

A STUDY  
"  
OF  
STRUCTURAL SOUTHERN PINE

By

OSCAR JENNINGS BLAKE  
"

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Approved:

[REDACTED]  
In Charge of Investigation

[REDACTED]  
Head of Department

[REDACTED]  
Dean of Engineering

[REDACTED]  
Chairman of Graduate Committee

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A STUDY  
OF  
STRUCTURAL SOUTHERN PINE

SYNOPSIS

A shipment of 5,591 board feet of dense select structural southern pine, graded and inspected to conform to standard grading rules for southern pine lumber, was carefully investigated in order to determine (1) whether the lumber is within grade limitations, (2) what are the variations in and ranges of its physical and mechanical properties, and (3) whether it fulfills anticipations of design specifications.

Within its limitations, the study indicates that the investigated lumber fulfills the expectations of the National Design Specification despite the fact that some of the planks are found below grade according to the standard grading rules. Hence, those recommendations of the National Design Specification subjected to investigation are substantiated by laboratory test data on a commercially stress-graded shipment of southern pine lumber.

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## I. INTRODUCTION

During the recent war as much wood as could be made available was used for structural purposes. Because of the shortage in lumber, certain deviations from pre-war practices were employed which, in most cases, proved to be satisfactory. Probably the most pronounced of these changes were increases in the allowable working stresses. Many of the expediencies employed were based on assumptions made by designing engineers because of the lack of readily available information on characteristics and properties of the material.

It is the purpose of this investigation to provide designing engineers with definite information concerning what they actually may expect from a shipment of stress-graded lumber on the basis of laboratory studies of such a shipment.\* Although only 1.5 to 2 percent of the nine billion board feet of southern pine lumber manufactured during 1947 were sold as dense select structural,<sup>1</sup> this grade was chosen for the study because the freedom of the grade from defects facilitates performance of directly comparative tests. Furthermore, it allows use of the highest allowable working stresses approved for any stress-graded southern pine lumber. For the latter reason, dense select structural southern pine requires especially careful inspection during grading to assure compliance with performance anticipations.

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\* "Testing of southern pine has been for the most part on small clear specimens, and tests have not been made on structural sizes in commercial grade groups which would show the expected range in strength properties." (Correspondence of August 11, 1949, from Forest Products Laboratory to the author.)

1. Southern Pine Association, "Lumber Production," New Orleans: 1947.

The grading of lumber, according to the provisions given in Standard Grading Rules for Southern Pine Lumber,<sup>2</sup> cannot be considered an exact science because it is based on visual inspection of each piece and on the judgment of the grader. No arbitrary rules for inspection can be maintained with full satisfaction; therefore, exceptions from any given rule are suggested on the basis of good judgment. On the other hand, the grading provisions are sufficiently explicit to establish an amount of five percent below grade as a reasonable variation in judgment between graders and inspectors.

The reasons for difficulties in carrying out with satisfaction definite grading rules result from the characteristic variations in any piece of wood. Because of the nonhomogeneity of the material and these difficulties in grading, variations in physical and mechanical properties within a specific structural grade of southern pine are expected to occur. These variations are accepted within given specified ranges taken into consideration in setting up design specifications.

The investigation was undertaken specifically in order to: (1) determine whether a shipment of 5,591 board feet of carefully inspected dense select structural southern pine is within grade limitations. (2) indicate the existing variations and ranges in physical and mechanical properties within this shipment of lumber. (3) determine if this shipment fulfills anticipations of design specifications.

All test data, except those found in U. S. Forest Products Laboratory publications, were obtained in the V.P.I. Wood Research Laboratory under

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2. Southern Pine Inspection Bureau of the Southern Pine Association, Standard Grading Rules for Southern Pine Lumber, New Orleans: 1948, p. 8.

the auspices of the V.P.I. Research Foundation, Inc. Funds for the investigation were provided from a Frederick Gardner Cottrell Grant of the Research Corporation to the V.P.I. Wood Research Laboratory for a study of box columns, this investigation being part thereof. Gratitude is expressed to E. George Stern, Research Professor of Wood Construction, for the recommendation of this study and his guidance throughout the investigation. Acknowledgment is made to B. Y. Kinzey, Jr., Assistant Professor of Architecture, for his advice and help during the performance of the tests. Appreciation is extended to the Roper Bros. Lumber Company, Inc., of Petersburg, Virginia, and to Mr. R. C. Gregory, Inspector of the Southern Pine Inspection Bureau of the Southern Pine Association, for selecting, grading, and certifying the lumber under investigation before its shipment to the V.P.I. Wood Research Laboratory.

## II. DESCRIPTION OF INVESTIGATION

### A. Procedure

The variations in physical properties within a random-selected 2-in. by 8-in. by 8-ft. sapwood plank were determined from 1/2-in. thick consecutive full cross-sectional specimens. Variations in physical properties within 317 planks were found according to similar specimens sawn from an end of 8-ft. planks or from mid-length of 16-ft. planks.

The number of annual rings per inch was measured according to rules given in the Standard Grading Rules for Southern Pine Lumber.<sup>3</sup>

The percentage of summerwood was estimated visually for each specimen.

The specific gravity was determined upon drying the specimens at 97° C. in an electric oven; weighing them with a triple-beam balance, having a sensitivity of 0.01 g. and a capacity of 200 g.; and weighing their volumes when submerged in mercury.

Minor bending tests were made at a constant testing speed of 0.100 in. per min. with a Timus Olsen testing machine, having a sensitivity of 50 lbs. and a capacity of 60,000 lbs.

### B. Material

The tests were performed on dense select structural southern pine planks, originating in the area of Petersburg, Virginia. These planks were sawn, air-dried, and planed in the mill of the Roper Brothers Lumber Company, Inc. of Petersburg, Virginia. Each plank was carefully inspected and stamped by an official inspector of the Southern Pine Inspection Bureau. Some of the planks were resawn, planed, and reinspected before their shipment to the V.P.I. Laboratory, where the lumber was stacked and stickered until used for test purposes.

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3. Ibid. p. 47

### III. TEST DATA

#### A. Physical Properties

The test data on variations in physical properties within a random-selected dense structural southern pine sapwood plank according to 136 consecutive specimens are tabulated in Appendix A. These data include the number of annual rings per inch, percentage of summerwood, and specific gravity.

The test data on variations in the above physical properties for specimens from the 317 dense select structural southern pine planks are tabulated in Appendix B.

#### B. Mechanical Properties

Minor test data on eight random-selected specimens tested in flexure for determination of the moduli of elasticity are given in Appendix C.



#### IV. TEST RESULTS

##### A. Variations in Physical Properties

###### 1. Specific Gravity

Specific-gravity limitations are not given in the grading of lumber because of the necessity of destructive sampling and the time required for sawing specimens and performing the tests. However, this property is limited indirectly by requirements concerning the number of annual rings per inch and the percentage of summerwood.

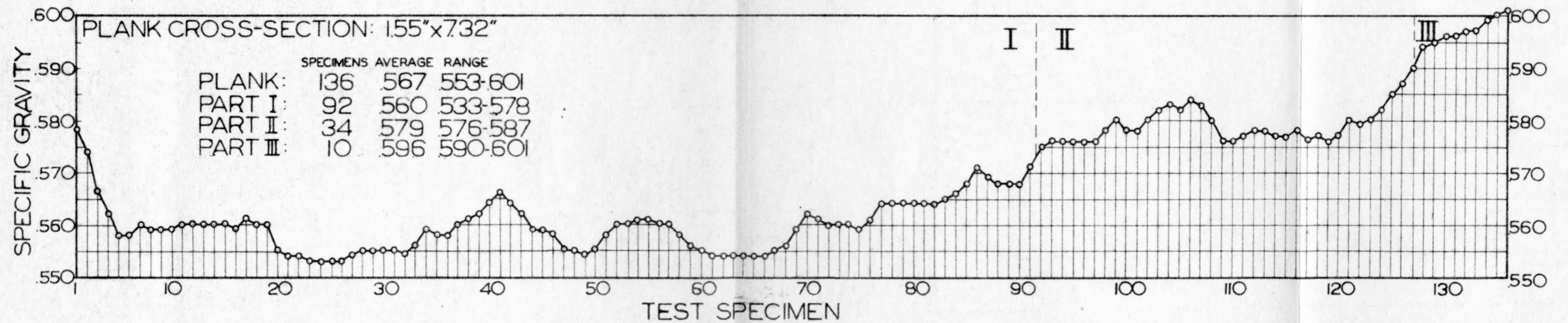
The variation in specific gravity according to the 136 specimens sawn from the random-selected sapwood plank is shown on Chart 1. The specific gravity ranges from 0.553 to 0.601 with an average of 0.567. As shown on Chart 2, ninety-five percent of the specific-gravity values are within the range of 0.553 to 0.594, and 50 percent of the values are within the range of 0.560 to 0.580.

