

Integrating Laser Scanning with Discrete Element Modeling for Improving Safety in Underground Stone Mines

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ACADEMIC ABSTRACT

According to the Mine Health and Safety Administration (MSHA), between 2006 and 2016, the underground stone mining industry had the highest fatality rate in 4 out of 10 years, compared to any other type of mining in the United States. Additionally, the National Institute for Occupational Safety and Health (NIOSH) stated that structurally controlled instability is a predominant failure mechanism in underground limestone mines. This type of instability occurs when the different discontinuity sets intercept with each other forming rock blocks that displace inwards the tunnel as the excavation takes place, posing a great hazard for miners and overall mine planning. In recent years, Terrestrial laser scanning (TLS) has been used for mapping and characterizing fractures present in a rock mass. TLS is a technology that allows to generate a three-dimensional multimillion point cloud of a scanned area. In addition to this, the advances in computing power throughout the past years, have allowed numerical modeling codes to represent more realistically the behavior of a fractured rock masses. This work presents and implements a methodology that integrates laser scanning technology along with Discrete Element Modeling as tools for characterizing, preventing, and managing structurally controlled instability that may affect large-opening underground mines. The stability of an underground limestone mine that extracts a dipping ore body with a room and pillar (and eventual stoping) mining method is analyzed with this approach. While this methodology is proposed based on a specific case study that does not meet the requirements to be designed with current NIOSH published guidelines, this process proposes a general methodology that can be applied in any mine experiencing similar failure mechanisms, considering site-specific conditions. The aim of this study is to ensure the safety of mine workers and to reduce accidents that arise from ground control issues. The results obtained from this methodology allowed us to generate Probability Density Functions to estimate the probability of rock fall in the excavations. These models were also validated by comparing the numerical model results with those obtained from the laser scans.

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GENERAL AUDIENCE ABSTRACT

According to the Mine Health and Safety Administration (MSHA), between 2006 and 2016, the underground stone mining industry had the highest fatality rate in 4 out of 10 years, compared to any other type of mining in the United States. Additionally, the National Institute for Occupational Safety and Health (NIOSH) stated that structurally controlled instability is one of the main causes of rock falls in underground limestone mines. This type of instability occurs when the fractures present in the rock mass intercept each other forming rock blocks that displace into the tunnel as the excavation takes place and poses a great hazard for miners. In recent years, Terrestrial laser scanning (TLS) has been used for mapping and characterizing fractures present in a rock mass. TLS is a technology that allows to generate a three-dimensional multimillion point cloud of a scanned area. In addition to this, the advances in computing power throughout the past years, have allowed simulation softwares such as the Discrete Element Model (DEM) to represent more realistically the behavior of a fractured rock mass under excavation. The aim of this work was to develop and evaluate a methodology that could complement already existing design guidelines that may not apply to all kind of underground mines. The presented methodology evaluates rock failure due to presence of discontinuities, through the integration of TLS with DEM and considers site specific conditions. An area of a case study mine was assessed with this methodology, where several laser scans were performed. Information extracted from this laser scans was used to simulate the response of the rock mass under excavation by running Discrete Element Numerical Models. Results from these models allowed us to estimate the probability of rock failure in the analyzed areas. These, rock block failure probability estimations provide engineers a tool for characterizing, preventing, and managing structurally controlled instability, and ultimately improving workers safety.

DEDICATION

In memory of Bernardo Monsalve Castaño, my grandfather. He became a miner when he was thirteen years old and devoted his whole life to Colombian Mining Industry. He was mentor of many mining engineers that worked in different companies such as Peldar, Geominas S.A. and Frontino Gold Mines. All his stories and teachings remain in my memory.



This work is also dedicated to my parents Juan Eugenio Monsalve and Francia Elena Valencia, and my sister Maria Alejandra Monsalve. They have been a fundamental pillar in both my personal and professional development. For their unconditional love and constant support.

Professor Oscar Jaime Restrepo from Universidad Nacional de Colombia whom showed me early on my career how to open doors and windows to find a way to make my dreams and the ones of those around me come true.

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PREFACE

This thesis is composed of three main chapters, which describe progression of work to develop and test a methodology that combines Terrestrial Laser Scanning (TLS) and Discrete Element Modeling (DEM) for the design of underground stone mines aimed at ensuring safety of the workers and reducing accidents due to ground control issues. Even though, this work is based off of a case study mine, the methodology here described can be applied to any mine posing a similar failure mechanism.

Chapter 1 presents the case study mine and describes the failure mechanisms that pose risks for workers and the overall operation. This paper explains and compares current engineering methods to design excavations under structurally controlled failure mechanism. Finally, A methodology that integrates TLS and DEM is proposed. This methodology included the following stages: Preliminary evaluation and site selection, site scanning and rock mass characterization, structural data processing, sample collection, rock mechanics laboratory testing, numerical modeling and design considerations and monitoring.

Chapter 2 provides further details on how to plan and perform a laser scanning survey for rock mass characterization, virtual discontinuity mapping and Discrete Fracture Network (DFN) generation. In addition to this, nine laser scans were performed. The resulting point cloud was processed and virtual discontinuity mapping was carried out. A total of 874 fractures were mapped. These were later used in order to identify the main structural sets and generating DFNs for subsequent numerical modeling.

Chapter 3 focusses in the Discrete Element Modeling and proposes a stochastic approach to analyze block failure in the case study mine. These work describes the assumptions made to generate the model. Thirty iterations were run to generate probability density functions for total volume and number of failed blocks in a 20 m long tunnel, considering the four discontinuity sets defined in previous work and the geometries of the excavation. In addition, information about failed block such as geometries, sizes and kinematics were obtained. The results obtained from the models were validated by comparing blocks sizes and geometries with gaps left after rock fall in the laser scanned tunnels.

Finally, Chapter 4 summarizes the overall work and highlights the accomplishments of it. A series of conclusions derived from this work are presented, as well as a series of recommendations aimed at implementing this methodology in underground limestone industry for improving safety and reducing ground control related accidents.

Chapter 1 A Preliminary Investigation for Characterization and Modeling of Structurally Controlled Underground Limestone Mines by Integrating Laser Scanning with Discrete Element Modeling.¹

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Abstract.

Stability of large opening underground excavations in jointed rock masses primarily depends on the distribution and properties of the geological discontinuities. Conventional methods for structural mapping may not be optimal for rock mass characterization. Laser scanning is a technology that rapidly sends out laser pulses in order to measure the position of certain objects by generating a massive point cloud with millimeter precision. This paper reviews the application of laser scanning along with discrete element method (DEM) modeling, to produce a more realistic response of the rock mass behavior during excavation compared to analytical approaches. Additionally, a methodology to evaluate the stability in a structurally controlled underground limestone mine with these two technologies is proposed.

Key words. Limestone Mines, Terrestrial Laser Scanning, 3DEC, Structurally Controlled Instability.

1. INTRODUCTION

Over the past ten years, 40% of underground mining fatalities were caused by ground control issues related to roof, rib collapses, and pillar bursts (MSHA, 2016). Over the same time period, the underground stone mining industry had the highest fatality rate in four of those ten years (MSHA, 2016), more than any other sector. Underground limestone mines in the eastern U.S. have become more common over the past decade and typically there is less underground experience compared to underground mines in the western part of the country, resulting in a need for more engineering, education and training. In addition, the large openings and great spans between pillars can create instability conditions, leading to a large collapse of the roof or pillar if the mining system is not designed properly or if pillars deteriorate over time.

In 2011, the National Institute for Occupational Safety and Health (NIOSH) developed empirical guidelines for designing underground stone mines. This was limited to room and pillar mines in flat-lying bedded formations located in the eastern and Midwestern United States (NIOSH, 2011). However, these guidelines are not necessarily applicable to mines that do not share the same conditions as the mines in which the study was performed.

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A case study mine (CSM) in which NIOSH guidelines does not apply will be evaluated. The CSM is a dipping (approximately 30°), underground room and pillar limestone mine. This rock has UCS of 159.2 MPa ± 21.25 MPa, a tensile strength of 6.3 MPa ± 1.99 MPa and a Young's modulus of 64.11 GPa ± 2.37 MPa. The deepest point in the mine is approximately 700 m below ground surface. This mine leaves 24 m by 24 m pillars and the dimensions of the drifts and crosscuts are generally 12.8 m wide and 7.6 m high. The mine's main failure mechanism is structurally controlled failure, evidenced by the jointing pattern and spacing observed in the tunnels, the amount of fallen blocks observed on the floor, and other geological structures such as faults and contacts that may generate a rock fall in the absence of ground support. In this case, further analysis may be performed in order to ensure the stability of the excavation and safety during operation. Even though general guidelines may not apply for this case, analysis methodologies may be proposed in order to analyze mines that present similar modes of failure.

In recent years, the development of digital mapping techniques through Terrestrial Laser Scanning (TLS) and photogrammetry have allowed the development of three-dimensional digital terrain models as tools for characterizing, modeling, and designing underground structures, not only in tunneling projects but also in mining environments (Cacciari & Futai, 2017; Rogers, Bewick, Brzovic, & Gaudreau, 2017; Fekete & Diedrichs, 2013; Slaker, Westman, Fahrman, & Luxbacher, 2013; Grenon, Landry, Hadjigeorgiou, & Lajoie, 2017). In addition, it has been proven that this technology is useful, faster, and cheaper than conventional mapping methods, and also that with the right analyses and data interpretation the results may offer a better representation of the rock mass structure than conventional methods.

Not only have great advances been made in digital mapping techniques during the past decade, but the advances in computing power allow numerical models to more realistically represent the behavior of rock masses (Lorig & Varona, 2013). Due to these current advances, the use of distinct element codes such as 3DEC, is proposed over conventional analysis methods to evaluate the stability of the mine workings in order to better understand the rock mass behavior and ultimately improve safety performance in the operation.

The following paper reviews both technologies: Laser Scanning for rock mass characterization and discrete element model software for analyzing the instability of a structurally controlled limestone mine. Additionally, a methodology that integrates these technologies is proposed for analyzing the stability in the case study mine.

2. STRUCTURALLY CONTROLLED INSTABILITY

Structurally controlled instability has been widely described by many authors (Goodman, 1989; Brady & Brown, 1985; Hudson & Harrison, 2000; Hoek, 2000). This mode of failure has been defined as the sliding or natural falling of rock blocks either from the roof or the

walls due to gravity action (Brady & Brown, 1985). This type of instability occurs in rock masses that present two or more structural features that intersect, generating wedges; once the tunnel is excavated, these wedges generate blocks that tend to displace or rotate towards the opening. Additionally, researchers such as Martin, Kaiser, & Christiansson (2003) have proposed risk assessment charts in order to identify whether the conditions of the rock mass allow for either stress controlled instability or structurally controlled instability based on the stress condition, strength of the intact rock (σ_{ci}) and the Geological Strength Index (GSI); they state that when the rock mass has a GSI less than 30 and the σ_{ci} is greater than 2 times the vertical stress (σ_v), or when the GSI is greater than 40 and the principal stress (σ_1) is less than $0.15 \sigma_{ci}$, the rock mass is under structurally controlled instability.



