS(A11I) + DABS(A12R) + DABS(A12I) .LT.
X      DABS(A21) + DABS(A22R) + DABS(A22I)) GO TO 482

```

```

A1 = A12R
A1I = A12I
A2 = -A11R
A2I = -A11I
GO TO 485
482  A1 = A22R
     A1I = A22I
     A2 = -A21
     A2I = 0.0D0
C    ..... CHOOSE COMPLEX Z .....
485  CZ = DSQRT(A1*A1+A1I*A1I)
     IF (CZ .EQ. 0.0D0) GO TO 487
     SZR = (A1 * A2 + A1I * A2I) / CZ
     SZI = (A1 * A2I - A1I * A2) / CZ
     R = DSQRT(CZ*CZ+SZR*SZR+SZI*SZI)
     CZ = CZ / R
     SZR = SZR / R
     SZI = SZI / R
     GO TO 490
487  SZR = 1.0D0
     SZI = 0.0D0
490  IF (AN .LT. (DABS(E) + EI) * BN) GO TO 492
     A1 = CZ * B11 + SZR * B12
     A1I = SZI * B12
     A2 = SZR * B22
     A2I = SZI * B22
     GO TO 495
492  A1 = CZ * A11 + SZR * A12
     A1I = SZI * A12
     A2 = CZ * A21 + SZR * A22
     A2I = SZI * A22
C    ..... CHOOSE COMPLEX Q .....
495  CQ = DSQRT(A1*A1+A1I*A1I)
     IF (CQ .EQ. 0.0D0) GO TO 497
     SQR = (A1 * A2 + A1I * A2I) / CQ
     SQI = (A1 * A2I - A1I * A2) / CQ
     R = DSQRT(CQ*CQ+SQR*SQR+SQI*SQI)
     CQ = CQ / R
     SQR = SQR / R
     SQI = SQI / R
     GO TO 500
497  SQR = 1.0D0
     SQI = 0.0D0
C    ..... COMPUTE DIAGONAL ELEMENTS THAT WOULD RESULT
C    IF TRANSFORMATIONS WERE APPLIED .....
500  SSR = SQR * SZR + SQI * SZI
     SSI = SQR * SZI - SQI * SZR
     I = 1
     TR = CQ * CZ * A11 + CQ * SZR * A12 + SQR * CZ * A21
X     + SSR * A22
     TI = CQ * SZI * A12 - SQI * CZ * A21 + SSI * A22
     DR = CQ * CZ * B11 + CQ * SZR * B12 + SSR * B22
     DI = CQ * SZI * B12 + SSI * B22
     GO TO 503
502  I = 2
     TR = SSR * A11 - SQR * CZ * A12 - CQ * SZR * A21
X     + CQ * CZ * A22

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      TI = -SSI * A11 - SQI * CZ * A12 + CQ * SZI * A21
      DR = SSR * B11 - SQR * CZ * B12 + CQ * CZ * B22
      DI = -SSI * B11 - SQI * CZ * B12
503  T = TI * DR - TR * DI
      J = NA
      IF (T .LT. 0.0D0) J = EN
      R = DSQRT(DR*DR+DI*DI)
      BETA(J) = BN * R
      ALFR(J) = AN * (TR * DR + TI * DI) / R
      ALFI(J) = AN * T / R
      IF (I .EQ. 1) GO TO 502
505  ISW = 3 - ISW
510  CONTINUE
      B(N,1) = EPSB
C
      RETURN
      END