The specific-gravity frequency distribution of this particular plank appears to fall into three groups. These groups are fully attributable to origin of samples within the tested plank, as shown on Chart 1.

The variation in specific gravity within the 315 planks tested is given on Chart 3. The specific gravity ranges from 0.448 to 0.745 with an average of 0.581. Ninety-five percent of the values are within the range from 0.490 to 0.700. Fifty percent of the values are within the range from 0.550 to 0.616.

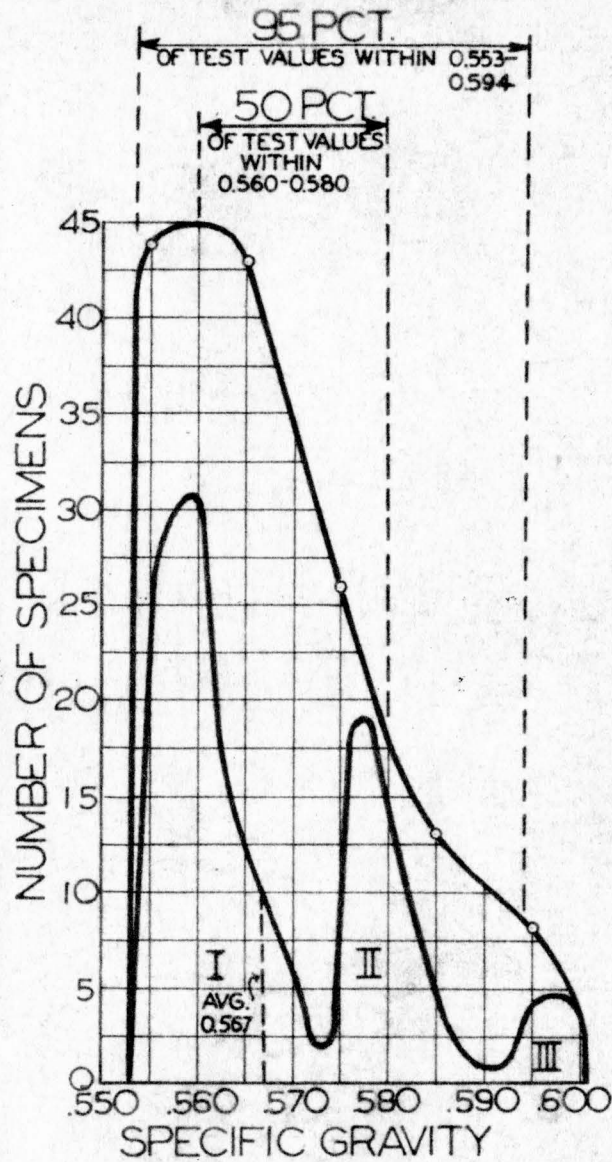
CHART I

# VARIATION IN SPECIFIC GRAVITY WITHIN 8-FT. DENSE STRUCTURAL SOUTHERN PINE SAPWOOD PLANK



SPECIFIC-GRAVITY DISTRIBUTION WITHIN PLANK

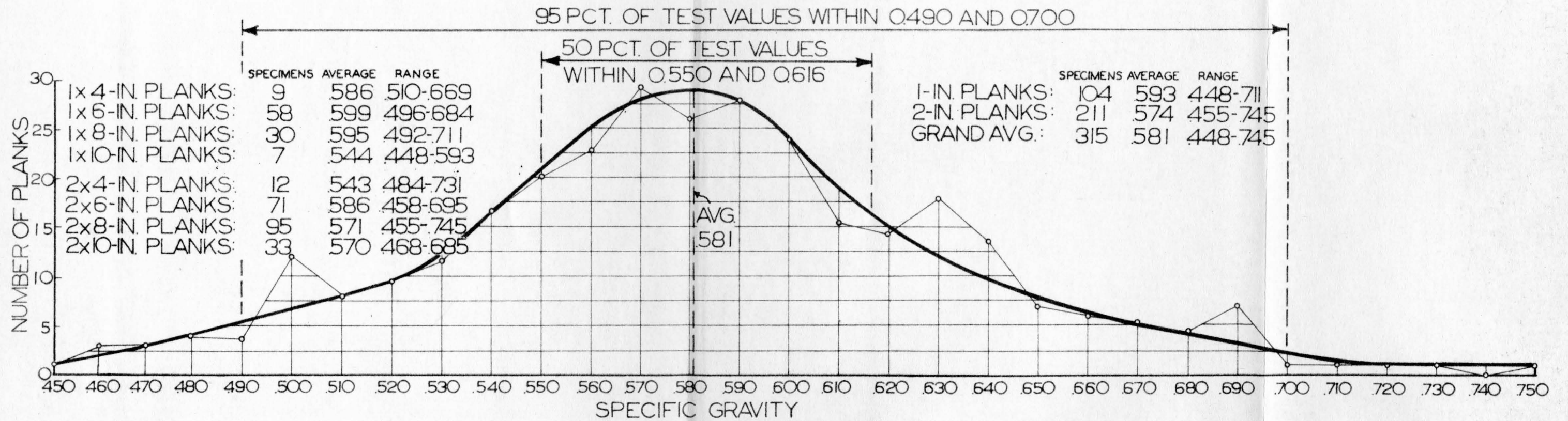
# VARIATION IN SPECIFIC GRAVITY WITHIN 8-FT. DENSE STRUCTURAL SOUTHERN PINE SAPWOOD PLANK



AVERAGE FREQUENCY-DISTRIBUTION CURVE

CHART 3

# VARIATION IN SPECIFIC GRAVITY OF 315 16-FT. DENSE STRUCTURAL SOUTHERN PINE PLANKS



AVERAGE FREQUENCY-DISTRIBUTION CURVE

## 2. Number of Annual Rings and Percentage of Summerwood

According to the grading rules for southern pine lumber,<sup>4</sup> a plank must have a minimum of six annual rings per inch, and in addition, one-third or more summerwood to meet the requirements of dense structural grade. However, a plank with less than six annual rings per inch meets the requirements of this grade if it averages one-half or more summerwood.

The number of annual rings per inch for the 317 planks ranges, as shown on Chart 4, from 4 to 19 with an average of 9. Ninety-five percent of the test values range between 5 and 16 annual rings per inch. Fifty percent of the test values are within the range from 8 to 11.

