

The Role of Structural/Foundation Damping in Offshore Wind Turbine Dynamics

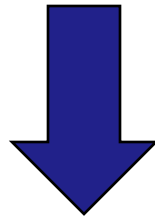
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Goal and Purpose

Determine how foundation damping affects structural demands over a variety of wind, wave, and operating conditions



- Foundation damping advantageously incorporated into design guidelines
- More efficient OWT design
- Reduction in large cost of support structure

Overview

- Motivation
- Tools, software, and models
- Conditions
- Parameter study methods
- Effects of damping on peak loads
- Fatigue damage methods
- Effects of damping on fatigue life
- Conclusions

Motivation

Wind energy moving offshore to allow **larger turbines** access to higher, more consistent wind speeds

Offshore development requires **expensive** support structure: 20-30% total cost (Musial)

Costs kept low by using **minimum materials/weight**

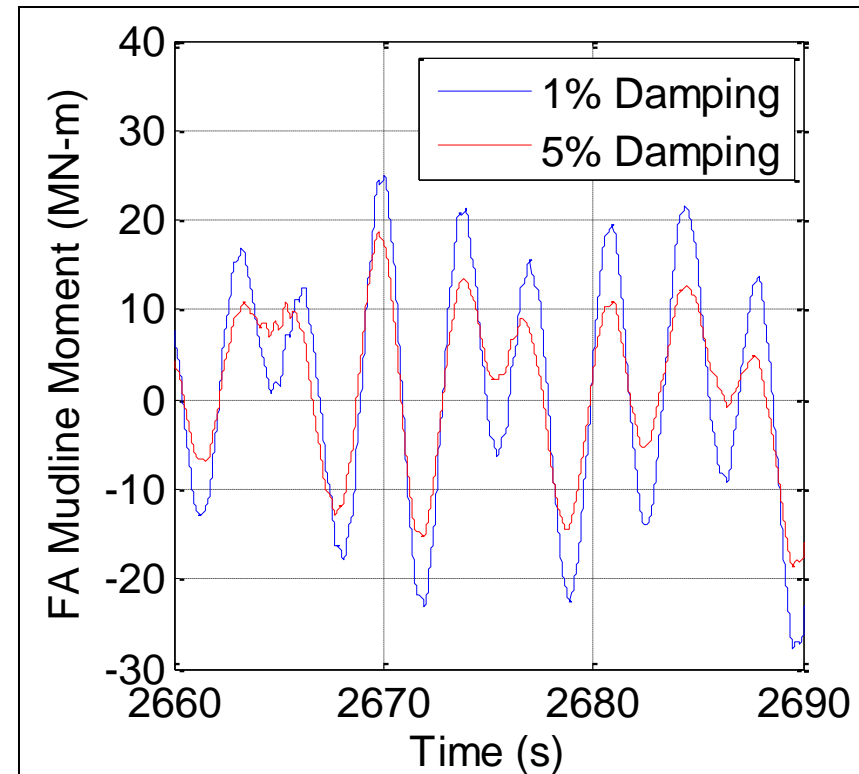
Results in slender & flexible structure with **resonant frequency close to excitation frequencies**

Turbine falls subject to **load amplification** and **cyclic fatigue**

Foundation Damping

Damping is crucial in counteracting load amplifications at or near resonant conditions

- Damping sources:
 - Aerodynamic
 - Hydrodynamic
 - Structural
 - Tuned mass
 - Soil (Foundation)
- Most damping sources determined accurately, but soil's complexity makes damping difficult to define
 - IEC standards do not account for soil damping, which can be 1.5% (Versteijlen, 2011)