      SUBROUTINE QZVEC(NM,N,A,B,ALFR,ALFI,BETA,Z)
C
      INTEGER I,J,K,M,N,EN,II,JJ,NA,NM,NN,ISW,ENM2
      DOUBLE PRECISION A(NM,N),B(NM,N),ALFR(N),ALFI(N),BETA(N),Z(NM,N)
      DOUBLE PRECISION
D,Q,R,S,T,W,X,Y,DI,DR,RA,RR,SA,TI,TR,T1,T2,W1,X1,
X      ZZ,Z1,ALFM,ALMI,ALMR,BETM,EPSB
C
C      THIS SUBROUTINE IS THE OPTIONAL FOURTH STEP OF THE QZ ALGORITHM
C      FOR SOLVING GENERALIZED MATRIX EIGENVALUE PROBLEMS,
C      SIAM J. NUMER. ANAL. 10, 241-256(1973) BY MOLER AND STEWART.
C
C      THIS SUBROUTINE ACCEPTS A PAIR OF REAL MATRICES, ONE OF THEM IN
C      QUASI-TRIANGULAR FORM (IN WHICH EACH 2-BY-2 BLOCK CORRESPONDS TO
C      A PAIR OF COMPLEX EIGENVALUES) AND THE OTHER IN UPPER TRIANGULAR
C      FORM. IT COMPUTES THE EIGENVECTORS OF THE TRIANGULAR PROBLEM AND
C      TRANSFORMS THE RESULTS BACK TO THE ORIGINAL COORDINATE SYSTEM.
C      IT IS USUALLY PRECEDED BY QZHES, QZIT, AND QZVAL.
C
C      ON INPUT
C
C      NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C      ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C      DIMENSION STATEMENT.
C
C      N IS THE ORDER OF THE MATRICES.
C
C      A CONTAINS A REAL UPPER QUASI-TRIANGULAR MATRIX.
C
C      B CONTAINS A REAL UPPER TRIANGULAR MATRIX. IN ADDITION,
C      LOCATION B(N,1) CONTAINS THE TOLERANCE QUANTITY (EPSB)
C      COMPUTED AND SAVED IN QZIT.
C
C      ALFR, ALFI, AND BETA ARE VECTORS WITH COMPONENTS WHOSE
C      RATIOS ((ALFR+I*ALFI)/BETA) ARE THE GENERALIZED
C      EIGENVALUES. THEY ARE USUALLY OBTAINED FROM QZVAL.
C
C      Z CONTAINS THE TRANSFORMATION MATRIX PRODUCED IN THE
C      REDUCTIONS BY QZHES, QZIT, AND QZVAL, IF PERFORMED.

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C           IF THE EIGENVECTORS OF THE TRIANGULAR PROBLEM ARE
C           DESIRED, Z MUST CONTAIN THE IDENTITY MATRIX.
C
C ON OUTPUT
C
C           A IS UNALTERED.  ITS SUBDIAGONAL ELEMENTS PROVIDE INFORMATION
C           ABOUT THE STORAGE OF THE COMPLEX EIGENVECTORS.
C
C           B HAS BEEN DESTROYED.
C
C           ALFR, ALFI, AND BETA ARE UNALTERED.
C
C           Z CONTAINS THE REAL AND IMAGINARY PARTS OF THE EIGENVECTORS.
C           IF ALFI(I) .EQ. 0.0, THE I-TH EIGENVALUE IS REAL AND
C           THE I-TH COLUMN OF Z CONTAINS ITS EIGENVECTOR.
C           IF ALFI(I) .NE. 0.0, THE I-TH EIGENVALUE IS COMPLEX.
C           IF ALFI(I) .GT. 0.0, THE EIGENVALUE IS THE FIRST OF
C           A COMPLEX PAIR AND THE I-TH AND (I+1)-TH COLUMNS
C           OF Z CONTAIN ITS EIGENVECTOR.
C           IF ALFI(I) .LT. 0.0, THE EIGENVALUE IS THE SECOND OF
C           A COMPLEX PAIR AND THE (I-1)-TH AND I-TH COLUMNS
C           OF Z CONTAIN THE CONJUGATE OF ITS EIGENVECTOR.
C           EACH EIGENVECTOR IS NORMALIZED SO THAT THE MODULUS
C           OF ITS LARGEST COMPONENT IS 1.0 .
C
C QUESTIONS AND COMMENTS SHOULD BE DIRECTED TO BURTON S. GARBOW,
C MATHEMATICS AND COMPUTER SCIENCE DIV, ARGONNE NATIONAL LABORATORY
C
C THIS VERSION DATED AUGUST 1983.
C
C -----
C
C EPSB = B(N,1)
C ISW = 1
C ..... FOR EN=N STEP -1 UNTIL 1 DO -- .....
C DO 800 NN = 1, N
C     EN = N + 1 - NN
C     NA = EN - 1
C     IF (ISW .EQ. 2) GO TO 795
C     IF (ALFI(EN) .NE. 0.0D0) GO TO 710
C ..... REAL VECTOR .....
C     M = EN
C     B(EN,EN) = 1.0D0
C     IF (NA .EQ. 0) GO TO 800
C     ALFM = ALFR(M)
C     BETM = BETA(M)
C ..... FOR I=EN-1 STEP -1 UNTIL 1 DO -- .....
C DO 700 II = 1, NA
C     I = EN - II
C     W = BETM * A(I,I) - ALFM * B(I,I)
C     R = 0.0D0
C
C DO 610 J = M, EN
610 R = R + (BETM * A(I,J) - ALFM * B(I,J)) * B(J,EN)
C
C     IF (I .EQ. 1 .OR. ISW .EQ. 2) GO TO 630