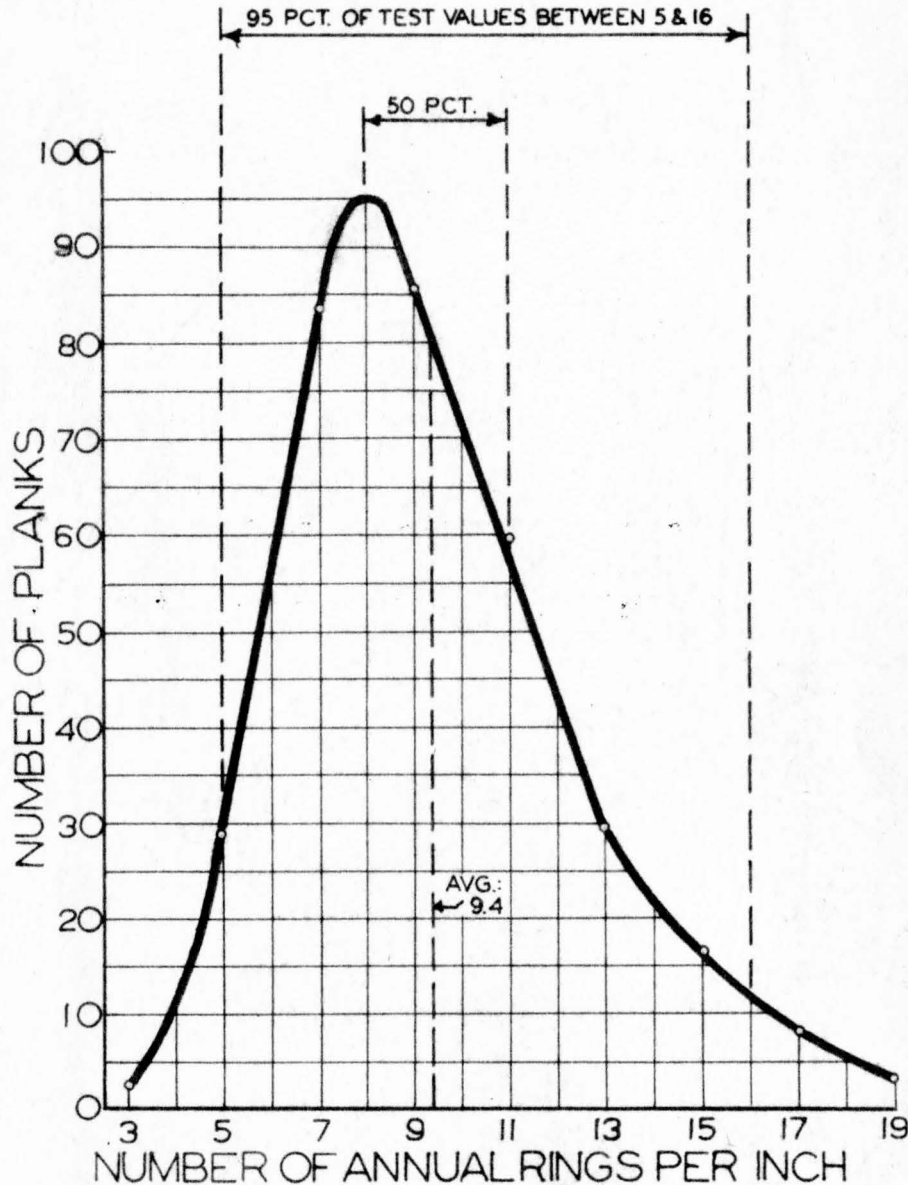
The amount of summerwood for the 317 planks ranges, as shown on Chart 5, from 25 to 65 percent with an average of 44 percent. Ninety-five percent of the test values range from 30 to 55 percent. Forty-six percent of the test values are within the range from 40 to 45 percent.

According to the above grading rules, 23 planks, that is 7.3 percent, do not meet grade requirements. This amount is 2.3 percent more than the 5 percent below grade established as a reasonable variation in observation. Of these 23 planks, 13 fail to meet the requirements for dense structural southern pine because each contains from one-third to one-half summerwood and less than six annual rings per inch. Ten planks, including two which average less than six annual rings per inch, fail to meet the above requirements because each has less than one-third summerwood.

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4. Ibid. p. 47.

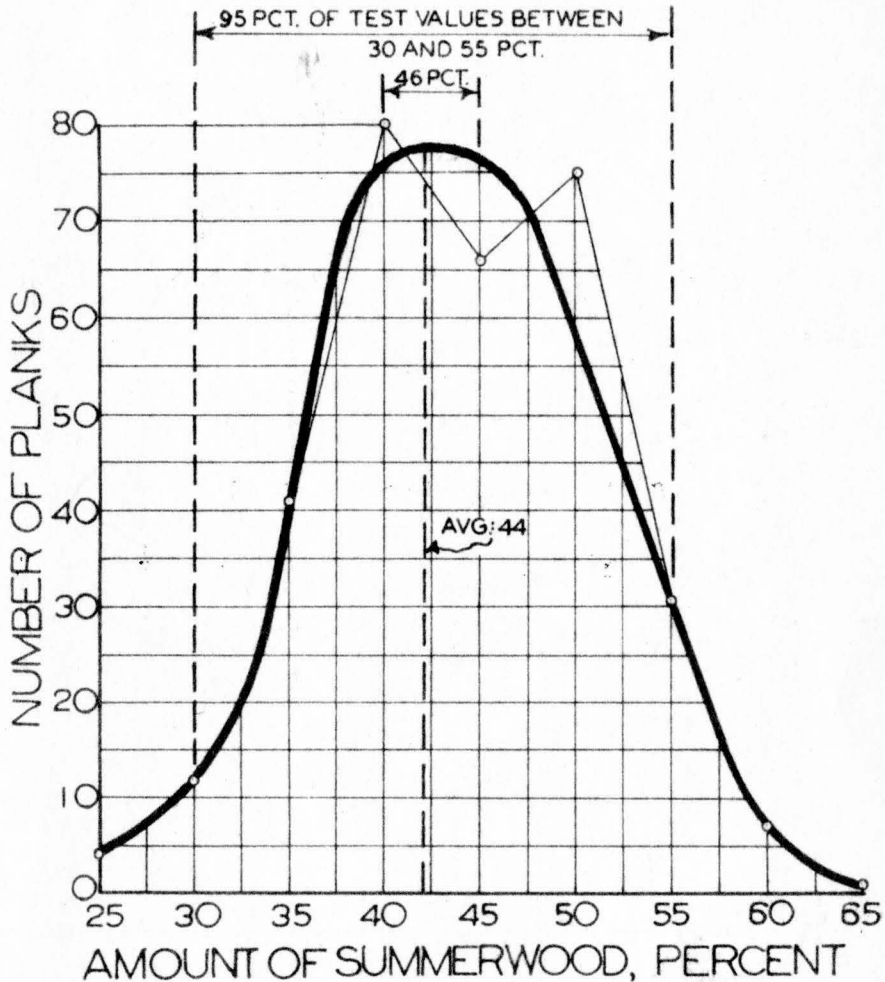
# VARIATION IN NUMBER OF ANNUAL RINGS PER INCH OF 317 16-FT. DENSE STRUCTURAL SOUTHERN PINE PLANKS



## AVERAGE FREQUENCY-DISTRIBUTION CURVE

	SPECIMENS	AVERAGE	RANGE
1x4-IN. PLANKS:	9	10	7-13
1x6-IN. PLANKS:	58	10	5-18
1x8-IN. PLANKS:	30	9	5-15
1x10-IN. PLANKS:	7	7	5-9
2x4-IN. PLANKS:	12	11	5-15
2x6-IN. PLANKS:	72	9	5-16
2x8-IN. PLANKS:	96	9	4-18
2x10-IN. PLANKS:	33	9	4-19
1-IN. PLANKS:	104	10	5-18
2-IN. PLANKS:	213	9	4-19
AVERAGE:	317	9	4-19

# VARIATION IN AMOUNT OF SUMMERWOOD OF 317 16-FT. DENSE STRUCTURAL SOUTHERN PINE PLANKS



## AVERAGE FREQUENCY-DISTRIBUTION CURVE

	SPECIMENS	AVERAGE	RANGE
1x4-IN. PLANKS:	9	47	40-50
1x6-IN. PLANKS:	58	46	25-60
1x8-IN. PLANKS:	30	48	40-60
1x10-IN. PLANKS:	7	42	25-55
2x4-IN. PLANKS:	12	35	30-40
2x6-IN. PLANKS:	72	45	35-65
2x8-IN. PLANKS:	96	42	25-55
2x10-IN. PLANKS:	33	42	30-50
1-IN. PLANKS:	104	46	25-60
2-IN. PLANKS:	213	43	25-65
AVERAGE:	317	44	25-65

### B. Variations in Mechanical Properties

Since a considerable range in specific gravity exists within the test material, a similar range in mechanical properties may be expected. The modulus of elasticity for eight random-selected specimens which were tested in flexure varies from 1,560,000 to 2,230,000 psi, that is -24 and +9 percent from the average of 2,040,000 psi.



## V. CORRELATION OF PHYSICAL AND MECHANICAL PROPERTIES

### A. Specific Gravity as Related to Mechanical Properties

The strength of wood is indicated by its density, which in the metric system is numerically equal to its specific gravity. Some mechanical properties of wood vary directly with the specific gravity, while others, such as modulus of rupture and fiber stress at proportional limit in flexure, vary with a power of specific gravity larger than one.

General formulas have been established<sup>5</sup> to determine the mechanical properties of wood on the basis of average relations of specific gravity to strength properties among different species. However, these formulas are not fully applicable to all individual wood species for the following reasons: (1) Some species of wood contain relatively large amounts of resins, gums, and other extractives, which increase the specific gravity but do not contribute as much to the strength as would a like amount of wood substance. (2) The structural arrangement of fibers varies among different species.