Mudline moment time history

Effect of increased damping on load amplitude

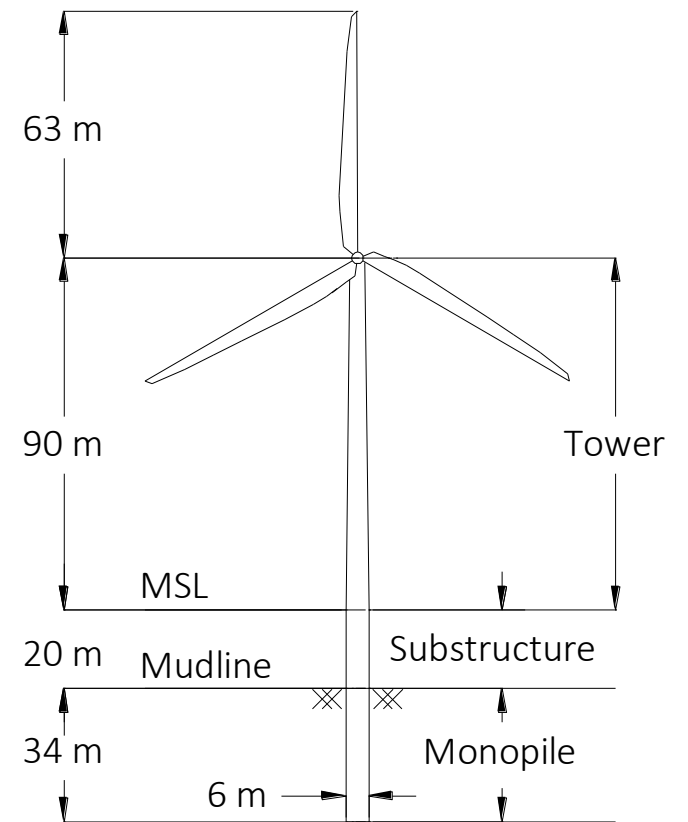
Tools, Software, and Models

- Theoretical OWT: **NREL 5MW Reference Turbine**
- Simulation Software: **FAST (NREL)**
 - Models both stochastic environmental loading and mechanical load effects
- Soil Damping Model:
 - Total system damping for 1st bending mode

$$\zeta_1 = \zeta_{\text{monopile}} + \zeta_{\text{tower}} + \zeta_{\text{aero}} + \zeta_{\text{hydro}} + \zeta_{\text{soil}}$$

- No soil damping input, ζ_{soil} , in FAST
 → **Changes in soil damping modeled through changes in structural damping input, ζ_{tower}**
- Structural damping in FAST modeled with simplified Rayleigh damping