Figure 1.1. Geotechnical conditions observed in the CSM.

Figure 1.1 presents the geotechnical condition observed in the CSM, where the structurally controlled instability was defined as main failure mechanism. This was evidenced by the jointing pattern and joint spacing observed in the tunnels, the amount of fallen blocks observed on the floor, and other geological structures such as faults and contacts that may generate a rock fall in the absence of the required support. This failure mechanism was enhanced by the multiple karst formations present in the mine, which during excavation have generated rock blocks up to 4 m^3 (144 ft^3) that pose high risk for workers, equipment, and the overall mining plan. Table 1.1 presents intact rock properties obtained from previous studies in CSM. During recent visits, GSIs greater than 75 have been identified throughout the mine. Considering the minimum strength conditions (137.95 MPa) and the greatest stress condition (20 MPa) the stress/strength relationship (σ_1 / σ_{ci}) is lower than 0.15, therefore, according to Martin, Kaiser, & Christiansson (2003) this mine is also defined as a structurally controlled mine.

Table 1.1. Intact rock property test results from CSM.

Lithology	Density (ton/m3)		UCS (MPa)		Brazilian Tensile Strength (MPa)		Young's Modulus (GPa)	
	Mean	SD	Mean	SD	Mean	SD	Mean	SD
Hanging Wall	2.69	0.01	163.74	37.84	11.96	3.14	61.02	6.79
Ore Body	2.69	0.01	159.20	21.25	6.30	1.99	64.11	2.37
Footwall	2.72	0.01	217.29	36.12	13.72	2.62	61.43	3.15

3. ANALYSIS METHODS FOR STRUCTURALLY CONTROLLED INSTABILITY

As described by Lorig & Varona (2013), two main analysis methods exist for the failure mechanism present in the CSM (structurally controlled instability); the limit equilibrium theory, also known as key block theory, described by Goodman & Shi (1985), Brady & Brown (1985) and Hudson & Harrison (2000); and the two and three dimensional discontinuous modeling methods, initially developed by Cundall (1971).

3.1. Limit Equilibrium Method

The limit equilibrium method is an analytical analysis method that consists of determining the geometry of the wedge formed by the intersection of 3 joint planes and the excavation using the block theory described by Goodman & Shi (1985). The forces acting on the wedge are then determined and its respective vectors calculated; The sliding direction is determined and so are the normal forces acting on each wedge plan; finally resisting forces due to joint shear strength and tensile strength are calculated, allowing for the calculation of a safety factor. These calculations may be performed by hand or using specialized software such as Unwedge (Rocscience, 2017).

Table 1.2 presents some capabilities and limitations of using a limit equilibrium software for stability analysis in jointed rock masses. Although, it is clear that this analysis method is not able to represent the real structural setting of the rock mass, it allows a general interpretation of the sizes, volumes and kinematics of the possible blocks to be performed.

Table 1.2. Advantages and Disadvantages of Using Limit Equilibrium Method for Analyzing Structurally Controlled Instability (Rocscience, 2017)

Limit Equilibrium Method (UNWEDGE)	
Capabilities	Limitations
<ul style="list-style-type: none"> - User friendly - Reproduce multiple models in short time. - Ability to consider support such as rock bolts and shotcrete. - Allows to perform statistical analysis based on the mechanical properties of the discontinuities. 	<ul style="list-style-type: none"> - can only use three discontinuity sets at a time for the analysis. - discontinuity surfaces are assumed to be persistent and to extend through the volume of interest. - complex geometries may not be analyzed.

	<ul style="list-style-type: none"> - discontinuities can occur at any location in the rock mass. - only considered one joint of the actual joint set.
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3.2. Two and three dimensional discontinuous methods

A discrete or distinct element method (DEM) according to Cundall & Hart (1985) refers to a computer program that allows finite displacements and rotations of discrete bodies, including complete detachment; and recognizes new contacts automatically as the calculation progresses.

3DEC is a numerical code that simulates the response of discontinuous media (such as a rock mass) subjected to either static or dynamic loading. In order to perform the analysis, it applies the discrete element model (DEM) proposed by Cundall (1971), which uses a time-stepping explicit algorithm to solve the equations of motion of the blocks. This software divides the rock mass into a mesh where each block is free to break or maintain its connection with the blocks that surround it. Additionally, it recognizes the new contacts between the displaced blocks as the calculation takes place. The input to this software consist on a series of parameters including physical and mechanical properties of both intact rock and discontinuities (ITASCA, 2016). Figure 1.2 presents an example of a 3DEC model in a road tunnel in Norway evidencing the effects of input parameters such as persistence on displaced blocks after the excavation, modified after (Fekete & Diedrichs, 2013).

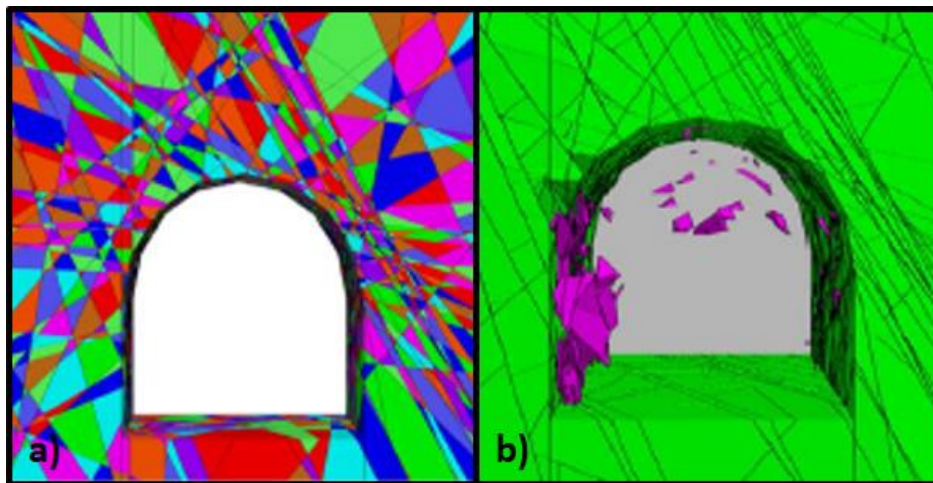


Figure 1.2. a) Construction of a deterministic model in a tunnel b) Deterministic model showing the block displacement in a tunnel, modified after (Fekete S. , 2010)

This numerical tool permits working with different constitutive models for both intact rock and discontinuities, generating models based on laboratory and field test data. Staged models can be performed using 3DEC due to its workflow methodology that allows running models stage by stage reproducing mining or excavation sequences. Complex geometries can be represented, thanks to its compatibility with CAD based software and other meshing plug-

ins such as Griddle to facilitate modeling. However, one of the most important characteristics of 3DEC is that it has its own programming language allowing the user to program their own functions, in order to create automated models.

3DEC is able to reproduce more reliable models that provide a better representation of the rock mass. This is possible due to its capability of elaborating discrete fracture networks (DFN) which simulate the structural setting in the rock mass. A DFN is a three-dimensional geometric representation of a geological structure based on statistical information of its characteristics measured on the field (Pierce, 2017). The parameters required to reproduce a DFN are joint orientation, density of the fractures (number of fractures per unit volume) and joint size generally quantified by the joint trace length (Grenon, Landry, Hadjigeorgiou, & Lajoie, 2017). All of these parameters can be collected by manual rock mass characterization; however, in order to generate reliable DFN's, great amounts of statistical data may be required, which may be difficult to obtain through manual mapping alone.

Some of the drawbacks of the 3DEC software are that when the model is very complex, it may take several iterations to process and significant time (days to weeks), compared to other modeling software it is expensive, and the reliability on the results strongly depends on the quality of the input data from the rock mass characterization.

Table 1.3 presents a summary of the capabilities and limitations described for discontinuous analysis methods such as 3DEC (ITASCA, 2016) for stability analysis in jointed rock masses. The application of this analysis tool will help to obtain a better understanding of the failure mechanism present in the CSM under present geotechnical conditions and propose alternative engineering designs, support measures and excavation methods to reduce structurally controlled instability and enhance safety conditions in the operation.

Table 1.3. Advantages and Disadvantages of Using Discontinuous Analysis Method for Analyzing Structurally Controlled Instability (ITASCA, 2016)

Discontinuous Analysis Methods (UDEC & 3DEC)	
Advantages (Capabilities)	Disadvantages (Limitations)
<ul style="list-style-type: none"> - Works with different constitutive models, both for discontinuities and intact rocks. - Ability to input statistical parameters of discontinuities properties. - Can represent complex geometries and sequential excavations. - Ability to consider multiple discontinuity sets by generating Discrete Fracture Networks (DFN). 	<ul style="list-style-type: none"> - Complex models are time consuming. - Representation of the behavior of the rock mass depends on the reliability on the geotechnical data.

4. ROCK MASS CHARACTERIZATION

In order to have sufficient information to perform any kind of analysis in a structurally controlled rock mass, a detailed survey of the geological structure must be done. Figure 1.3 presents a schematic representation modified after (Hudson & Harrison, 2000) of the properties to be measured in discontinuities present in a rock mass during a detailed rock mass characterization, according to (ISRM, 1978). These properties include spacing and frequency, orientation, persistence, roughness, aperture, seepage, filling, and wall strength. These properties are used in order to evaluate both geometrical and strength parameters of each discontinuity set along the rock mass.

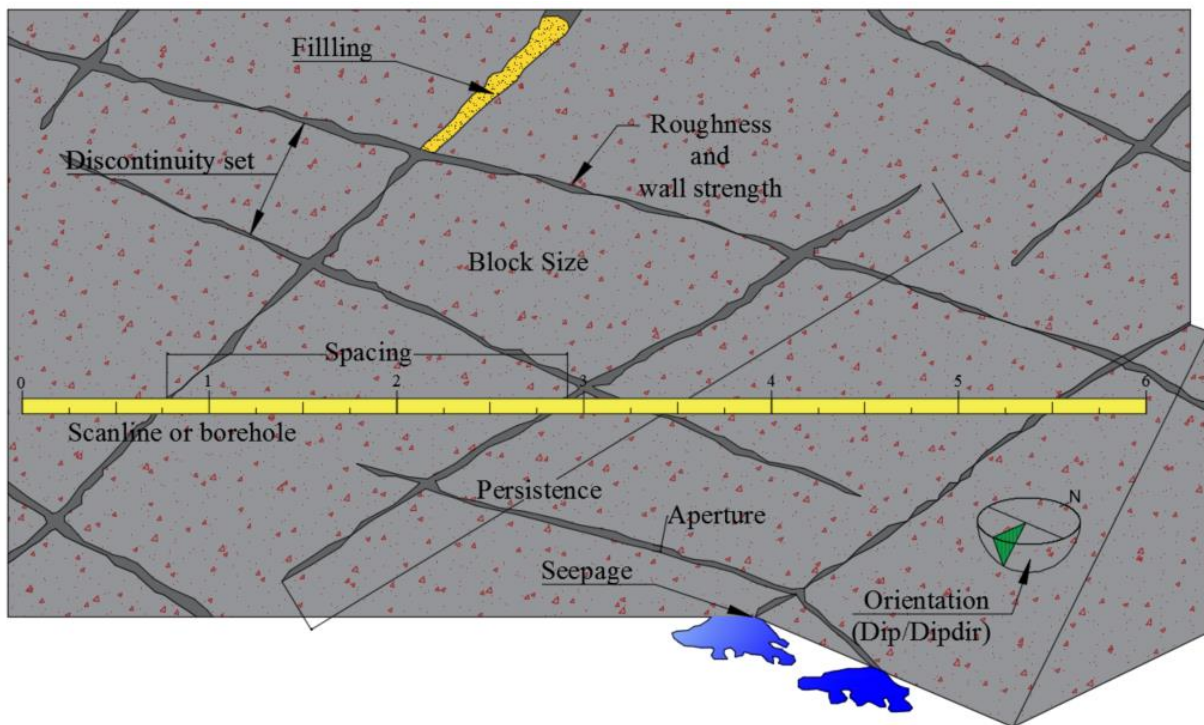


Figure 1.3. Schematic of the primary geometrical properties of discontinuities in rock, modified after (Hudson & Harrison, 2000)