A statistical analysis of the flexural test data, given in Appendix D, indicates that for the tested southern pine, the difference between the modulus of elasticity as computed from flexure tests and the modulus of elasticity as computed from specific-gravity data according to the above general formulas is statistically insignificant. Consequently, the use of the general formulas seems to be justified for determining modulus of elasticity and other mechanical properties of the test material.

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5. Markwardt, L. J. and Wilson, T. R. C., Strength and Related Properties of Woods Grown in the United States, Washington: United States Department of Agriculture. September, 1935, p. 37.

The ranges for moduli of elasticity and other mechanical properties, as computed according to the general Forest Products Laboratory formulas and specific-gravity test data, are presented in Table I, which indicate the formulas used, extreme ranges, 95 percent ranges, and average values.

**B. Number of Annual Rings and Percentage of Summerwood  
as Related to Mechanical Properties**

The average number of annual rings per inch within a plank is not a direct indication of the plank's mechanical properties. These annual rings merely show the tree's favorable or unfavorable rate of growth, which may or may not seriously affect the strength of the wood within the tree. Wood which is grown at a medium rate of growth in a tree supplied with plenty of plant food and a reasonable amount of water has better mechanical properties than does the same volume of wood grown at a very fast rate and formed from a sparse amount of plant food and an abundance of water. For this reason, the number of annual rings per inch alone, without consideration of the wood density, is no true criterion upon which to base the mechanical properties of wood.

The above statements are substantiated by the reported findings. Thirteen of the 23 planks fail to meet grade requirements for dense select structural southern pine because each contains between one-third and one-half summerwood and less than six annual rings per inch. However, the specific gravity for each of these planks is well above 0.490, which is the lower limit for the 95 percent range for specific gravity of the tested material. Furthermore, the specific-gravity values for four of these specimens are above the average value of 0.581 for the 315 tested planks. The structural use of these 13 planks appears justified as their mechanical properties, computed

TABLE I

Ranges in Mechanical Properties within 315 Dense Select Structural Southern Pine Planks as Expected According to Specific-Gravity Test Data

Property	Formula	Extreme Range	Ninety-Five Percent Range	Average
Specific Gravity		0.448 - 0.745	0.490 - 0.700	0.581
Static Bending:		Psi	Psi	Psi
Fiber Stress at Proportional Limit	$7,700 \left( \frac{G}{0.54} \right)^{1.25}$	6,100 - 11,500	6,810 - 10,600	8,440
Modulus of Rupture.....	$12,800 \left( \frac{G}{0.54} \right)^{1.25}$	10,100 - 19,100	11,200 - 17,400	14,000
Modulus of Elasticity.....	$\frac{1,760,000G}{0.54}$	1,460,000 - 2,430,000	1,600,000 - 2,280,000	1,890,000
Compression Parallel to Grain:				
Fiber Stress at Proportional Limit	$\frac{5,090G}{0.54}$	4,220 - 7,020	4,620 - 6,600	5,480
Maximum Crushing Strength.....	$\frac{7,700G}{0.54}$	6,390 - 10,600	6,990 - 9,980	8,280
Compression Perpendicular to Grain:				
Fiber Stress at Proportional Limit	$1,000 \left( \frac{G}{0.54} \right)^{2.25}$	659 - 2,060	803 - 1,790	1,180

Note: G represents specific gravity of oven-dry specimen.

according to previously given formulas, are high in value, even though the planks are expected to be below grade according to grading rules.

The remaining 10 of these 23 planks fail to meet the grade requirements because each contains less than one-third summerwood. The structural use of these 10 planks is not recommended for construction specifying dense select structural southern pine because all of the planks have specific-gravity values of less than 0.490, except two, which are only 3 percent and 1.6 percent above the value of 0.490.

From the previous cases, it appears that in the southern pine tested, the percentage of summerwood, as an indication of density, is of more importance than the number of annual rings per inch in estimating the mechanical properties of the material. This fact can be explained on the basis that an annual ring is composed of both springwood and summerwood. Summerwood is normally harder, stronger, and less brash than the springwood. Consequently, a higher percentage of summerwood gives higher mechanical properties of the material.

## VI. ANTICIPATED PROPERTIES VS. TEST PROPERTIES

A comparison of the test properties, shown in Table I, with basic information from which National Design Specification<sup>6</sup> design data were derived, may be made to determine whether the investigated shipment of lumber fulfills anticipations of this specification. The allowable design stresses were derived from U. S. Forest Products Laboratory average test data, as summarized in Technical Bulletin No. 479.<sup>7</sup> These average data and the corresponding 95 percent ranges, the latter according to Forest Products Laboratory files,<sup>8</sup> are compared in Table II with the respective properties of the investigated lumber. For comparative purposes, correction is made for the influence of seasoning since the design data were derived from test values for green material and the investigated shipment of lumber was tested when air-dry. This correction is made on the basis of data given in Table I of the above Technical Bulletin.

According to Table II, the 95 percent ranges of all test properties given for the investigated material are within the 95 percent ranges for the corresponding Forest Products Laboratory data. Furthermore, the minimum values for the extreme, that is, 100 percent ranges are greater than the corresponding values for the 95 percent ranges of the Forest Products Laboratory data. Hence, all of the investigated lumber fulfills anticipations of the National Design Specification despite the following facts:

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6. National Lumber Manufacturers Association, National Design Specification for Stress-Grade Lumber and Its Fastenings, Washington: 1948, p. 12.
  7. Markwardt, L. J. and Wilson, T. R. C., Strength and Related Properties of Woods Grown in the United States, Washington: United States Department of Agriculture. September, 1935, (Technical Bulletin No. 479), Table I.
  8. Correspondence of August 11, 1949, from Forest Products Laboratory to the author.

TABLE II

Properties According to Forest Products Laboratory Data  
for Small Clear Green Longleaf-Pine Test Specimens and for  
315 Dense Select Structural Southern Pine Planks, Corrected to Green Condition

(All Mechanical Properties in Psi.)

Property	F.P.L. Test Data		V.P.I. Test Data		
	95 Pct. Range	Average	95 Pct. Range	Extreme Range	Average
Specific Gravity	0.390 - 0.700	0.620	0.490 - 0.700	0.448 - 0.745	0.581 (-6.3%)*
<u>Static Bending:</u> Fiber Stress at Proportional Limit	2,500 - 6,000	5,200	3,800 - 5,900	3,400 - 6,400	4,720 (-9.2%)
Modulus of Rupture	-	8,700	6,600 - 10,000	6,000 - 11,000	8,290 (-4.7%)
Mod. of Elasticity	1,100,000 - 2,500,000	1,600,000	1,300,000 - 1,800,000	1,200,000 - 2,000,000	1,520,000 (-5.0%)
<u>Compression Paral- lel to Grain:</u> Fiber Stress at Proportional Limit	1,500 - 4,500	3,430	2,600 - 3,700	2,400 - 3,900	3,120 (-9.0%)
<u>Compression Perpen- dicular to Grain</u> Fiber Stress at Proportional Limit	200 - 1,000	590	400 - 900	327 - 1,000	585 (-0.8%)

\* Percentage Variation from F.P.L. Average.

(1) The average test data for the investigated lumber are from 0.8 to 9.2 percent lower than the corresponding average Forest Products Laboratory data. (2) According to the design of the standard grading rules, 7.3 percent of the 315 planks are found to be below grade.

The average specific gravity for the tested lumber, which is 6.3 percent smaller than that determined for the Forest Products Laboratory tested material, results in such smaller average strength properties as listed in Table II. These decreases are of no appreciable influence on the design data computed for the tested lumber. Analogous derivation of the flexural design stress for dense select structural southern pine according to both F.P.L. and V.P.I. data is presented in Table III. From an average modulus of rupture of 8,700 psi., according to F.P.L. tests, the design stress of 2,400 psi. is derived which corresponds to that given in the National Design Specification. From the corresponding V.P.I. data of 8,290 psi., the design stress of 2,320 psi. may be derived, if the F.P.L. variability factor of one-quarter is employed. However, a design stress of 2,470 psi. is computed if the variability factor of one-fifth for 95 percent of the tested 315 planks is used.

The presented data for the investigated lumber substantiate the justification of the use of a flexural design stress of 2,400 psi., as recommended since 1944 in the National Design Specification for dense select structural southern pine.

This design stress of 2,400 psi. was increased only on the basis of additional field experience with erected structures from 2,000 psi., recommended prior to 1944. Tests never were performed on commercially stress-graded lumber in order to verify the greater expected strength properties. Since this investigation of a carefully stress-graded and certified lumber

TABLE III

Derivation of Design Data for Flexural Stresses  
for  
Dense Select Structural Southern Pine

(All Stresses in Psi.)