NREL 5MW Reference Turbine Schematic



Carswell

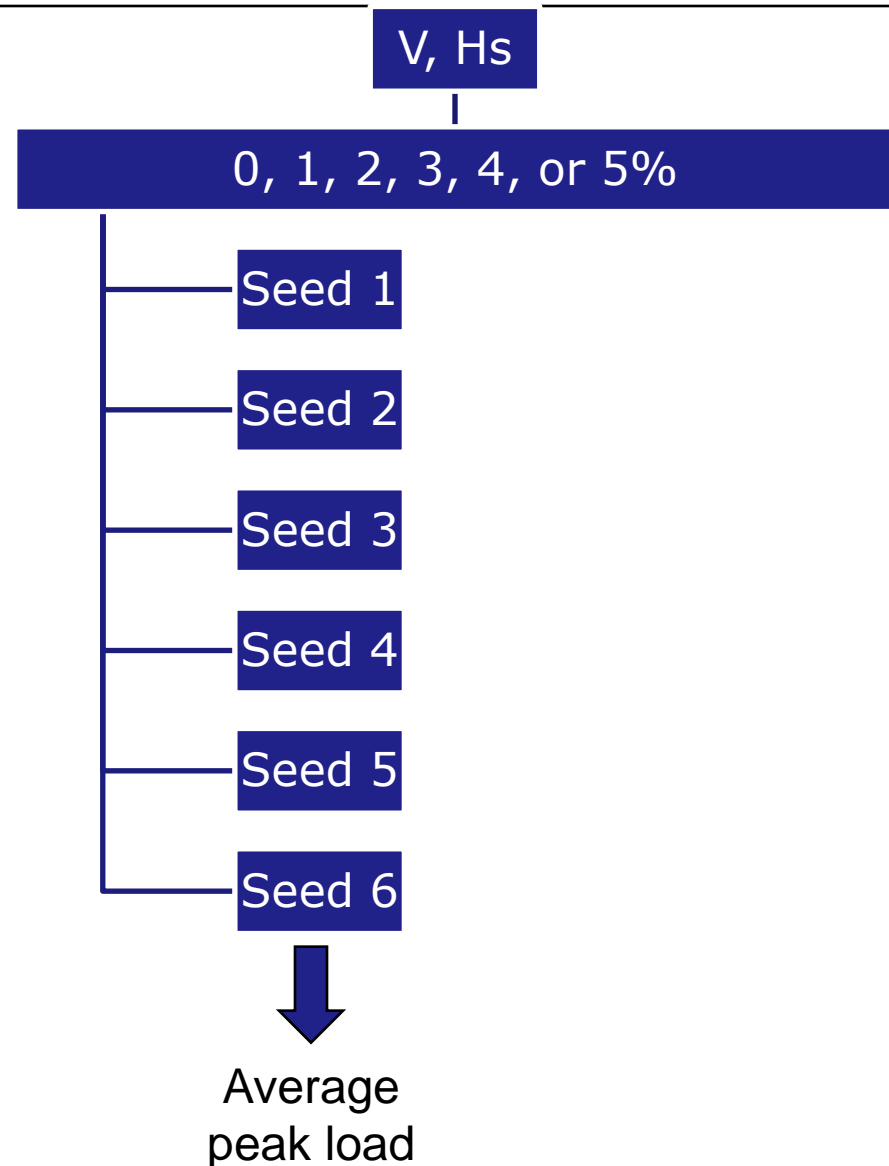
Conditions and Parameters

| Conditions | |
|----------------|--|
| Water Depth | 20 m |
| Platform Model | Fixed Bottom Monopile Offshore |
| Wind | Turbulent: TI = 11% IEC Kaimal Model |
| Waves | Irregular: JONSWAP/Pierson-Moskowitz spectrum |

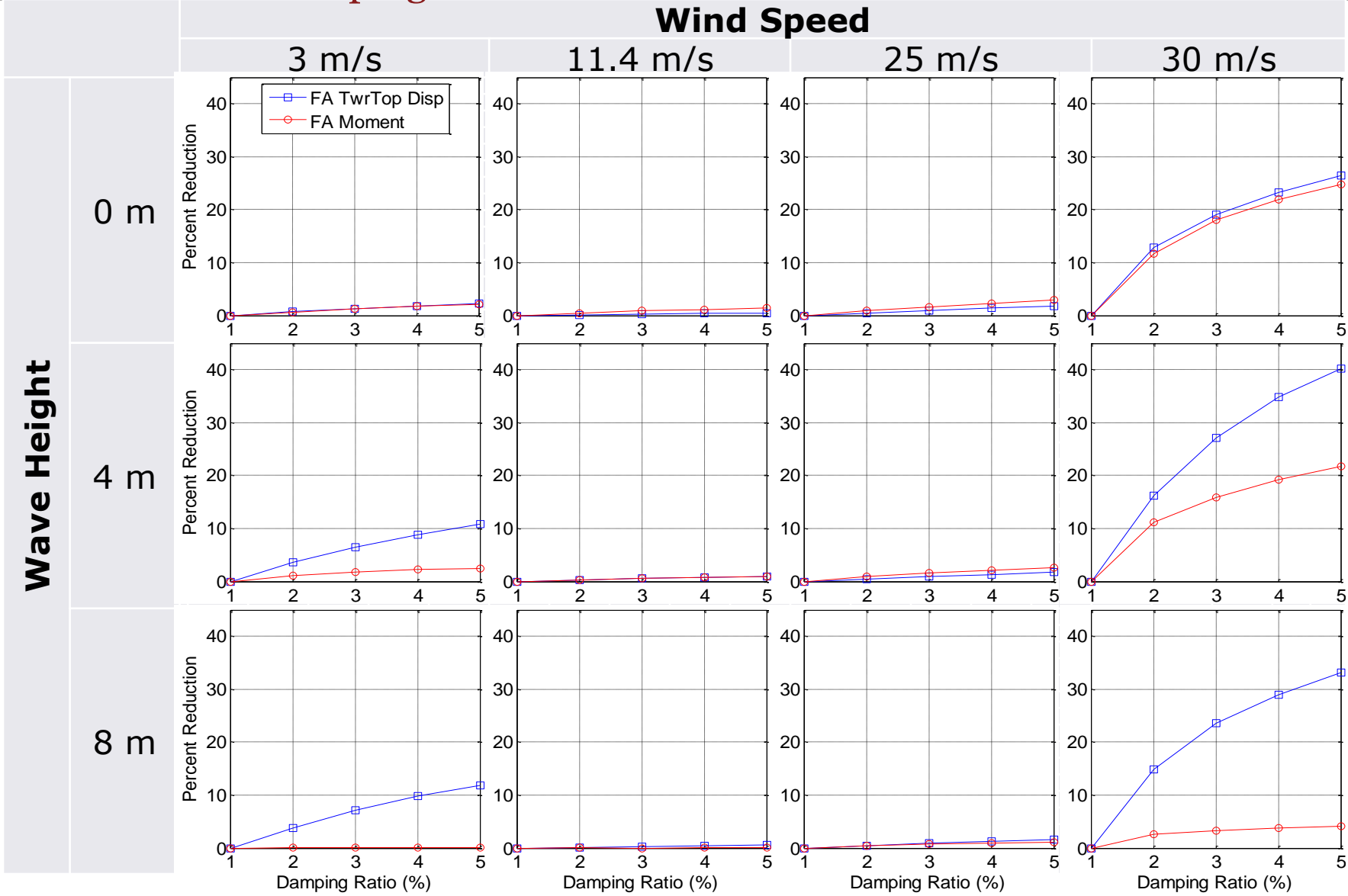
| Parameters | | |
|--------------------------|--------------------------|----------------------------|
| Damping Ratios | 0, 1, 2, 3, 4, 5% | |
| Significant Wave Heights | 0, 2, 4, 6, 8 m | |
| Wind Speeds | 3 m/s | $V_{\text{cut in}}$ |
| | 11.4 m/s | Rated |
| | 25 m/s | $V_{\text{cut out}}$ |
| | 30 m/s | Parked and Feathered (P&F) |