According to Voyay, Roncella, Forlani, & Ferrero (2006), in many cases a traditional survey performed manually may be dangerous or difficult, provide a small sample size, and take a considerable amount of time. Alternatively, digital mapping techniques such as laser scanning and photogrammetry have been proposed by different authors. Although these technologies were initially proposed for slope stability analysis, today their applications have been extended to underground environments. In this study, laser scanning was selected over photogrammetry due to its time savings while acquiring and processing the data. Photogrammetry requires illumination in order to obtain the images, which may present a logistic complication during field work. It is worth remarking that photogrammetry can also offer an alternative to laser scanning and present great results as shown by Grenon, Landry,

Hadjigeorgiou, & Lajoie (2017) and Rogers, Bewick, Brzovic, & Gaudreau (2017) who have used photogrammetry to generate DFN to analyze wedge stability in mining drifts.

4.1. Laser Scanning

Laser scanning is a technology that rapidly sends out laser pulses in order to calculate the position of certain objects by generating a massive point cloud with millimeter precision (Kemeny, Turner, & Norton, 2006). Laser scans provide a three dimensional detailed image of the rock mass that allows one to quickly map, with more precision and less bias, the structural features present in the rock mass. Additionally, it has also been proven that laser scanning obtains qualitatively sound data in a short time without affecting the mining plan in an active tunnel environment; it provides detailed quality control information on the precision excavation and installed support (Fekete & Diedrichs, 2013). Other authors such as Slaker, Westman, Fahrman, & Luxbacher (2013) have used laser scanning for deformation measurements in pillars in underground mines.

Even though laser scanning has proved to be a useful tool for analyzing instability in structurally controlled rock masses, its greatest challenges are related to structural data analysis. As mentioned before the main properties of interest for each joint set are joint orientation, density of the fractures and joint size. Many authors have been centering their research in identifying the best way to analyze the structural data in order to generate more reliable numerical models. Fekete & Diedrichs (2013) proposed a workflow for integrating structural data obtained from TLS into DEM analysis. They discuss some of the advantages and challenges of TLS data in underground rock mass evaluation considering some of the bias that can be present during the extraction of discontinuity properties. Additionally, they proposed two approaches (deterministic and stochastic) for modeling the excavation using TLS data, allowing the designer to have a widened view of the rock mass performance. Cacciari, Morikawa, & Futai (2015) generated a 3D block model using 3DEC based on a DFN obtained from discontinuity mapping analysis from laser scans. The results from the stability analysis were not presented. Cacciari & Futai (2015) analyzed different methods for estimating the mean trace length of the discontinuities from laser scans; they defined that the trace length estimation method proposed by Wu, Kulatilake, & Tang (2011) presented better results than other methodologies proposed by other authors. Cacciari & Futai (2017) proposed a practical approach to create single and continuous DFNs considering the variations of the fracture density along the tunnel. Additionally, they performed numerical models based on the methodology obtaining similar behaviors in the model than those presented in the field.

5. PROPOSED METHODOLOGY

Figure 1.4 presents the workflow proposed in order to integrate laser scanning with numerical modeling in the CSM. This methodology is divided into seven phases: Preliminary evaluation and site selection, site scanning and rock mass characterization, structural data processing,

sample collection, rock mechanics laboratory testing, numerical modeling and design considerations and monitoring. The stages are explained in detail below.

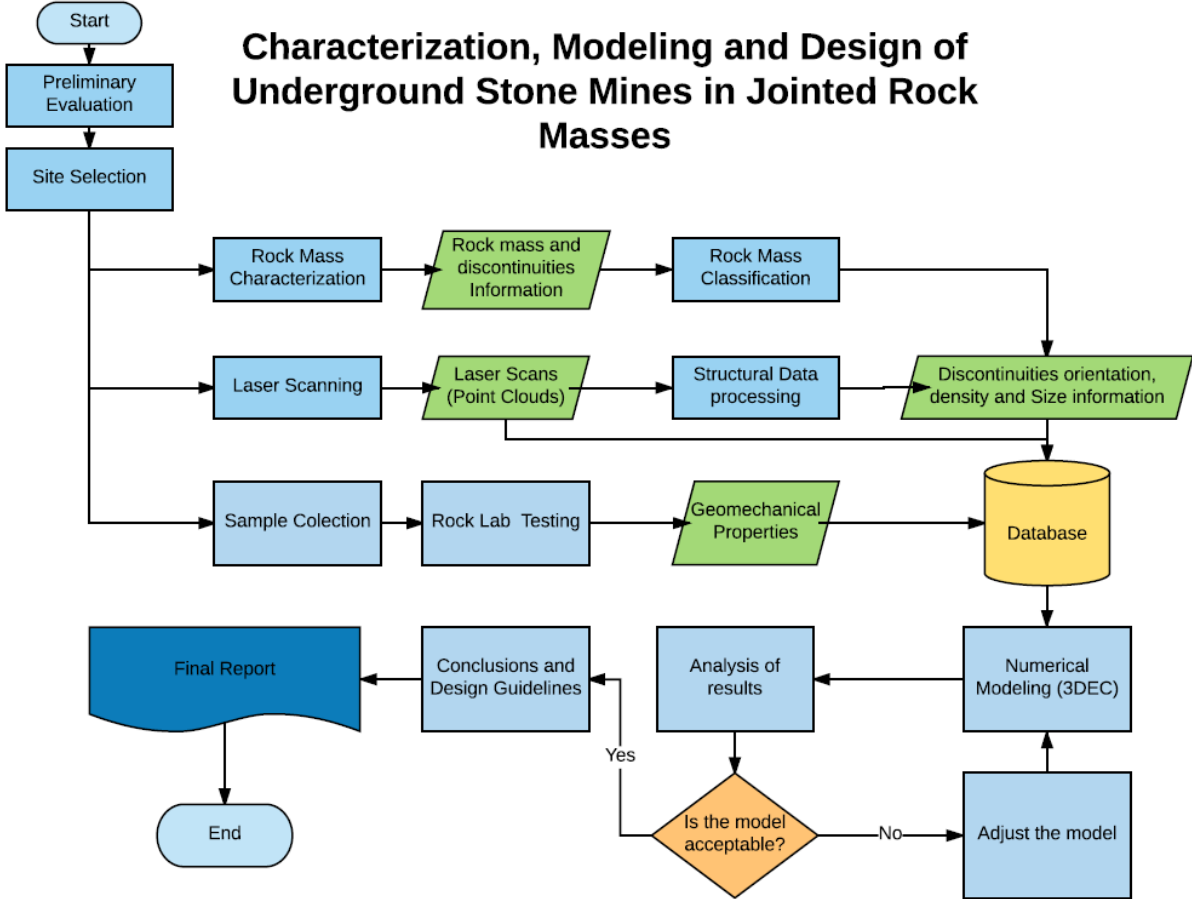


Figure 1.4. Proposed workflow for the integration of laser scanning and 3DEC modeling in the CSM

5.1. Preliminary evaluation and site selection

Several visits were made to the mine. Initial visits were carried out in order to get familiar with the mining method and geotechnical conditions of the mine. The main areas of concern for this study are working areas and travel ways where miners are at risk of roof falls. The areas in which the research will be performed will not only be those areas presenting greater geotechnical risks for the miners, but also those that present great interest for mine planning. These areas will be selected with mine management.

5.2. Site scanning and rock mass characterization

Once the locations for study are defined, the laser scans will be performed. In order to define what are the most appropriate operational conditions, a series of laser scans will be taken. Table 1.4 presents different operational conditions used by different authors, which will be

used as a reference to set the scanning conditions. A Faro Focus^{3D} laser scanner will be used for taking the scans, considering safety precautions defined by the manufacturer.

Table 1.4. Laser scan conditions used by different authors.

Research	Equipment	Measurement Speed (points/s)	Distance Accuracy (mm)	Spacing between scan Stations (m)
(Fekete & Diedrichs, 2013)	Leica Geosystems HDS6000	500,000	0.6	1 Diameter
(Cacciari & Futai, 2015)	Faro Focus ^{3D}	976,000	2	10
(Slaker, Westman, Fahrman, & Luxbacher, 2013)	Faro Focus ^{3D}	976,000	2	10

Additionally, conventional rock mass characterization will be performed in order to have control information that allows a comparison of results obtained from the scans and have a better understanding on the rock mass structure. This information also will be used in order to classify the rock mass with the conventional rock mass classifications such as Q, MRMR, GSI and RMi.

5.3. Structural data processing

In order to process the scans and extract discontinuities' properties the software I-Site Studio provided by MAPTEK will be used. I-Site is a point cloud processing and modeling software that extracts geotechnical information from laser scans by using a set of geotechnical tools (MAPTEK, 2017). This software will be used in order to identify the orientation, density and size of the discontinuities. Figure 1.5 present a preliminary structural analysis with a scan obtained from CSM.

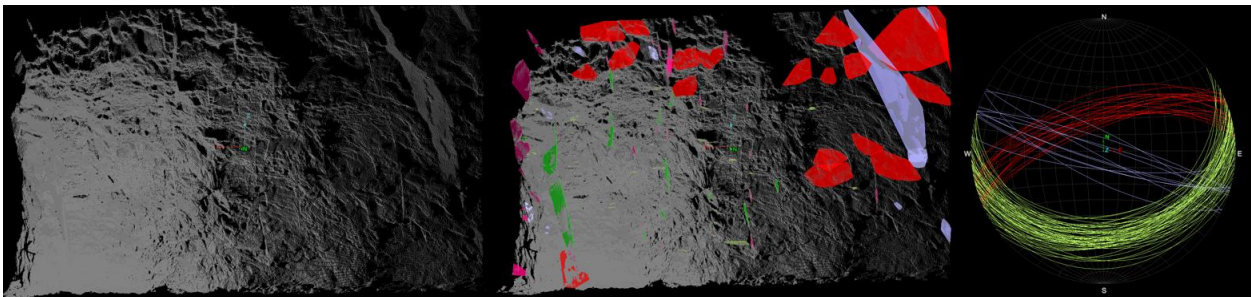


Figure 1.5. Extraction of joint sets from a laser scans using the software I-SITE.