		F.P.L. Derivation	V.P.I. Derivation for Variability of	
			1/4	1/5
Test Stress	Average Modulus of Rupture.....	8,700	8,290	8,290
	Deduct 1/4 or 25% for F.P.L. Variability....	6,525	6,218	6,632
	Deduct 7/16 or 43.7% for Time Effect.....	3,670	3,498	3,730
	Deduct 1/3 or 33.3% for Size Effect.....	2,437	2,332	2,487
Basic Stress	Add 1/20 or 5% for Field Experience.....	2,559	2,449	2,611
	Deduct (1/4) or 26% for Strength Ratio of 74% Plus 1/2 or 50% of Strength Ratio above 50% for Dry Location, that is, a total of (1/7) or 14%.....	2,201	2,106	2,245
Working Stress	Add 1/10 or 10% for Normal Loading.....	2,421	2,317	2,469
	Rounding Out.....	2,400	2,400	



shipment gives satisfactory evidence that the investigated anticipated strength properties are justified, the corresponding design data recommended since 1944 can be considered trustworthy and dependable.

## VII. SUMMARY

- (1) The shipment of 5,591 board feet of dense select structural southern pine, graded and inspected to conform to the standard grading rules for southern pine lumber, contained 23 of 317 planks, that is 7.3 percent, which do not fully meet grade requirements on the basis of careful laboratory inspection of full cross-sectional specimens sawn from mid-length of 16-ft. or an end of 8-ft. planks. This amount is 2.3 percent more than the 5 percent below grade established as a reasonable variation in judgment between graders and inspectors. However, the use of 13 of the 23 planks as dense select structural grade may be justified because of satisfactory specific-gravity values.
- (2) The variation in specific gravity within a random-selected sapwood plank ranges from -2.5 to +6.0 percent of its average value of 0.567.

The variation in specific gravity within the 315 planks tested ranges from -23 to +28 percent of its average value of 0.581. This latter value is 6.3 percent smaller than the respective value given by the Forest Products Laboratory for this grade of southern pine. Correspondingly, the computed average flexural fiber stress at proportional limit is 9 percent smaller, the modulus of rupture and flexural modulus of elasticity are 5 percent smaller, and the compression-parallel and perpendicular-to-grain fiber stresses at proportional limit are 9 and 1 percent smaller, respectively.

The computed 95 percent ranges for all test properties for the investigated material are within the 95 percent test ranges for the corresponding Forest Products Laboratory data, and the computed minimum values for the extreme ranges are greater than the corresponding

values for the 95 percent ranges of the Forest Products Laboratory data.

- (3) Since derivation of flexural design stresses for the investigated lumber shipment indicates that the use of a design stress of 2,400 psi. is justified, the shipment fulfills the anticipation of the National Design Specification. Hence, the flexural design data for the investigated lumber grade, as recommended in the National Design Specification, can be considered fully reliable in light of this investigation.

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APPENDIX A

VIRGINIA POLYTECHNIC INSTITUTE  
Engineering Experiment Station  
WOOD RESEARCH LABORATORY  
DATA SHEET

Page 27

Project No: Res. Found. No. 117 Series No.: S. Y. P.

Date: March, 1949

Project: Variations in Physical Properties of a 2" x 8" x 8' Dense Select Structural Southern Pine Plank (Sapwood Only)

Observers: O. J. Blake; E. G. Stern

Testing Machine: Triple-Beam Balance, Mercury Volumeter

Test Number	No. of Rings	Pct. of Summerw.	Oven-Dry Weight	Weight In Merc.	Specific Gravity
1	5 $\frac{1}{2}$	35	52.94	5215	.578
2	5 $\frac{1}{2}$	35	57.78	5745	.574
3	5 $\frac{1}{2}$	35	53.01	5345	.566
4	5 $\frac{1}{2}$	35	51.15	5190	.562
5	5 $\frac{1}{2}$	40	53.20	5435	.558
6	5 $\frac{1}{2}$	40	58.37	5950	.558
7	5 $\frac{1}{2}$	35	60.82	6190	.560
8	5 $\frac{1}{2}$	35	56.63	5805	.559
9	5 $\frac{1}{2}$	35	58.45	5985	.559
10	5 $\frac{1}{2}$	35	58.36	5965	.559
11	5 $\frac{1}{2}$	35	58.73	5980	.560
12	5 $\frac{1}{2}$	35	56.71	5775	.560
13	5 $\frac{1}{2}$	35	55.69	5680	.560
14	5 $\frac{1}{2}$	35	58.29	5935	.560
15	5 $\frac{1}{2}$	35	56.91	5795	.560
16	5 $\frac{1}{2}$	35	55.48	5670	.559
17	5 $\frac{1}{2}$	35	57.49	5835	.561
18	5 $\frac{1}{2}$	35	58.80	6005	.560
19	5 $\frac{1}{2}$	40	57.20	5820	.560
20	5 $\frac{1}{2}$	40	57.22	5895	.555
21	5 $\frac{1}{2}$	40	54.11	5580	.554
22	5 $\frac{1}{2}$	40	57.81	5960	.554
23	5 $\frac{1}{2}$	40	56.72	5860	.553
24	5 $\frac{1}{2}$	40	55.88	5760	.553
25	5 $\frac{1}{2}$	40	57.25	5900	.553
26	5 $\frac{1}{2}$	40	58.47	6025	.553
27	5 $\frac{1}{2}$	40	58.19	5990	.554
28	5 $\frac{1}{2}$	40	55.31	5685	.555
29	5 $\frac{1}{2}$	40	58.32	6030	.555
30	5 $\frac{1}{2}$	35	56.84	5865	.555
31	5 $\frac{1}{2}$	35	58.96	6040	.555
32	5 $\frac{1}{2}$	35	58.50	6020	.554
33	5 $\frac{1}{2}$	35	57.31	5870	.556
34	5 $\frac{1}{2}$	35	55.82	5690	.559
35	5 $\frac{1}{2}$	35	55.62	5660	.558
36	5 $\frac{1}{2}$	35	58.91	6020	.558
37	5 $\frac{1}{2}$	35	60.90	6210	.560
38	5 $\frac{1}{2}$	35	59.25	6035	.561
39	5 $\frac{1}{2}$	35	59.05	6005	.562
40	5 $\frac{1}{2}$	35	57.56	5840	.564
41	5 $\frac{1}{2}$	35	57.23	5740	.566
42	5 $\frac{1}{2}$	35	57.76	5830	.564
43	5 $\frac{1}{2}$	35	57.43	5840	.562
44	5 $\frac{1}{2}$	35	59.63	6110	.559
45	5 $\frac{1}{2}$	35	55.84	5710	.559
46	5 $\frac{1}{2}$	35	56.65	5800	.558

47	5	35	57.24	5895	.555
48	5	35	58.22	5970	.555
49	5	35	58.25	5940	.554
50	5	35	56.82	5800	.555
51	5	40	57.69	5900	.558
52	5	40	58.61	5955	.560
53	5	40	57.10	5805	.560
54	5	40	57.27	5805	.561
55	5	40	57.44	5840	.561
56	5	40	57.17	5830	.560
57	5	40	57.23	5840	.560
58	5	40	57.90	5920	.558
59	5	40	57.09	5850	.558
60	5	40	57.18	5865	.557
61	5	40	57.76	5955	.554
62	5	40	58.42	6020	.554
63	5	40	57.47	5920	.554
64	5	40	55.09	5655	.554
65	5	40	57.88	5920	.554
66	5	40	57.87	5925	.554
67	5	40	55.04	5655	.555
68	5	40	56.29	5765	.556
69	5	40	57.28	5830	.559
70	5	40	58.88	5975	.562
71	5	40	55.60	5635	.561
72	5	40	59.15	6015	.560
73	5	40	57.33	5855	.560
74	5	40	55.52	5655	.560
75	5	40	56.78	5780	.559
76	5	40	57.24	5815	.561
77	5	40	59.28	5985	.564
78	5	40	60.11	6080	.564
79	5	40	56.75	5735	.564
80	5	40	56.61	5745	.564
81	5	40	57.12	5785	.564
82	5	40	60.19	6075	.564
83	5	40	55.94	5645	.566
84	5	40	53.76	5395	.567
85	5	40	56.90	5700	.568
86	5	40	55.82	5560	.571
87	5	40	59.61	5975	.569
88	5	40	53.24	5360	.568
89	5	40	58.84	5910	.568
90	5	40	56.31	5650	.568
91	5	40	59.85	5950	.571
92	5	40	59.18	5855	.575
93	5	40	56.45	5560	.576
94	5	40	58.59	5790	.576
95	5	40	58.45	5780	.576
96	5	40	58.14	5750	.576
97	5	40	57.58	5685	.576
98	5	40	59.13	5805	.578
99	5	40	59.13	5800	.580
100	5	40	58.83	5795	.578
101	5	40	57.25	5640	.578
102	5	40	53.63	5250	.580