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        IF (BETM * A(I,I-1) .EQ. 0.0D0) GO TO 630
        ZZ = W
        S = R
        GO TO 690
630      M = I
        IF (ISW .EQ. 2) GO TO 640
C       ..... REAL 1-BY-1 BLOCK .....
        T = W
        IF (W .EQ. 0.0D0) T = EPSB
        B(I,EN) = -R / T
        GO TO 700
C       ..... REAL 2-BY-2 BLOCK .....
640      X = BETM * A(I,I+1) - ALFM * B(I,I+1)
        Y = BETM * A(I+1,I)
        Q = W * ZZ - X * Y
        T = (X * S - ZZ * R) / Q
        B(I,EN) = T
        IF (DABS(X) .LE. DABS(ZZ)) GO TO 650
        B(I+1,EN) = (-R - W * T) / X
        GO TO 690
650      B(I+1,EN) = (-S - Y * T) / ZZ
690      ISW = 3 - ISW
700      CONTINUE
C       ..... END REAL VECTOR .....
        GO TO 800
C       ..... COMPLEX VECTOR .....
710      M = NA
        ALMR = ALFR(M)
        ALMI = ALFI(M)
        BETM = BETA(M)
C       ..... LAST VECTOR COMPONENT CHOSEN IMAGINARY SO THAT
C       EIGENVECTOR MATRIX IS TRIANGULAR .....
        Y = BETM * A(EN,NA)
        B(NA,NA) = -ALMI * B(EN,EN) / Y
        B(NA,EN) = (ALMR * B(EN,EN) - BETM * A(EN,EN)) / Y
        B(EN,NA) = 0.0D0
        B(EN,EN) = 1.0D0
        ENM2 = NA - 1
        IF (ENM2 .EQ. 0) GO TO 795
C       ..... FOR I=EN-2 STEP -1 UNTIL 1 DO -- .....
        DO 790 II = 1, ENM2
            I = NA - II
            W = BETM * A(I,I) - ALMR * B(I,I)
            W1 = -ALMI * B(I,I)
            RA = 0.0D0
            SA = 0.0D0
C
            DO 760 J = M, EN
                X = BETM * A(I,J) - ALMR * B(I,J)
                X1 = -ALMI * B(I,J)
                RA = RA + X * B(J,NA) - X1 * B(J,EN)
                SA = SA + X * B(J,EN) + X1 * B(J,NA)
760      CONTINUE
C
        IF (I .EQ. 1 .OR. ISW .EQ. 2) GO TO 770
        IF (BETM * A(I,I-1) .EQ. 0.0D0) GO TO 770
        ZZ = W