5.4. Sample collection

Sample collection will take place in the areas previously selected. Due to the fact that the ore body lies between the two different types of limestone on both Hanging Wall and Footwall, samples from these formations will also be considered. The samples will be rock blocks which will be transported to the rock mechanics preparation lab, where NQ diameter cores will be extracted. The goal will be to obtain at least 3 samples for Triaxial test, 3 samples for direct shear, 3 samples for tensile strength, 3 samples for uniaxial compressive strength and

4 samples for point load, for each type of rock at each location. The results from this sampling stage will be the maps showing the different locations where the samples were extracted.

5.5. Rock mechanics laboratory testing

Among the properties to be considered for the blocks include mass density, intrinsic deformability properties (bulk & shear modulus or Young modulus or Poisson's ratio), intrinsic strength properties (Constitutive models and strength parameters such as compressive strength and tensile strength) and post-failure properties. On the other hand, joint properties including physical properties for joint friction angle, cohesion, dilation angle, and tensile strength, as well as joint normal and shear stiffness. The joint cohesion and friction angle correspond to the parameters in the Coulomb strength criterion (ITASCA, 2016). Generally, these parameters are obtained from rock laboratory tests. The laboratory tests proposed are presented in Table 1.5. This table also shows the intact rock properties and the discontinuity properties obtained from each test. These properties are the parameters that will feed numerical models.

Table 1.5. Rock lab testing standards defined by the ASTM description and properties obtained.

ASTM Standard	Description	Properties obtained
D2664-04	Standard test methods for Triaxial compressive strength of undrained rock core specimens without pore pressure measurements	-Hoek & Brown failure criteria -Young's modulus at different confinement pressures - Basic friction angle for artificial joints
D7012-14	Standard test methods for compressive strength and elastic moduli of intact rock core specimens under varying states of stress and temperatures	- Uniaxial compressive Strength - Young modulus - Poisson's ratio
D5607-08	Standard test method for performing laboratory direct shear strength tests of rock specimens under constant normal force	- Mohr-Coulomb failure criteria for discontinuities (ϕ , c) - Shear stiffness
D5731-08	Standard test method for determination of the point load strength index of rock and application to rock strength classifications	- Uniaxial compressive Strength (Indirect)
D3967-16	Standard test method for splitting tensile strength of intact rock core specimens	- Tensile strength of the rock (indirect)
D2845-08	Standard test method for laboratory determination of pulse velocities and ultrasonic elastic constants of rock	- Young's modulus (indirect) - density - Poisson's ratio
D57873-14	Standard test method for determination of rock hardness by rebound hammer method	- Uniaxial compressive Strength (Indirect)

The expected results from the laboratory plan will be descriptive statistics comparing the results for all the geomechanical parameters (UCS, tensile strength, E, & Poisson's) comparing the properties between different types of rock. Additionally, the spatial variability of this properties will be analyzed. Summary maps will be elaborated in order to present the variability of the geomechanical properties of the mine.

5.6. Numerical modeling and design Considerations

A modeling plan has been developed in order to define a route that allows to generate models that provide a better understanding on the usage of 3DEC, but also to obtain models that represent the actual conditions of the CSM. The models include a continuous single pillar, a single pillar with a single oblique discontinuity, a single tunnel considering a discrete fracture network, a single pillar considering a discrete fracture network, the mining sequence of the CSM without considering any discontinuities and the mining sequence of the same mine considering the geotechnical conditions represented using a DFN. The numerical models predict and anticipate the potential wedges before mining into a new area. The aim of these models will be to set guidelines and methodologies that can apply to any underground project and help them to obtain particular answers to each specific problem.

5.7. Monitoring

After numerical modeling results are obtained, a site evaluation will be performed in order to verify these results. Once the simulations coincide with the actual behavior observed in the rock mass, design considerations and engineering measures to control the instability mode will be proposed.

6. CONCLUSIONS

Different authors have proven that TLS technologies are a powerful suite of tools for rock mass characterization. Not only are they time efficient, but they also provide detailed information, which processed adequately considering actual limitations in both processing and modeling software, allows to generate models that represent accurately jointed rock mass behavior. DEM software is a powerful tool to interpret and analyze structurally controlled instability in underground excavations. This software with detailed structural information may provide not only deterministic but also stochastic analysis allowing the engineer to have a better understanding of rock mass behavior and provide better engineering solutions. The integration of both technologies (TLS and DEM) may be potentially used in order to prevent rock falls in underground excavations to enhance worker's safety. The CSM is an ideal environment to apply both technologies, and to develop risk analysis methodologies based on these technological advances, ultimately reducing accidents related to rock falls in the underground stone mine industry. The proposed methodology will be applied in the CSM and results will be presented in further work.

7. ACKNOWLEDGEMENTS

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Chapter 2 Application of Laser Scanning for Rock Mass Characterization and Discrete Fracture Network Generation in an Underground Limestone Mine²

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Abstract.

Terrestrial laser scanning (TLS) is a useful technology for rock mass characterization. A laser scanner produces a massive point cloud of a scanned area, such as an exposed rock surface in an underground tunnel, with millimeter precision. The density of the point cloud depends on several parameters from both the TLS operational conditions and the specifications of the project, such as the resolution and the quality of the laser scan, the section of the tunnel, the distance between scanning stations, and the purpose of the scans. One purpose of the scan can be to characterize the rock mass and statistically analyze the discontinuities that compose it for further discontinuous modeling. In these instances, additional data processing and a detailed analysis should be performed on the point cloud to extract the parameters to define a discrete fracture network (DFN) for each discontinuity set. I-Site Studio is a point cloud processing software that allows users to edit and process laser scans. This software contains a set of geotechnical analysis tools that assist engineers during the structural mapping process, allowing for greater and more representative data regarding the structural information of the rock mass, which may be used for generating DFNs. This paper presents the procedures used during a laser scan for characterizing discontinuities in an underground limestone mine and the results of the scan as applied to the generation of DFNs for further discontinuous modeling.

Keywords: rock mass characterization, laser scanning, discrete fracture network, I-Site Studio

1. INTRODUCTION

According to the Mine Health and Safety Administration (MSHA), between 2006 and 2016, the underground stone mining industry had the highest fatality rate in 4 out of 10 years, compared to any other kind of mining (MSHA, 2016). Additionally, during that same time, 40% of the fatalities were due to ground control issues, such as roof and rib collapses and pillar bursts. The National Institute for Occupational Safety and Health (NIOSH) developed

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guidelines for designing underground stone mines (Esterhuizen, Dolinar, Ellenberger, and Prosser, 2011). However, these guidelines do not apply to all underground stone mines.

Monsalve, Baggett, Bishop, and Ripepi (2018) present a case study of an underground limestone mine experiencing a structurally controlled mode of failure. The authors analyze different methods to study that type of instability. They conclude that the integration of terrestrial laser scanning (TLS) with discrete element modeling (DEM) can be used to prevent rock falls in underground excavations to enhance worker safety. However, an adequate rock mass characterization and structural mapping must be performed in order to generate reliable models that allow the engineer to have a better understanding of the rock mass behavior.

The final product of a laser scan is a point cloud that represents the surfaces of the objects scanned, in this case the rock surface in an underground environment. The density of the point cloud depends on a series of parameters from both the laser scanner operational conditions and the specifications of the project, such as the resolution and the quality of the laser scan, the section of the tunnel, the distance between scanning stations, and the purpose of the scans. The lead engineer defines these parameters. One purpose of the scan can be to characterize the rock mass and statistically analyze the discontinuities that compose it for further discontinuous modeling. In these instances, additional data processing and a detailed analysis should be performed on the point cloud to extract the parameters that define a discrete fracture network (DFN) for each discontinuity set. A DFN is a geometric representation in three dimensions of a geological structure defined by statistical information of the structure characteristics measured in the field, such as orientation, density, and size (Pierce, 2017).

This paper describes the methodology used in a case study mine to perform a structural mapping on a rock mass in order to obtain the input data needed to create a DFN for DEM. Different software packages were used to perform these analyses:

- 1) SCENE was used to import the laser scans and generate a project point cloud (Faro, 2018).
- 2) I-Site Studio was used to perform the structural mapping and to estimate the trace length and spacing of the different fractures (Maptek, 2018).
- 3) Dips was used to characterize and group the discontinuity sets (Rocscience, 2018).
- 4) 3DEC™ was used to generate the DFN (Itasca, 2018).

2. GEOTECHNICAL CONDITIONS

A specific area was defined in the case study mine to focus the present research. The mine research team and personnel selected this area because of the presence of significant ground control issues, mainly defined as structurally controlled instability failure. This was evidenced by:

- 1) jointing pattern where at least four joint sets were well defined,

- 2) observed wide-joint spacing, generally ranging from 0.6 m to 2 m,
- 3) amount of fallen blocks observed on the floor with cubical and tabular shapes,
- 4) joint surfaces defined as mostly closed, flat and smooth, with a JRC ranging from 2 to 4, completely dry and fresh,
- 5) other geological structures, such as faults and contacts, that could generate a rock fall in the absence of the required support

This failure mechanism is enhanced by the multiple karst formations present in the mine, which, during excavation, tends to generate rock blocks up to 4 m³ (144 ft³) that pose a high risk for workers, equipment, and the overall mining plan (Monsalve, Baggett, Bishop, and Ripepi, 2018).

Figure 2.1 shows a plan view of the area of interest. This illustrates a karst structure that crosses both tunnels and has a variable aperture reaching up to 0.5 m. Figure 2.1.A) is a view from the tunnel to the karst void, while Figure 2.1.B) presents a pile of material that fell down from the karst structure. Water and mud have also fallen out of the karst void. For these reasons, laser scans were carried out close to this area in order to map the structural conditions and to perform further numerical modeling to understand and prevent rock falls in this section of the mine.