103	5	40	58.50	5705	.582
104	5	40	59.12	5765	.583
105	5	40	57.02	5580	.582
106	5	40	58.93	5760	.584
107	5	40	56.55	5525	.583
108	5	40	57.85	5685	.580
109	5	40	54.32	5375	.576
110	5	40	59.36	5855	.576
111	5	40	59.67	5865	.577
112	5	40	58.18	5730	.577
113	5	40	59.59	5860	.578
114	5	40	58.00	5710	.577
115	5	40	56.37	5540	.577
116	5	40	56.64	5555	.579
117	5	40	56.72	5615	.576
118	5	40	58.56	5765	.577
119	5	40	59.08	5845	.576
120	5	40	58.87	5795	.577
121	5	40	57.98	5655	.580
122	5	40	59.47	5855	.579
123	5	40	58.21	5700	.580
124	5	40	58.77	5725	.582
125	5	40	56.30	5475	.585
126	5	40	59.40	5735	.587
127	5	40	59.20	5680	.590
128	5	40	57.93	5520	.594
129	5	40	58.54	5575	.595
130	5	40	59.83	5690	.596
131	5	40	60.64	5765	.596
132	5	40	59.31	5625	.597
133	5	40	59.85	5680	.597
134	5	40	59.44	5625	.599
135	5	40	59.32	5580	.600
136	5	40	62.71	5900	.601

AVG. .567



APPENDIX B

VIRGINIA POLYTECHNIC INSTITUTE  
Engineering Experiment Station  
WOOD RESEARCH LABORATORY  
DATA SHEET

Page 31

Project No: Res. Found. No. 117, Series No.: S. Y. P.

Date: March, 1949

Project: Variations in Physical Properties of Dense Select Structural Southern Pine

Observers: O. J. Blake; E. G. Stern

Testing Machine: Triple-Beam Balance, Mercury Volumeter

Test Number	Plank Size	Plank Sign	No. of Rings Per In.	Pct. of Summerw.	Pct. of Heartw.	Oven-Dry Weight g.	Weight In Merc. g.	Specific Gravity
1	1"x4"	JA33Nac	11	50	0	15.00	1400	.607
2		JA33Nbd	12	45	0	15.03	1450	.589
3		JA33'Nac	10	45	0	10.33	1100	.535
4		JA33'Nbd	9	40	0	12.40	1240	.568
5		JA33"Nac	8	50	0	12.94	1215	.603
6		JA33"Nbd	12	45	0	12.00	1340	.510
7		JB34Nd	7	50	0	12.20	1030	.669
8		JB34'Nbd	8	55	0	12.76	1140	.630
9		JB34"Nbd	13	40	0	8.18	830	.560
Avg.								.586
10	1"x6"	JB34Nb	17	50	0	15.77	1390	.640
11		JB34Nac	13	55	0	18.80	1685	.633
12		JB34'Nac	16	55	0	19.06	1665	.649
13		JB34"Nac	9	50	0	18.97	1725	.622
14		IA31Nac	8	45	0	21.30	1975	.612
15		IA31Nbd	13	40	15	21.17	2010	.597
16		IA31'Nac	10	40	0	18.80	1910	.560
17		IA31'Nbd	14	50	0	20.57	1945	.600
18		IA31"Nac	9	50	0	21.44	1975	.615
19		IA31"Nbd	8	55	0	21.91	1970	.631
20		IB32Nbd	6	60	0	14.80	1530	.550
21		IB32'Nbd	9	45	0	14.42	1540	.535
22		IB32"Nac	7	40	0	14.97	1480	.575
23		IB32"Nbd	7	35	0	16.53	1690	.557
24		GA23Na	10	35	0	17.65	1725	.581
25		GA23Nb	11	50	0	21.20	1830	.656
26		GA23Nc	14	45	0	17.31	1600	.612
27		GA23Nd	10	50	0	18.01	1630	.625
28		GA23'Na	5	45	0	17.51	1680	.590
29		GA23'Nb	8	55	0	13.95	1255	.629
30		GA23'Nc	14	40	50	18.08	1760	.584
31		GA23'Nd	9	50	0	19.47	1735	.635
32		GA23"Nac	9	50	0	23.30	1960	.673
33		GA23"Nb	13	35	0	9.90	995	.565
34		GA23"Nc	8	35	0	17.07	1745	.556
35		GA23"Nd	12	55	0	12.62	1035	.685
36		GA24Na	9	45	0	18.02	1660	.616
37		GA24Nb	12	55	0	31.68	2700	.663
38		GA24Nc	12	35	0	17.60	1780	.564
39		GA24Nd	10	35	0	17.43	1690	.585
40		GA24'Na	7	40	0	16.90	1595	.600
41		GA24'Nb	11	45	0	14.36	1635	.500
42		GA24'Nc	10	35	0	19.54	1795	.619
43		GA24'Nd	8	35	0	18.07	1805	.570
44		GB25Na	8	45	0	19.73	1750	.638

45	gB25Nb	7	55	0	19.14	1695	.640
46	gB25Nc	18	50	0	17.07	1730	.560
47	gB25Nd	6	60	0	19.56	1850	.600
48	gB25'Na	9	50	0	19.32	1710	.640
49	gB25'Nb	18	40	25	17.75	1775	.570
50	gB25'Nc	10	45	0	18.86	1810	.592
51	gB25'Nd	9	50	0	16.83	1655	.580
52	gB25''Na	15	40	0	18.71	1775	.598
53	gB25''Nb	7	55	0	17.85	1625	.623
54	gB25''Nc	7	45	0	17.68	1780	.565
55	gB25''Nd	7	50	0	18.89	1685	.635
56	gBS26Na	6	25	0	14.90	1710	.498
57	gBS26Nb	12	50	68	18.09	1695	.605
58	gBS26Nc	10	35	69	14.59	1685	.496
59	gBS26Nd	18	50	70	16.88	1605	.597
60	gBS26'Na	7	35	71	16.21	1745	.532
61	gBS26'Nb	8	45	72	19.43	1690	.650
62	gBS26'Nc	15	50	73	13.97	1255	.630
63	gBS26'Nd	8	50	74	32.77	3300	.565
64	gBS26''Na	16	60	75	18.98	1690	.635
65	gBS26''Nb	10	55	76	26.77	2430	.629
66	gBS26''Nc	13	40	77	17.89	1580	.640
67	gBS26''Nd	10	50	78	28.22	3035	.532
				79			.599

Avg.

68	1"x8"	1B32Nac	11	50	80	18.84	1575	.675
69		1B32'Nac	11	60	81	23.94	2166	.627
70		gAS24''Na	14	55	82	24.35	2135	.646
71		gAS24''Nb	7	45	83	20.43	2180	.536
72		gAS24''Nc	8	50	84	22.63	2155	.597
73		gAS24''Nd	13	50	85	25.94	2120	.691
74		gB25Nac	9	50	86	20.17	1670	.683
75		gB25'Nac	7	45	87	23.77	2275	.593
76		gB25''Nac	15	45	88	25.47	2280	.633
77		gBS26Nac	7	40	89	20.27	2305	.504
78		gBS26'Nac	7	50	90	25.84	2305	.634
79		gBS26''Nac	5	40	91	7.86	900	.499
80		hA27Nac	8	40	92	23.36	2490	.535
81		hA27Nbd	12	50	93	22.74	2485	.524
82		hA27'Nac	7	45	94	16.25	1505	.611
83		hA27'Nbd	11	55	95	23.56	2425	.553
84		hA27''Nac	5	45	96	23.24	2470	.538
85		hA27''Nbd	15	55	97	25.70	2325	.627
86		hAS28Nac	6	40	0	33.22	3110	.605
87		hAS28Nbd	9	45	0	25.17	2060	.688
88		hAS28'Nac	9	50	0	23.32	2430	.547
89		hAS28'Nbd	8	45	0	23.67	2435	.553
90		hAS28''Nac	7	45	0	22.11	2345	.539
91		hAS28''Nbd	12	50	0	26.20	2360	.629
92		hB29Nbd	9	55	0	23.16	2070	.634
93		hB29N'bd	10	55	0	22.29	2105	.601
94		hB29N''bd	7	50	0	21.48	2145	.571
95		hBS30Nbd	7	50	0	21.93	2195	.570
96		hBS30'Nbd	7	40	0	19.90	2320	.492
97		hBS30''Nbd	12	60	0	26.01	2060	.711
								.595

Avg.