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```

        Z1 = W1
        R = RA
        S = SA
        ISW = 2
        GO TO 790
770      M = I
        IF (ISW .EQ. 2) GO TO 780
C       ..... COMPLEX 1-BY-1 BLOCK .....
        TR = -RA
        TI = -SA
773      DR = W
        DI = W1
C       ..... COMPLEX DIVIDE (T1,T2) = (TR,TI) / (DR,DI) .....
775      IF (DABS(DI) .GT. DABS(DR)) GO TO 777
        RR = DI / DR
        D = DR + DI * RR
        T1 = (TR + TI * RR) / D
        T2 = (TI - TR * RR) / D
        GO TO (787,782), ISW
777      RR = DR / DI
        D = DR * RR + DI
        T1 = (TR * RR + TI) / D
        T2 = (TI * RR - TR) / D
        GO TO (787,782), ISW
C       ..... COMPLEX 2-BY-2 BLOCK .....
780      X = BETM * A(I,I+1) - ALMR * B(I,I+1)
        X1 = -ALMI * B(I,I+1)
        Y = BETM * A(I+1,I)
        TR = Y * RA - W * R + W1 * S
        TI = Y * SA - W * S - W1 * R
        DR = W * ZZ - W1 * Z1 - X * Y
        DI = W * Z1 + W1 * ZZ - X1 * Y
        IF (DR .EQ. 0.0D0 .AND. DI .EQ. 0.0D0) DR = EPSB
        GO TO 775
782      B(I+1,NA) = T1
        B(I+1,EN) = T2
        ISW = 1
        IF (DABS(Y) .GT. DABS(W) + DABS(W1)) GO TO 785
        TR = -RA - X * B(I+1,NA) + X1 * B(I+1,EN)
        TI = -SA - X * B(I+1,EN) - X1 * B(I+1,NA)
        GO TO 773
785      T1 = (-R - ZZ * B(I+1,NA) + Z1 * B(I+1,EN)) / Y
        T2 = (-S - ZZ * B(I+1,EN) - Z1 * B(I+1,NA)) / Y
787      B(I,NA) = T1
        B(I,EN) = T2
790      CONTINUE
C       ..... END COMPLEX VECTOR .....
795      ISW = 3 - ISW
800      CONTINUE
C       ..... END BACK SUBSTITUTION.
C           TRANSFORM TO ORIGINAL COORDINATE SYSTEM.
C           FOR J=N STEP -1 UNTIL 1 DO -- .....
DO 880 JJ = 1, N
        J = N + 1 - JJ
C
DO 880 I = 1, N
        ZZ = 0.0D0

```

```

C
      DO 860 K = 1, J
860      ZZ = ZZ + Z(I,K) * B(K,J)
C
      Z(I,J) = ZZ
880 CONTINUE
C
      ..... NORMALIZE SO THAT MODULUS OF LARGEST
C
      COMPONENT OF EACH VECTOR IS 1.
C
      (ISW IS 1 INITIALLY FROM BEFORE) .....
      DO 950 J = 1, N
      D = 0.0D0
      IF (ISW .EQ. 2) GO TO 920
      IF (ALFI(J) .NE. 0.0D0) GO TO 945
C
      DO 890 I = 1, N
      IF (DABS(Z(I,J)) .GT. D) D = DABS(Z(I,J))
890 CONTINUE
C
      DO 900 I = 1, N
900      Z(I,J) = Z(I,J) / D
C
      GO TO 950
C
920      DO 930 I = 1, N
      R = DABS(Z(I,J-1)) + DABS(Z(I,J))
      IF (R .NE. 0.0D0) R = R * DSQRT((Z(I,J-1)/R)**2
X
      + (Z(I,J)/R)**2)
      IF (R .GT. D) D = R
930 CONTINUE
C
      DO 940 I = 1, N
      Z(I,J-1) = Z(I,J-1) / D
      Z(I,J) = Z(I,J) / D
940 CONTINUE
C
945      ISW = 3 - ISW
950 CONTINUE
C
      RETURN
      END

```

References

- Li, Jing; Kapania, R.K.** 2000: A geometrically exact curved beam theory and its finite element formulation/implementation. *Thesis for Master of Science in Aerospace Engineering*. Virginia Polytechnic Institute and State University
- Kapania, R.K., Li, J.** 1998: A Four Noded 3-D geometrically nonlinear curved beam element with large displacements/rotations, in *Modeling and Simulation Based Engineering*, S.N. Atluri and P.E. O'Donoghue (ed.), Technology Science Press, 679-784