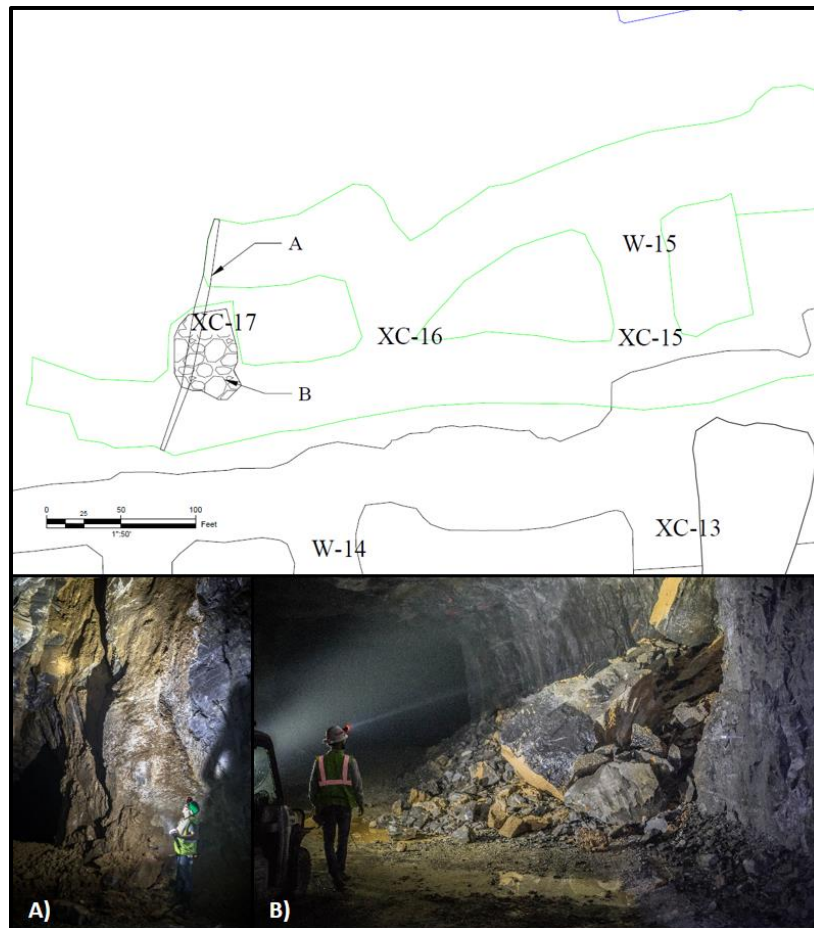


Figure 2.1. Geotechnical conditions observed in the study area. A) View from the tunnel towards the karst void on the upper half of the drift. B) Pile of fallen material from the karst void on the lower drift.

3. METHODOLOGY

The following methodology describes in detail the procedures used by the authors to perform the laser scans in the study area, to import and process the information obtained from those scans, and to create the respective DFNs resulting from the virtual structural mapping. This methodology was designed based on lessons learned from experiences presented in previous works (Fekete, 2010a; Fekete 2010b; Slaker, 2015) and practices using the laser scan and different software packages for these procedures. Additionally, documents, such as the Laser Scan User Manual (FARO, 2011) and user guides on planning and performing the laser scans provided by the laser scan provider company (DiCarlo, 2013), were used to complement this methodology.

3.1. Definition of operational conditions

A laser scanner is a surveying apparatus that produces a massive point cloud (millions of points per scan), indicating the positions of the scanned objects. This equipment sends an infrared laser beam into the center of a rotating mirror, which deflects the laser beam onto a vertical rotation into the environment being scanned (Figure 2.2.A). Scattered light from surrounding objects is then reflected back into the scanner. The equipment is able to identify two waves: the one sent out by the equipment and the one reflected by the object. The phase shift between both waves is used to calculate the distance of the measured object (FARO, 2011). Then, by using angle encoders to measure the mirror rotation and horizontal rotation of the scanner, the x, y, z coordinates of each point are calculated, resulting in a point cloud that is stored in a removable SD memory card (FARO, 2011). Figure 2.2.B) presents the vertical and horizontal rotation of the laser scan. The laser scan unit used during this study was a FARO® Focus3D, acquired by the Mining and Minerals Engineering Department of Virginia Tech in 2011.

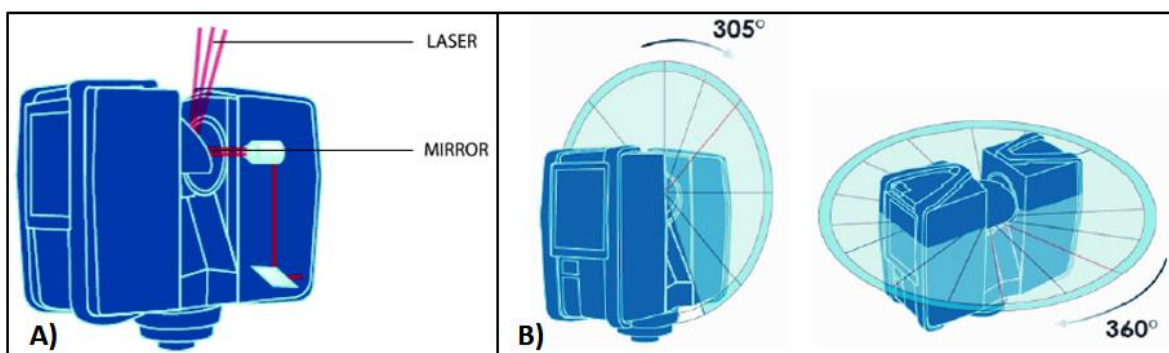


Figure 2.2. A) Laser deflection and B) Vertical and horizontal laser rotation (FARO, 2011).

The two main variables to be defined during the scan are the resolution and the quality. These two variables affect the time of the scan and the density of the point cloud (DiCarlo, 2013). The size of the excavation to be scanned, the distance of the scanner from the tunnel face, and the spacing between stations also affect the results. According to Fekete, the optimal

positioning of a laser scan from the face is between 0.5 to 1 times the diameter of the excavation (2010b). In this particular case, the diameter of the excavation is 13 m, and 1 times the diameter was used as the distance between the scanner and the face.

Once the distance from the face to the scanner was defined, the adequate resolution and quality were selected. Since these parameters vary from site to site, a set of scans were performed in order to define which combination of resolution and quality values provided, in a reasonable scan time, a scan with an acceptable point cloud density to perform an adequate structural mapping of the rock mass.

Table 2.1 shows the different operational conditions tested in the mine, the real time that it took to perform each scan, and the point cloud size and the point cloud density. Twelve scans were performed, processed, and converted into point clouds, which were imported to I-Site Studio for analysis. The total point count for each cloud was measured. To measure the average point cloud density, three 5 m x 5 m mapping windows were generated, one on the right wall, another one on the left wall, and the last one on the roof. The amount of points contained within each mapping window was measured and recorded. The point cloud density was calculated by dividing the number of points by the area of each mapping window (25 m²). Then, for each scan, the average point cloud density was calculated by averaging the point cloud density on each mapping window. The scans performed with the resolutions of 710.7 million of points and 177.7 million of points were discarded since the amount of data obtained exceeded the requirements and significantly increased the scan time.

Table 2.1. Proposed resolutions and qualities evaluated on the field.

Scan	Resolution		Quality	Real Scan time [hh:mm:ss]	Point Count	Average Point Cloud Density [Points/cm ²]	Overall Rating
	Millions of Points/Scan						
S-006	44.4	1/4	1x	0:05:12	38,103,388	11.42	5.04
S-007	44.4	1/4	2x	0:06:06	41,229,797	12.48	4.31
S-008	44.4	1/4	3x	0:07:53	41,684,133	12.64	3.36
S-009	44.4	1/4	4x	0:11:28	41,229,797	12.66	2.34
S-010	28.4	1/5	2x	0:05:27	26,273,301	8.06	4.79
S-011	28.4	1/5	3x	0:06:36	26,573,225	8.13	3.96
S-012	28.4	1/5	4x	0:08:54	26,630,099	7.97	2.95

For selecting the best operational conditions, the real scan time and the average point cloud density were taken into account. These values were normalized and averaged. The best operational conditions for this case were a resolution of 1/4 and a quality of 1x, which required a scan time of 5 minutes and 12 seconds and yielded a point cloud density of 11 points/cm². The obtained point cloud density is considered acceptable for structural mapping, bearing in mind that previous work has performed structural mapping on a LiDAR extracted

point cloud with either a density of 4 points/cm² (Lato, Diederichs, Hutchinson, and Harrap, 2009) or with a density of 16 points/cm² (Cacciari and Futai, 2017).

3.2. Stations location

Laser scanning has been used for multiple applications in both the tunneling and mining industry, such as support evaluation, scaling assessments, leakage mapping, analysis of structurally controlled overbreak, structural mapping, roughness evaluation, and deformation analysis (Fekete & Diederichs, 2013). Fekete (2010b) determined that the best practice for the laser scanner distance from the face is from 0.5 to 1 diameter. In addition, Fekete defined the optimal separation between stations is 1 diameter of the excavation to ensure the maximum coverage of the area with minimum overlap between scans and data redundancy (2010b).

Using these recommendations, the stations were positioned 13 m apart. Figure 2.3 shows the estimated locations of the different stations in which the laser scanner was set. The survey was performed around the pillar between crosscuts 15 and 16.

3.3. Scans Referencing

The laser scanner used does not have an integrated global positioning system. This means the scans are not georeferenced unless there is a reference point with known coordinates. The laser scanner used does have an internal compass and inclinometer that allows orientation of each scan with respect to magnetic north (FARO, 2011).

Without a georeferencing tool, the only way to integrate different scans into a single model is to identify common reference points, or objects, present between scans. FARO suggests the use of checkboards or spherical references placed in strategic places that can be detected from the two stations that will be referenced together. In this particular case, 21.5 cm diameter inflatable balls were used as spherical targets between scans. Table 2.2 shows the recommended distance from the target to the laser as a function of the target size and laser scan operational conditions. Considering the recommended target distances, spacing between stations, and reference target sizes, four references were used in order to reference the current station with the two other stations (one forward and one behind). As the station moves, the references that were behind it will be leapfrogged forward as the scans advance in order to reference all the scans performed in the area together. This procedure is depicted in Figure 2.4.