Small  
Fitch rockets



151	cB13"Na	10	55	0	48.44	3965	.690	Small Pitch Pocket
152	cB13"Ne	9	50	0	41.30	3800	.617	
153	bBS10Nac6	14	45	0	37.26	3625	.585	
154	bBS10Nd	15	55	0	11.43	1110	.580	
155	bBS10Nb	7	40	0	38.59	3765	.583	Small Pitch Pocket
156	bBS10'Nac6	12	55	0	34.34	3435	.570	
157	bBS10'Nd	10	45	0	33.40	3540	.539	
158	bBS10'Nb	9	40	0	32.37	3485	.530	
159	bBS10"Nac6	7	40	0	40.79	3695	.627	
160	bBS10"Nd	9	45	0	40.33	3565	.641	
161	bBS10"Nb	8	50	0	40.87	3805	.610	
162	bb9Nac6	10	45	20	35.02	3175	.625	Small Pitch Pocket
163	bb9Nb	8	35	15	24.29	2650	.524	Small Pitch Pocket
164	bb9Nd	13	50	0	45.27	4355	.592	
165	bb9'Nac6	7	45	0	36.05	3530	.582	
166	bb9'Nb	8	45	0	28.33	3000	.542	
167	bb9'Nd	10	40	0	23.62	2450	.549	
168	bb9"Nac6	11	45	0	34.79	3520	.563	
169	bb9"Nb	6	35	0	42.06	4075	.588	
170	bb9"Nd	11	55	0	42.70	3465	.695	
171	bAS8Na	10	50	0	39.46	3445	.647	
172	bAS8Nb	10	40	0	34.18	3465	.562	
173	bAS8Nc	12	40	5	32.11	3045	.598	
174	bAS8Nd	8	50	0	43.14	3645	.670	
175	bAS8'Na	9	40	0	30.59	2955	.589	
176	bAS8'Nb	7	30	0	29.15	4070	(.414)	
177	bAS8'Nc	11	50	0	30.74	3115	.562	
178	ba7Na	12	45	0	38.95	3725	.592	
179	ba7Nb	12	55	0	36.51	3415	.607	
180	ba7Nc	9	40	0	36.44	4010	.520	
181	ba7Nd	8	40	0	35.06	3550	.562	
182	ba7'Na	11	35	15	38.70	3715	.592	Small Pitch Pocket
183	ba7'Nb	8	40	0	39.62	3540	.635	
184	ba7'Nc	11	45	15	37.36	3635	.585	
185	ba7'Nd	7	45	0	39.15	3630	.612	
186	ba7"Na	8	40	0	12.69	1200	.598	
187	ba7"Nb	9	45	10	37.28	3625	.585	
188	ba7"Nc	6	40	0	14.05	1615	.497	Pitch Pocket
189	ba7"Nd	12	45	65	43.75	3915	.633	
Avg.							.586	

191	eB20NGac	13	40	20	38.48	4290	.513	
192	eB20'NGac	8	50	0	33.73	3505	.548	
193	eB20"NGac	11	35	0	36.45	4260	.490	
194	eB19Nac	7	35	0	35.12	3785	.530	
195	eB19'Nac	10	45	10	41.00	3745	.620	
196	eB19"Nac	9	40	0	39.18	4050	.548	
197	dB16Nbd	7	50	0	34.96	3190	.620	
198	dB16'Nbd	9	35	0	36.39	3610	.575	
199	dB16"Nbd	7	45	5	37.77	3715	.578	
200	dA15Nac	8	35	0	37.42	3820	.558	
201	dA15Nbd	8	40	0	28.32	3388	.479	
202	dA15'Nac	4	45	0	44.03	4435	.567	
203	dA15'Nbd	8	35	0	38.61	4350	.508	
204	dA15"Nac	16	40	35	40.58	4055	.571	
205	dA15"Nbd	11	40	50	17.61	2230	.455	

2" x 8"

206	cBL4NGa6	10	40	0	41.67	3900	.606	
207	cBL4NGac	11	45	10	38.68	3890	.566	
208	cBL4"NGac	13	50	5	40.44	3835	.598	
209	cBL4"NGac	7	45	30	35.28	3530	.568	
210	cBL3Nac	16	40	35	32.60	3325	.558	
211	cBL3"Nac	7	45	0	46.53	4470	.591	
212	cBL3"Nac	6	30	20	37.50	3860	.554	
213	bBS10Nac	8	40	15	45.33	4110	.625	
214	bBS10"Nac	9	50	0	23.28	2175	.603	Close to Knot
215	bBS10"Nc	10	35	0	24.24	2505	.550	Small Pitch Pocket
216	bBS10"Na	9	35	0	39.29	3955	.567	
217	bb9Nac	7	40	0	36.63	3600	.578	
218	bb9"Nac	11	45	0	35.73	3850	.530	
219	bb9"Nac	8	40	5	18.08	1660	.617	Pitch
220	bAS8"Nd	11	40	50	31.06	2905	.604	
221	bAS8"Na	7	45	95	41.43	3410	.685	Pitch Pocket
222	bAS8"Nc	8	35	0	37.02	3890	.543	
223	bAS8"Nd	11	40	0	40.82	4275	.548	
224	bAS8"Nb	13	45	1	26.46	2780	.544	
225	aB(s)6NGac	11	45	5	36.74	4175	.505	
226	aB(s)6NGd	5	50	0	51.21	5010	.580	
227	aB(s)6NGb	8	55	25	52.19	4480	.660	Small Pitch Pocket
228	aB(s)6"NGd	12	50	0	22.55	1530	(.812)	Pitch
229	aB(s)6"NGac	8	50	1	24.60	2030	.685	
230	aB(s)6"NGb	7	30	0	35.25	4010	.505	
231	aB(s)6"NGac	8	45	0	30.61	2980	.583	
232	aB(s)6"NGd	5	55	0	14.98	1295	.655	
233	aB(s)6"NGb	10	35	0	37.40	4175	.514	
234	aBS5Nac	7	35	10	37.11	3925	.540	
235	aBS5Nb	6	35	0	32.72	3630	.516	
236	aBS5Nd	6	35	0	47.81	5180	.551	
237	aBS5"Nac	14	40	0	32.81	3355	.560	
238	aBS5"Nb	10	50	25	38.55	3775	.581	
239	aBS5"Nd	8	35	45	51.33	5705	.515	Small Pitch
240	aBS5"Nac	10	35	2	39.99	4035	.564	
241	aBS5"Nb	6	50	0	57.69	5495	.595	
242	aBS5"Nd	7	45	0	53.58	5180	.583	
243	aB4Nac	9	40	0	16.30	1615	.575	Close to Knot
244	aB4Nb	10	35	0	68.16	6670	.582	
245	aB4Nd	10	50	0	55.23	4775	.655	
246	aB4"Na	8	55	0	53.88	4790	.687	
247	aB4"Nb	8	40	0	51.47	5565	.530	
248	aB4"Nc	10	50	0	48.03	4710	.580	
249	aB4"Nd	6	40	0	34.71	3400	.580	
250	aB4"Nac	11	40	0	49.99	4995	.570	
251	aB4"Nb	6	55	0	34.87	3325	.595	
252	aB4"Nd	9	40	0	51.72	5350	.550	
253	aA(s)3NGa	14	50	2	45.15	4130	.620	
254	aA(s)3NGb	15	50	1	30.22	3050	.555	
255	aA(s)3NGc	7	55	0	51.20	4705	.616	Pitch
256	aA(s)3NGd	8	40	0	27.14	2620	.588	
257	aA(s)'3NGa	4	40	0	32.19	3420	.536	
258	aA(s)'3NGb	12	40	5	39.80	4520	.503	
259	aA(s)'3NGd	14	45	0	38.27	3725	.585	
260	aA(s)"3NGa	6	40	0	22.99	2640	.499	