Methods

- For each distinct combination of wind speed and wave height:
 - 6 1-hr cases for each damping ratio 0-5%
 - Peak value from each differently seeded case averaged together



Percent Reduction of **FA Tower Top Displacement** & **FA Mudline Moment** from Value at 1% Damping Ratio



- **Operating cases**

- Increased damping has negligible effects on load reduction

- **Parked & Feathered cases**

- Increased damping has significant effects on load reduction

→Lack of aerodynamic damping from spinning rotor

Fatigue Damage Accumulation

Recommended Practice DNV-RP-C203 (Fatigue Design of Offshore Steel Structures)

- Palmgren-Miner linear cumulative damage

$$D = \sum_{i=1}^k \frac{n_i}{N_i} \leq \eta$$

D = accumulated fatigue damage

k = # of stress blocks (minimum 20)

n_i = # of stress cycles in stress block i

N_i = # of cycles to failure at stress range $\Delta\sigma$

η = usage factor (1/Design Fatigue Factor)

= 1/3 for turbine base connection

Fatigue Damage Accumulation: Step 1

Use moment to calculate bending stress

$$\sigma = \frac{My}{I}$$

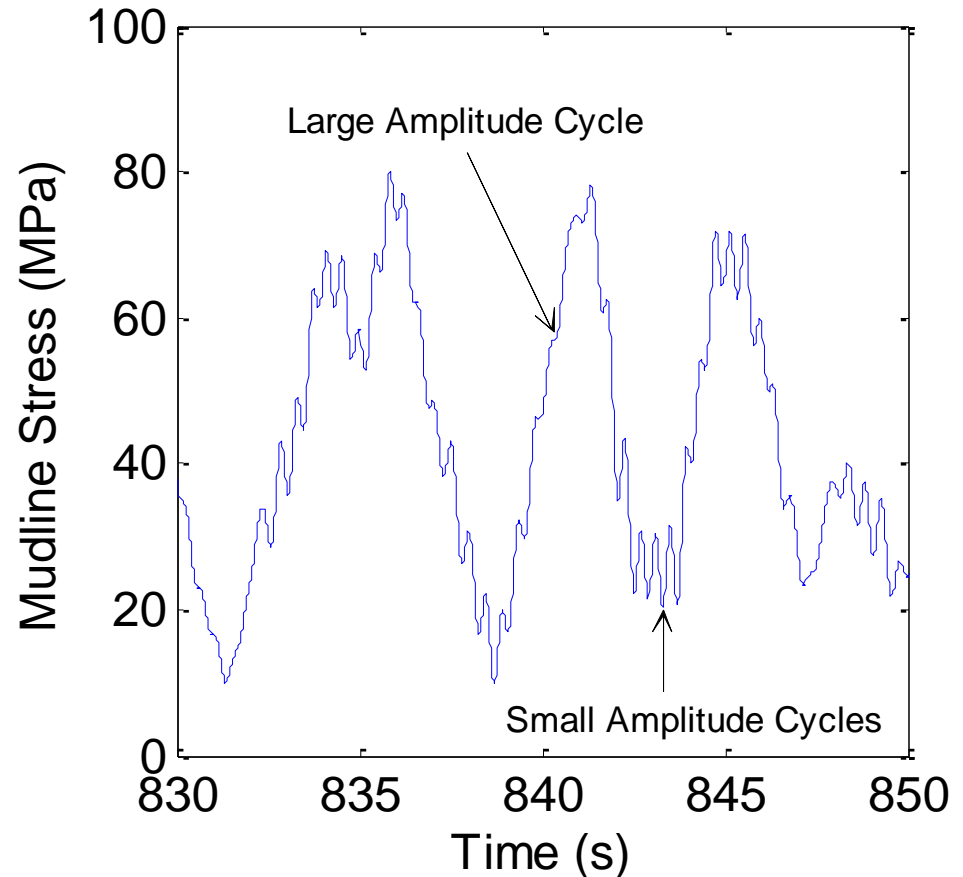
■ NREL 5MW Turbine

- FAST simulations → FA mudline moment, M
- Base diameter = 6 m → $y = 3$ m (maximum)
- Base thickness = 0.027 m → $I = 2.26$ m⁴

Fatigue Damage Accumulation: Step 2

Rainflow counting to interpret stress time history

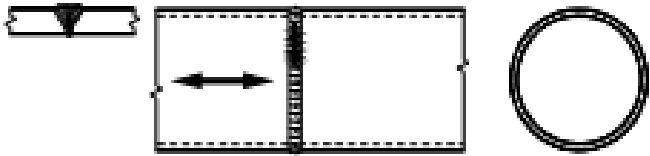
- Stochastic environmental loading → large variations in stress cycle amplitudes
- Rainflow counting digests stress time history to produce # of cycles, n_i , at different stress ranges, $\Delta\sigma$