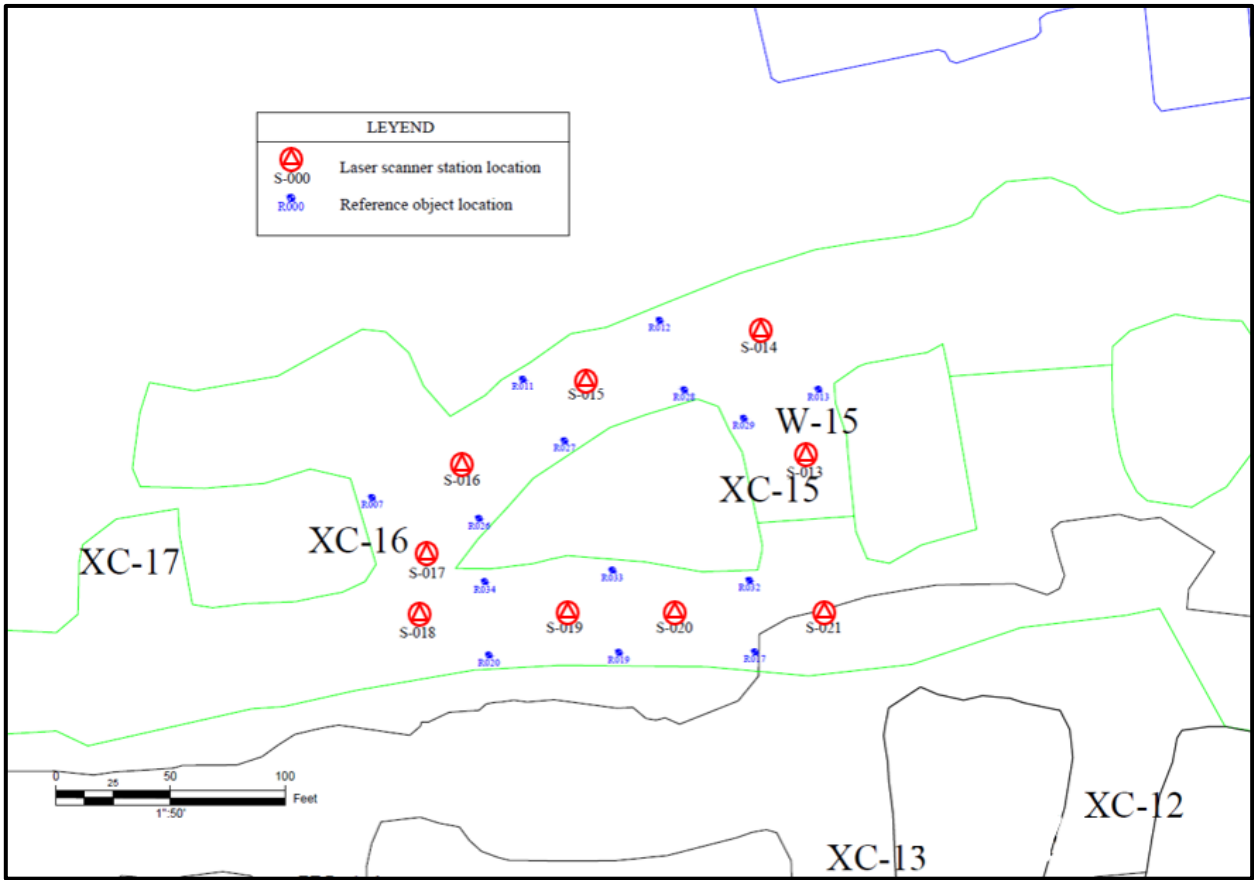


Figure 2.3. Estimated location of the laser scan stations.

Table 2.2. Recommended target spacing from laser scanner (DiCarlo, 2013).

Recommended target spacing			
Sphere size	145 mm	Sphere size	230 mm
Resolution Setting	Target Distance (max) in m	Resolution Setting	Target Distance (max) in m
1/16	5	1/16	7
1/10	7	1/10	11
1/8	9	1/8	14
1/5	15	1/5	22
1/4	18	1/4	27
1/2	37	1/2	55
1/1	73	1/1	110

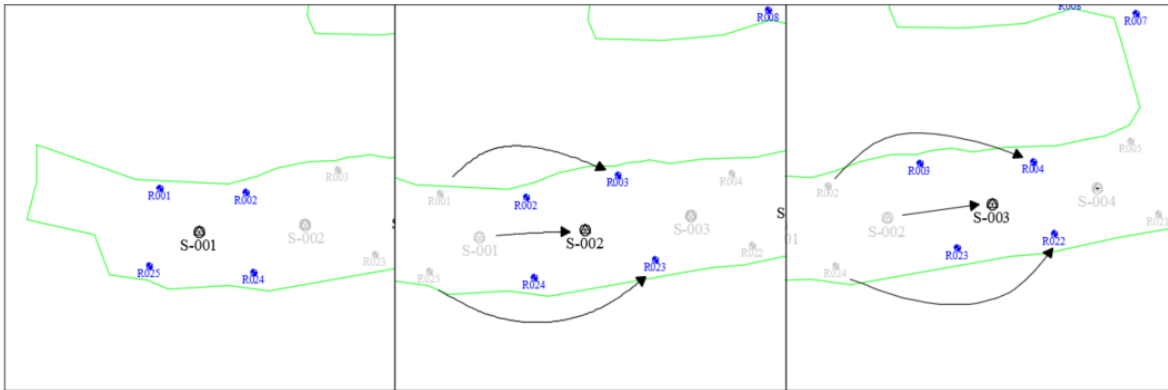


Figure 2.4. Reference objects placement along different scan stations.

3.4. Data importing and processing

Once the operational conditions and the scanning procedure were defined, the laser scans were performed. Nine laser scanning stations were set around the pillar between crosscuts 15 and 16. In order to download the laser scans and convert them into point clouds, the software SCENE was used. Before generating the final compiled point cloud, the scans were spatially referenced with each other using the reference points. This process is defined as “registration.”

When a laser scan is performed, the scanner sets the position of the mirror as the coordinates $x=0$, $y=0$ and $z=0$. If a reference point is observed from two different places, a spatial relation between those stations can be made (FARO, 2018). SCENE allows the user to open two stations at the same time and select the common points between them. Figure 2.5 presents the spatial registration process at Stations 015 and 016. There are two reference spheres on each scan.

After referencing all the scans, SCENE evaluates the distance error between points and the overlap between scan stations. This software suggests that errors with a value of less than 8 mm and overlap values greater than 25% provide acceptable results on the registration process (FARO, 2018). When the errors are greater than 20 mm and the overlap between stations is less than 10%, the results from the laser scan are not acceptable; therefore, the scanning process must be repeated by using more reference objects or by reducing the distance between stations. Values between this ranges may still be accepted, considering a less precision on the laser scan results.

Table 2.3 presents the results obtained from the registration of the nine laser scans, which obtained acceptable values for both the errors (maximum point error 6.7 mm) and the overlap (minimum overlap 25.2 %). According to these results, the recommendations made by Fekete (2010b) regarding the laser scan station locations are acceptable in this particular case. Finally, in order to proceed with the analysis, the point cloud was saved as a laser scan file.

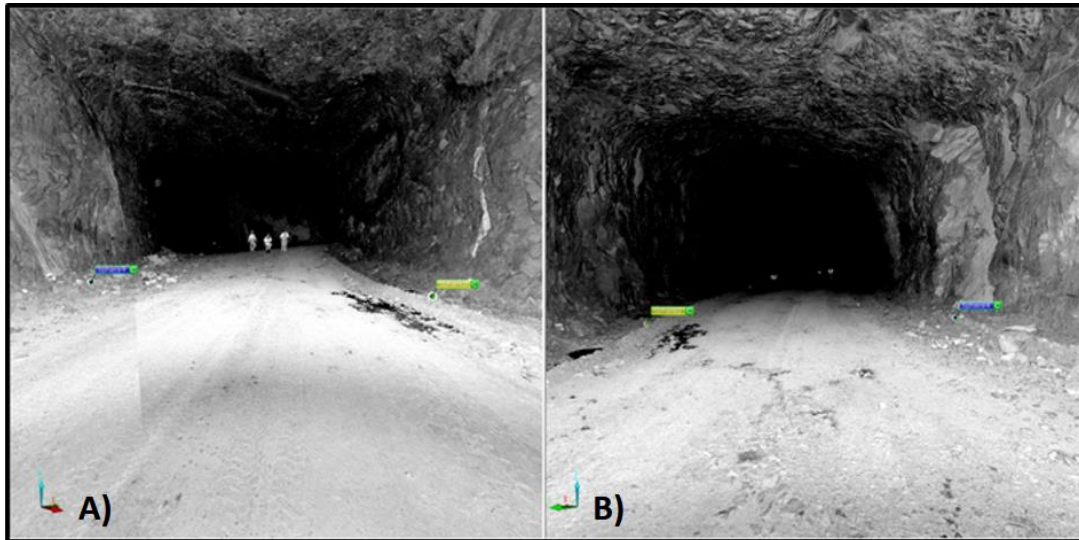


Figure 2.5. Registration and verification of stations 015 and 016. A) View from the station 015 towards station 016 and the reference spheres. B) View from the station 016 towards station 015 and the reference spheres.

Table 2.3. Point cloud registrations statistics (FARO, 2018).

Scan Point Statistics	Obtained Values	Acceptable values	Unacceptable values
Maximum Point Error	6.7 mm	< 8 mm	> 20 mm
Mean Point Error	4.3 mm		
Minimum Overlap	25.2 %	> 25.0 %	< 10.0 %

3.5. Structural data processing

I-Site Studio is a point cloud processing software that allows users to edit and process laser scans. This software contains a set of geotechnical analysis tools that assist engineers during the structural mapping process. This software allows for greater and more representative data regarding the structural information of the rock mass, which can be used to generate DFNs. The point cloud generated from the laser scans was imported into I-Site Studio to perform a structural mapping along the scanned tunnels. The generated file contained 331,230,871 points, or 14.7 gigabytes, of spatial data. Due to computational limitations, the point cloud was divided into 10 smaller sections, and each section was divided into right wall, left wall, and roof. This allowed researchers to perform the structural mapping with a useful amount of information that prevented the system from crashing.

Figure 2.6 shows the left wall of the third section generated from the overall point cloud, where it is possible to observe the geological structures. Some of these structures exhibit similar orientations and relatively large exposed areas, while others are not easy to observe due to the sight angle. Additionally, the bottom half of the figure shows the mapped discontinuities output from I-Site Studio. For the mapping process, the points that belong to

the same discontinuity plane have to be manually selected. Using the geotechnical tool “query dip and strike,” an average plane is generated that accounts for the coordinates of each selected point. This process was repeated along the whole model section by section. Figure 2.7 shows a plan view of the final point cloud with the mapped discontinuities. In total, 874 structures were mapped in the area.