2.63		aA(S)"3NGb	9	45	0	29.12	2830	.585	
2.64		aA(S)"3NGc	18	50	0	47.19	4190	.638	
2.65		aA(S)"3NGd	10	45	0	41.23	4185	.564	
2.66		aAS2Na	7	50	0	27.99	2805	.567	
2.67		aAS2Nc	8	50	0	39.66	3770	.598	
2.68		aAS2Nb	5	40	0	34.31	3375	.590	
2.69		aAS2Nd	9	55	0	24.96	1880	.745	
2.70		aAS2'Na	16	40	2	37.29	3635	.582	
2.71		aAS2'Nb	8	40	40	42.73	4085	.595	
2.72		aAS2'Nc	8	35	0	36.43	4165	.520	
2.73		aAS2'Nd	17	40	25	37.91	3730	.580	
2.74		aAS2"Na	12	45	0	26.47	2330	.643	
2.75		aAS2"Nb	6	40	5	39.55	4215	.538	
2.76		aAS2"Nc	9	25	2	35.31	4245	.475	Small Pitch Pocket
2.77		aAS2"Nd	8	50	0	36.37	3495	.591	
2.79		aAlNa	9	50	0	36.10	2850	.715	Small Pitch Pocket
2.80		aAlNc	10	50	0	33.72	3470	.555	
2.81		aAlNd	7	45	0	37.59	3705	.578	
2.82		aAl'Na	11	40	0	28.27	3190	.510	
2.83		aAl'Nb	9	35	0	29.24	3370	.498	
2.84		aAl'Nc	11	50	0	20.08	1825	.625	
2.85		aAl'Nd	10	30	0	34.92	4250	.475	
2.86		aAl"Na	5	35	0	34.13	3745	.522	
2.87		aAl"Nb	12	40	10	37.85	4375	.488	
2.88		aAl"Nc	11	50	0	43.94	4265	.588	
2.89		aAl"Nd	6	35	0	41.94	4580	.575	
	Avg.							.571	
2.90	2"x10"	aB4Nac	12	40	50	17.27	1525	.640	Pitch
2.91		aB4'Nac	7	40	0	59.73	6270	.545	
2.92		aB4"Nac	10	50	0	69.17	5685	.685	Small Pitch
2.93		aBS5Nac	8	40	0	50.45	5755	.504	
2.94		aBS5'Nac	5	35	0	52.76	5145	.584	
2.95		aBS5"Nac	12	50	10	53.80	4585	.665	
2.96		aB(S)6NGac	9	50	0	50.13	5015	.570	
2.97		aB(S)6'NGac	8	40	0	56.79	5715	.566	
2.98		aB(S)6"NGac	5	50	0	17.18	1835	.536	
2.99		cAllNac	9	45	0	54.26	5450	.566	
3.00		cAllNbd	8	40	5	47.71	5180	.631	
3.01		cAll'Nac	8	40	0	45.03	5175	.500	
3.02		cAll'Nbd	6	30	0	41.30	5070	.468	
3.03		cAll"Nac	12	40	20	57.90	5690	.578	Small Knot
3.04		cAll"Nbd	6	40	0	61.88	6145	.575	
3.05		cAl2NGac	10	40	20	55.26	5530	.570	
3.06		cAl2NGbd	7	45	0	45.42	4335	.596	
3.07		cAl2'NGac	6	35	0	53.60	5910	.520	
3.08		cAl2'NGbd	8	45	20	59.53	6005	.565	
3.09		cAl2"NGac	11	45	10	34.74	3685	.545	
3.10		cAl2"NGbd	19	50	40	55.67	5750	.561	
3.11		cBl3Nbd	18	50	20	58.86	5510	.605	
3.12		cBl3'Nbd	6	40	0	46.22	5015	.528	
3.13		cBl3"Nbd	4	40	0	64.28	6575	.558	
3.14		cBl4NGbd	7	50	0	58.69	6065	.551	
3.15		cBl4'NGbd	6	45	2	54.96	6400	.492	
3.16		cBl4"NGbd	16	50	2	58.13	5440	.605	
3.17		dB16Nac	9	40	5	59.19	5915	.570	

318	dB16'Nac	11	50	5	64.19	5940	.611	
319	dB16'Nac	4	35	0	61.03	6110	.570	
320	fA21Nac	7	40	0	55.60	6115	.520	
321	fA21'Nac	5	35	0	53.83	5005	.611	Pitch
322	fA21Nbd	8	50	20	59.88	5590	.608	
							.570	
	Avg.						.581	



APPENDIX C

Minor Tests on Specimens  
from  
Random-Selected Dense Select Structural Southern Pine Planks

Test Number	Height in.	Width in.	I in. <sup>4</sup>	No. Rings per in.	Oven Dry Weight	Weight In Merc.	Specific Gravity G	Computed E <sup>#</sup> 1000psi	Def. for 1000lb.	Slope lb./in.	Test E' 1000psi	Corrected E' 1000psi <sup>*</sup>
1	1.54	1.54	0.4687	8	11.13	1,015	0.620	2,020	0.245	4,080	2,100	2,080
2	1.56	1.57	0.4967	8	12.82	1,220	0.595	1,940	0.310	3,230	1,570	1,560
3	1.55	1.54	0.4779	7	12.31	1,080	0.644	2,100	0.215	4,650	2,350	2,230
4	1.54	1.57	0.4778	6	11.18	1,030	0.613	2,000	0.261	3,830	1,930	1,920
5	1.53	1.56	0.4656	9	10.05	1,065	0.537	1,750	0.233	4,290	2,220	2,200
6	1.55	1.55	0.4610	8	12.30	1,240	0.564	1,840	0.215	3,650	1,910	1,890
7	1.53	1.56	0.4656	16	11.11	1,185	0.530	1,730	0.256	3,910	2,030	2,010
8	1.57	1.54	0.4866	9	11.18	1,190	0.535	1,740	0.256	3,910	1,940	1,920

# Computed E =  $\frac{1,760,000 G}{0.54}$

\*Corrected for Form Factor of 1.0084

APPENDIX D

Minor Tests on Modulus of Elasticity of Dense Select Structural Southern Pine  
For Determination of Validity of Use of Modulus of Elasticity  
On Basis of Specific-Gravity Data According to General F.P.L. Formula

Test Number	Computed E psi.	Test E' psi.	Variation of E from E' pct.	d psi.	d-d̄	(d-d̄) <sup>2</sup>
1	2,020,000	2,080,000	- 2.9	- 60,000	-210,000	44,100,000,000
2	1,940,000	1,560,000	+24.3	+380,000	+230,000	52,900,000,000
3	2,100,000	2,230,000	- 5.8	-130,000	-280,000	78,400,000,000
4	2,000,000	1,920,000	+ 4.2	+ 80,000	- 70,000	4,900,000,000
5	1,750,000	2,200,000	-20.0	-450,000	-600,000	360,000,000,000
6	1,840,000	1,890,000	- 2.6	- 50,000	-200,000	40,000,000,000
7	1,730,000	2,010,000	-13.9	-280,000	-430,000	184,900,000,000
8	1,740,000	1,920,000	- 9.4	-180,000	-330,000	108,900,000,000
						Σ = 874,100,000,000
Avg.	1,890,000 1,890,000	2,040,000 2,040,000	- 3.3			
				d̄ = -150,000		

Range of variability from -20.0 to +24.3 pct., with average variability of - 3.3 pct.  
Very small average variability for given range, of practical insignificance.

$$t = (2,040,000 - 1,890,000) / \sqrt{874,100,000,000 / (8 \times 7)} = 1.2$$

For 7 degrees of freedom: t = 3.5 at 1 pct. level.

Hence, statistically insignificant difference for test and computed data.

APPENDIX E

SYMBOLS

E Modulus of elasticity computed from specific gravity by means of Forest Products Laboratory Formulas.

E' Modulus of elasticity determined from flexural tests.

d Difference between E and E'.

$\bar{d}$  Difference between average E and average E'.

G Specific gravity.

I Moment of inertia.

Pct. Percent.

n Number of samples.

$\Sigma$  Sum.

$$t = \frac{\bar{d}}{\sqrt{\frac{(d-\bar{d})^2}{(n-1)n}}}$$

Psi. Pounds per square inch.