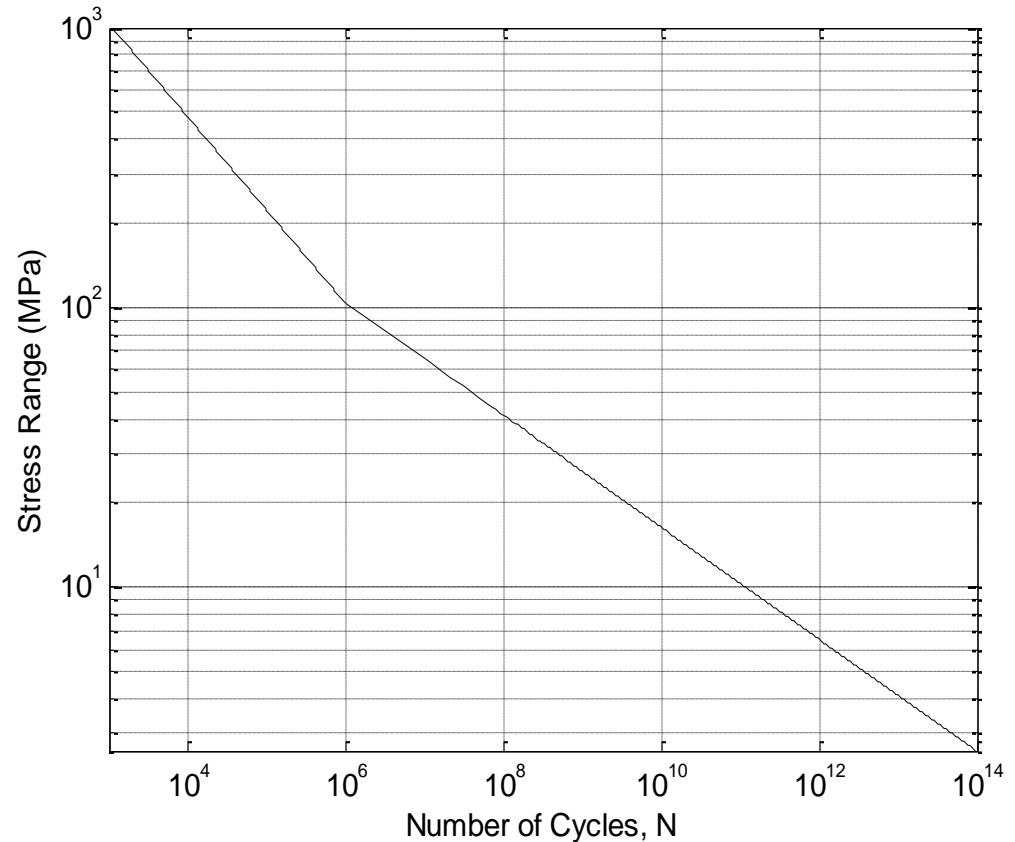
Fatigue Damage Accumulation: Step 3

Stress life curve to determine cycles to failure

- Curve C1 best modeled tubular steel pipe connecting the turbine to the foundation at the mudline



C1 in section A.9 (Hollow Sections): circumferential butt weld made from both sides dressed flush



C1 S-N curve for steel in seawater with cathodic protection (DNV 2005)

Fatigue Damage Accumulation Results

$$D = \sum_{i=1}^k \frac{n_i}{N_i} \leq \eta$$

Accumulated Fatigue Damage, D , for 1 hour (scaled)

| | | Wind Speed | | | | | | | | | | | |
|-------------|-----|------------|------|------|----------|------|------|--------|------|------|--------|------|------|
| | | 3 m/s | | | 11.4 m/s | | | 25 m/s | | | 30 m/s | | |
| | | 0% | 3% | 5% | 0% | 3% | 5% | 0% | 3% | 5% | 0% | 3% | 5% |
| Wave Height | 0 m | 1e-4 | 4e-5 | 3e-5 | 1.7 | 1.4 | 1.3 | 7.2 | 5.0 | 4.3 | 0.17 | 4e-3 | 2e-3 |
| | 2 m | 0.28 | 0.20 | 0.17 | 4.1 | 3.4 | 3.1 | 11.5 | 8.3 | 7.2 | 11.0 | 0.61 | 0.36 |
| | 4 m | 1.7 | 1.3 | 1.2 | 8.7 | 7.5 | 7.0 | 19.4 | 14.8 | 13.2 | 31.6 | 2.9 | 2.0 |
| | 6 m | 6.6 | 5.5 | 5.1 | 18.0 | 15.7 | 14.9 | 32.3 | 25.6 | 23.3 | 57.0 | 9.6 | 7.3 |
| | 8 m | 18.8 | 16.1 | 15.3 | 36.3 | 32.5 | 31.4 | 56.6 | 46.2 | 42.6 | 115.8 | 24.9 | 19.9 |

Least damage



Most damage

*Values for **comparison purposes only**

Conclusions

Increased damping in Operational Conditions

- **Small effect** on peak load reduction (<5% most cases) and fatigue damage reduction

Increased damping in Parked & Feathered Conditions

- **Significant peak load reduction and fatigue damage reduction** due to lack of aerodynamic damping
 - Up to 40% reduction in peak FA tower top displacement
 - Up to 27% reduction in peak FA mudline moment

Future Work

Use NREL Mlife software to:

- 1.) Calculate fatigue life and compare to Palmgren-Miner
- 2.) Evaluate effect of damping on both short-term and lifetime damage equivalent loads (DELs)

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References

- Damgaard, Mads, Jacob K F Andersen, Lars Bo Ibsen, and Lars V Andersen. 2012. "Natural Frequency and Damping Estimation of an Offshore Wind Turbine Structure" 4: 300–307.
- DNV (Det Norske Veritas). 2011. "Design of Offshore Steel Structures , General (Lrfd Method)," no. April.
- Jonkman, J, S Butterfield, W Musial, and G Scott. 2009. "Definition of a 5-MW Reference Wind Turbine for Offshore System Development Definition of a 5-MW Reference Wind Turbine for Offshore System Development," no. February.
- Veritas, Det Norske. 2013. "DNV-OS-J101 Design of Offshore Wind Turbine Structures," no. February.
- Veritas, Dn. 2005. "Fatigue Design of Offshore Steel Structures." *Recommended Practice DNV-RPC203*, no. April. <ftp://128.84.241.91/tmp/MSE-4020/Fatigue-Design-Offshore.pdf>.