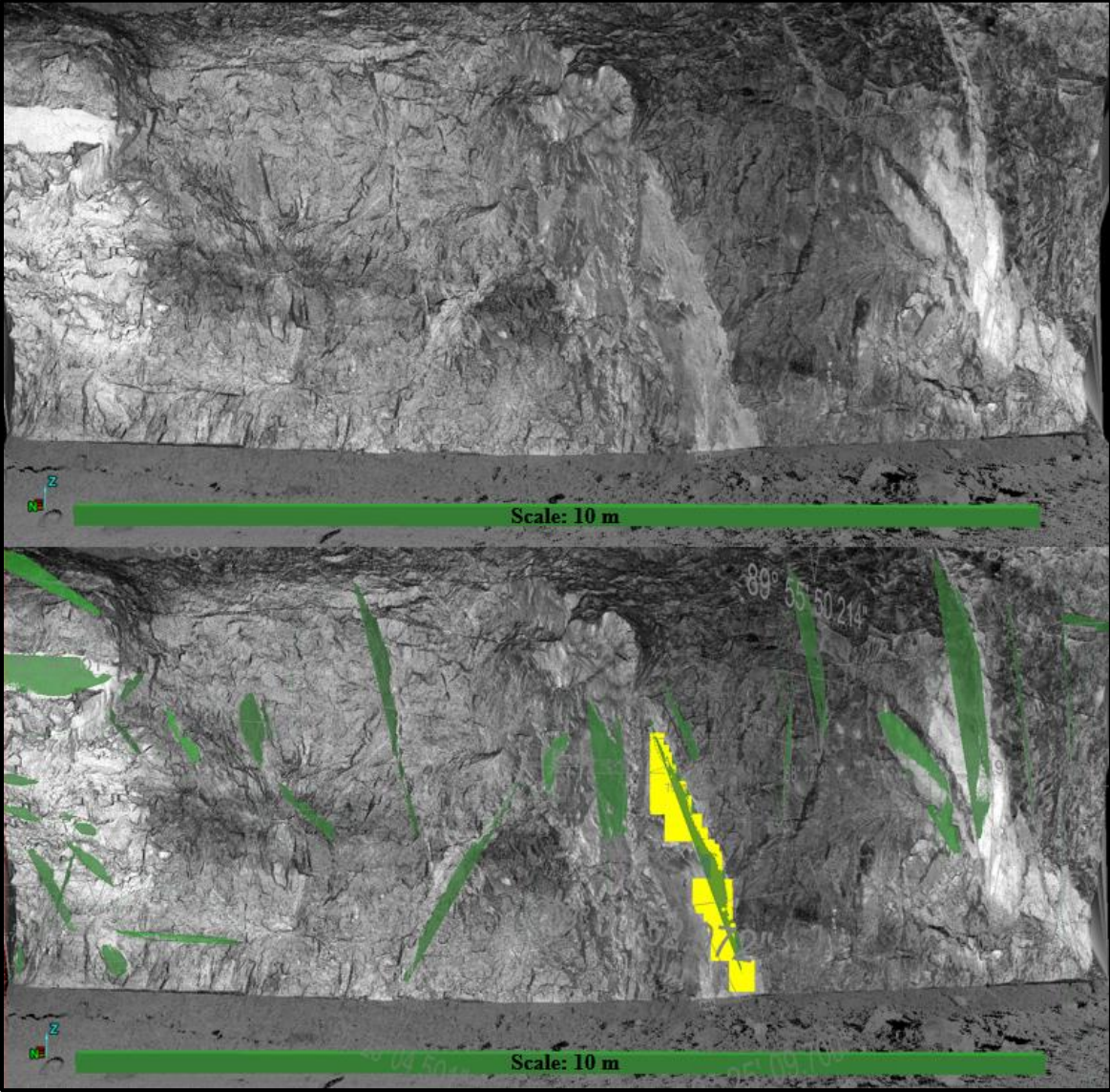


Figure 2.6. Structural mapping in I-Site.

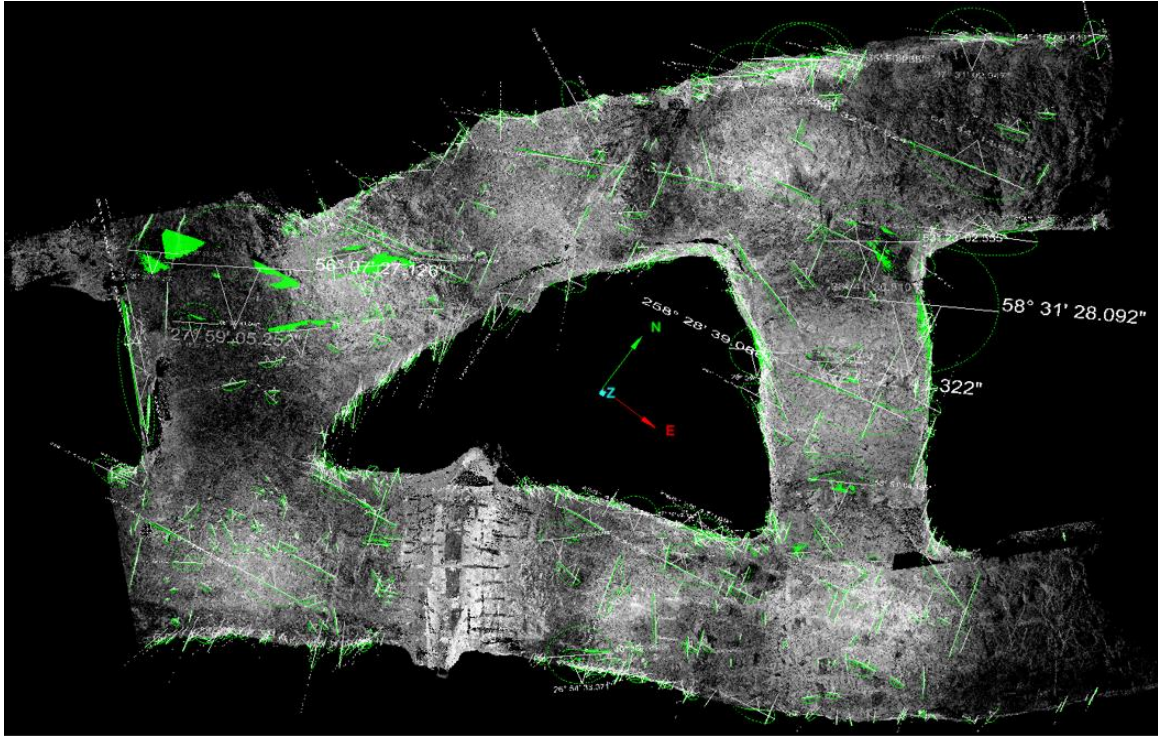


Figure 2.7. Plan view of the scanned area with structural features mapped.

3.6. DFN generation

Once all the relevant structural features observed in the 3D model were mapped, the structural data was exported into an Excel file. Structural features extracted from a point cloud into I-Site Studio contain x, y, and z coordinates; orientation information (dip, dip direction, and strike); and size information (trace length and area). This information was imported into the software Dips, which was used to analyze and classify the structures into joint sets. The defined joint sets can be filtered and reimported into I-Site Studio as individual joint sets, where geotechnical tools can calculate the spacing between discontinuities from the same family.

3DEC is a DEM software that can use DFNs to generate geological structures in a rock mass. These DFNs are a set of discrete, planar, disk-shaped, and finite size fractures, which are used to cut through the blocks that constitute the model. These sets of disks are defined based on statistical information on the characteristics of the measured fractures in the field, such as orientation, size, and density (Pierce, 2017). All of these characteristics can be obtained from the information mapped from the laser scans.

The orientation is defined by the dip (steepest declination) and dip direction (measured clockwise from true north) of a plane. This defines the spatial position with respect to the true north and a horizontal plane (ISRM, 1978). Orientation parameters are defined as circular data. Due to this, the most adequate distribution to represent this data is the Fisher distribution, which is used to represent 3D orientation vectors (Rocscience, 2018). This

distribution depends on the factor k that can be estimated from datasets greater than 30 poles. The software Dips is able to calculate each k coefficient, mean dip, and dip direction for each joint set.

While the actual size of a structure is difficult to measure, it is regularly estimated as the trace length of the discontinuity, which is the length of the exposed structure. In this case, the trace length was considered the maximum length of the mapped plane. This information provided a statistical distribution for the trace length of each joint set.

Fracture density is a measure of the frequency of discontinuities belonging to the same family. This parameter could be measured as the number of fractures per unit length (P_{10}), the length of fractures per unit area (P_{21}), or the area of fractures per unit volume (P_{32}) (ITASCA, 2018). When a DFN is generated in 3DEC, this parameter serves as a stopping condition (Cacciari, Morikawa, and Futai, 2015). For instance, when P_{10} is used as the density parameter, a sampling line is defined, and 3DEC will randomly create discs based on the predefined size and orientation distributions until the number of fractures intersecting that scan line meet the value set as P_{10} . For this work, the P_{10} estimate was based on the spacing of the discontinuities calculated using I-Site Studio, considering the definition of discontinuity frequency on a scan line as the reciprocal of the mean spacing (Hudson and Harrison, 2000). In this case, spacing values less than 50 cm were not considered when calculating the fracture density because their respective fracture densities were not comparable with those observed in the field.

4. RESULTS AND DISCUSSION

The methodology described in this work allowed the authors to obtain a 3D virtual structural mapping of the area of interest. This information is important to help visualize and understand the structural setting for an area of interest and to identify possible blocks that may form and generate rock falls. Figure 2.8 summarizes the virtual structural mapping and the statistical analysis for the trace length and the lineal fracture density of each identified structural set. Four discontinuity sets were defined from the mapped discontinuities:

- Set 1 is almost perpendicular to the tunnel orientation and presents a sub-vertical dip.
- Sets 2 and 3 are oblique joints with a steep dip.
- Set 4 corresponds to the bedding planes and contacts between rock units, which are almost parallel to the tunnel orientation and its mean dip of 29° .

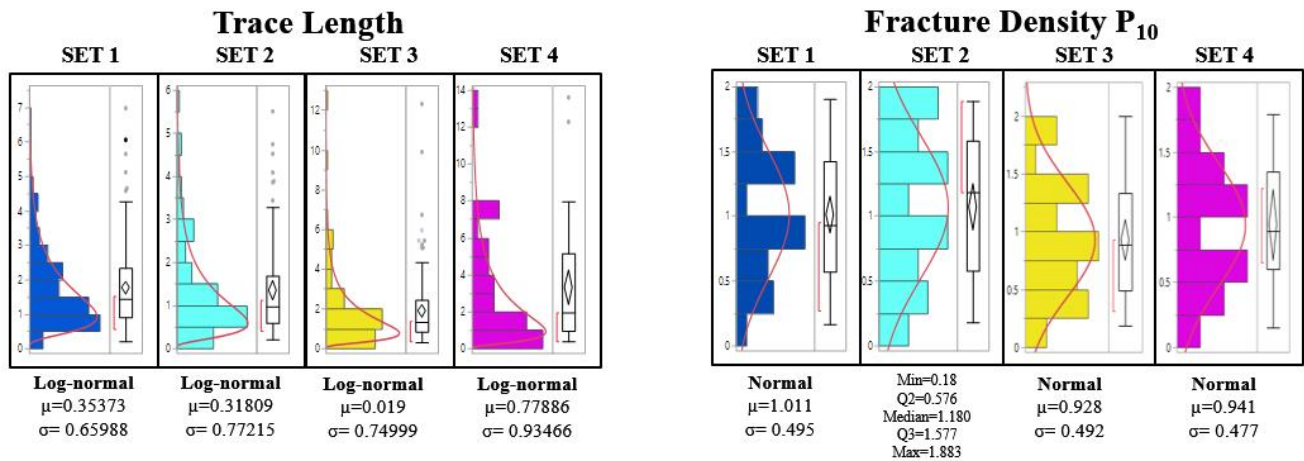
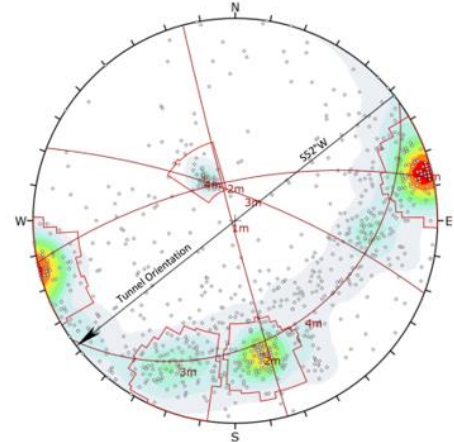
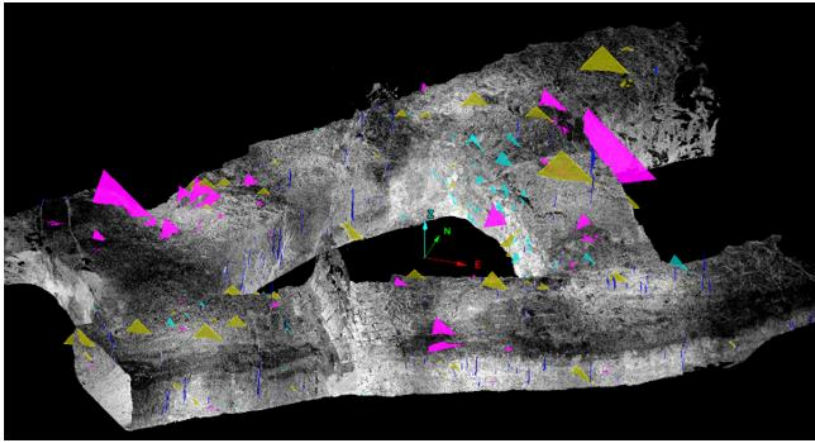


Figure 2.8. Stereo net Analysis and trace length and fracture density summary statistics.

Table 2.4 shows the statistical summary for the orientation, size, and density of each individual discontinuity set. This information was used to generate a DFN in 3DEC for each discontinuity set. Figure 2.9 shows each individual DFN and the intersection of all four DFNs. It is also possible to observe a stereographic analysis of the generated disks, which can be compared with the stereographic analysis obtained from the virtual mapping. The major challenge identified during DFN generation was density definition. If the lineal density is considered, the value is only considered along a single sample line. Therefore, if the control volume in which the fracture network is being generated is significantly larger than the sizes of the disks, a large number of disks will be generated before the fracture density condition is met.

Table 2.4. Statistical Summary of the joint properties for each joint set.

SET		S1	S2	S3	S4
PARAMETERS		N=157	N=127	N=97	(Bedding) N=45
Orientation	Dip [°]	88	68	75	29
	Dip Direction [°]	255	348	21	144
	K (Fisher)	103.9	102.4	69.5	197.3
Size	Distribution	Log-normal	Log-normal	Log-normal	Log-normal
	Mean	0.353	0.318	0.018	0.778
	Standard deviation	0.659	0.772	0.749	0.934
Density	Number of fractures per unit length of scan line (P10)	Normal $\mu=1.011$ $\sigma=0.495$	Min=0.18 Q2=0.576 Median=1.180 Q3=1.577 Max=1.883	Normal $\mu=0.928$ $\sigma=0.492$	Normal $\mu=0.941$ $\sigma=0.477$

It is important to note that the DFNs obtained in this work that resulted from a virtual mapping on a laser-scanned area do not represent the final structural setting of the simulated rock mass. 3DEC uses these discs to cut through the blocks that conform the model, ultimately obtaining a set of blocks with fractures that were generated based on the DFNs. Because of this, it is important to compare the final structural setting of the simulated rock mass with conditions observed in the field. If these differ, the model should be calibrated to obtain the best representation of the actual rock mass. Additionally, it is worth mentioning that the DFN models result from statistical distributions obtained from measured values; therefore, each time the model is run, it generates a different DFN producing different outputs. In order to obtain a significant result for the model, a stochastic analysis must be performed to address significant conclusions from the simulations.

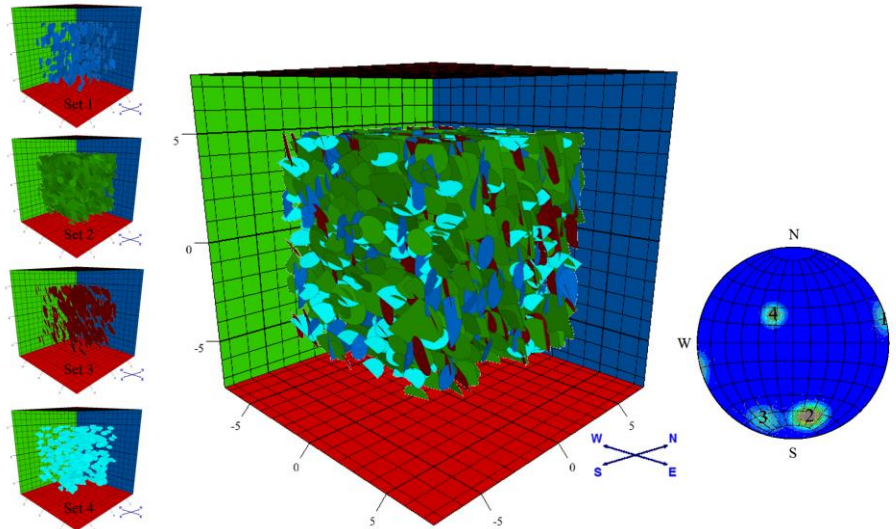


Figure 2.9. DFN Generated based on the scanned data.

5. FURTHER WORK

Since the methodology presented in the present study was developed to obtain information relevant for generating DFNs only, no mechanical properties of discontinuities on the rock mass have been measured yet. Thus, performing rock mechanics laboratory tests to measure the joint strengths is necessary. In addition, fieldwork aimed at measuring the wall strength and the roughness of each joint set will be performed. In addition, intact rock samples will also be collected to evaluate the mechanical properties of the footwall, orebody, and hanging wall. These parameters will be used as inputs for the numerical models and compared with observed conditions in the mine.

6. CONCLUSIONS

This paper describes the methodology used to perform virtual structural mapping in an underground mine from information obtained from a set of laser scans in an area that presents a structurally controlled failure mechanism. The following conclusions are derived from this work:

1. Planning the scanning project is fundamental in order to save time during the scanning process, reduce errors on the resulting point cloud, and obtain the necessary information required for the specific need.
2. While there is no existing guideline for performing an underground laser-scanning project, the experiences from other authors are valuable in defining the conditions of the scanning. For this study, following the recommendations of Fekete and Diedrichs (2013) allowed us to obtain a maximum point error of 6.7 mm and a minimum overlap between scans of 25.2%, both acceptable values according to FARO (2018).

3. It is important that geologists or engineers performing the structural mapping from the project point cloud have spent adequate time in the field in order to avoid any misleading interpretation and to ensure the results concur with those observed in the field.
4. Information extracted from the laser scans provides sufficient information to perform a statistical analysis of the variables required to generate a DFN for each identified structural set. However, further numerical models are required to define the effectiveness of this methodology in accurately representing the rock mass structure.
5. In order to perform this kind of analysis, the integration of four different software packages is required. Therefore, it is important to define an adequate workflow, which identifies the inputs required for each program and the outputs to be obtained. It is also important for the engineer to understand the limitations of each software and how these limitations can affect the results of the models.
6. Finally, the criteria of the engineer, based on their experience in the field and observed ground conditions, are fundamental for deriving a conclusion from the results of the analyses.

7. ACKNOWLEDGEMENTS

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Chapter 3 Stability Analysis of an Underground Limestone Mine Using Terrestrial Laser Scanning with Stochastic Discrete Element Modeling.³

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Abstract.

The underground limestone mining industry has one of the highest fatality rates compared to all other types of mining in the United States (MSHA, 2016). Ground control issues heavily contribute to these fatalities. According to the National Institute for Occupational Safety and Health (NIOSH, 2011), structurally controlled instability is a predominant failure mechanism in underground limestone mines. Existing analysis methods, such as the limit equilibrium method, do not represent the real structure and behavior of a rock mass in-situ. The Discrete Element Method (DEM) when coupled with representative Discrete Fracture Networks (DFN) has been proven to provide accurate models of the actual response of a rock mass undergoing excavation. However, considering the statistical nature of DFN's, multiple simulations need to be run in order to obtain statistically significant results of the model. The following paper introduces a stochastic approach for analyzing rock block falls in underground excavations. This approach utilizes DEM (Itasca's 3DEC) and DFN's representing virtually mapped discontinuities obtained from terrestrial laser scans. The numerical models were carried out considering rigid blocks. Failed blocks were defined as those blocks that had displaced more than 2 cm and presented velocities indicating that the block was still in movement. This approach allows engineers to define the probability of block failure based on the geometry and weight of failed blocks formed by the intersection of discontinuities in the section of interest, as well as to define kinematics of the blocks. Such information can provide mining operators control measures to evaluate, map and mitigate risks associated with rock falls in underground mines, ultimately improving safety in the underground limestone industry.

1. INTRODUCTION

Ground control instability issues are one of the main causes of accidents and fatalities in underground mining in the U.S. (MSHA, 2016). Within these ground control issues, structurally controlled instability is one of the main failure mechanisms in underground limestone mines (NIOSH, 2011). This type of instability takes place when two or more structural sets intersect forming rock blocks that can fall as the excavation advances (Goodman & Shi, 1985) (Hudson & Harrison, 2000). Additionally, in room and pillar mining

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operations, a rock fall due to structurally controlled instability may affect the shape of a pillar, subsequently reducing its strength parameters, and ultimately leading to a pillar failure (Brady & Brown, 1985). This failure mechanism depends on several factors, such as the structural condition of the rock mass (including number of discontinuity sets, orientation, spacing and size of discontinuities), the mechanical properties of the discontinuities, and the orientation and size of the excavations. These structural features are generated as a response to the stress changes throughout the geological history of the rock mass, and they can vary from location to location and even within the same mine. In order to evaluate the block fall risk these discontinuities should be adequately identified, measured and mapped.

Previous research has shown that terrestrial laser scanning (TLS) and photogrammetry technologies are able to obtain adequate density point clouds for virtual discontinuity mapping (Lato, Diederichs, Hutchinson, & Harrap, 2009) (Cacciari & Futai, 2017) (Monsalve J. , Baggett, Bishop, & Ripepi, 2019). The information obtained from these mapping processes can be used to perform engineering analyses aimed at identifying the potential blocks that form in the excavation. In an earlier work, Monsalve, et al. (2018) describe and compare the two current analysis methodologies to evaluate structurally controlled instability: the Limit Equilibrium Method (LEM) and the Discrete Element Method (DEM). The authors concluded that the DEM with detailed structural information, such as the one obtained from TLS by virtual discontinuity mapping and Discrete Fracture Network (DFN) generation, provides both deterministic and stochastic results to better understand the rock mass behavior under excavation (Monsalve J. , Baggett, Bishop, & Ripepi, 2018).

The DEM is a numerical analysis tool that can simulate the behavior of a discrete body, such as a rock mass, under either static or dynamic loads. The blocks that constitute the rock mass are formed by intersecting explicit fractures whose locations, orientations and dimensions are necessary to define the model (Jing & Stephansson, 2007). Specifically, 3DEC has the capability to generate Discrete Fracture Networks (DFNs) and use them to cut through a block until a structural condition similar to that of the rock mass is reproduced (ITASCA, 2016). Each DFN is composed by a number disk shaped fractures. When a block is cut with a DFN, each fracture belonging the DFN must completely bisect a block. If a fracture is in between two different blocks, both blocks will be completely bisected, even if this fracture is smaller than this blocks. Due to this, in a final fracture model it is not possible to have isolated circular fractures as the ones belonging to the DFN. Figure 3.1 shows a comparison between the DFNs and the final fractured model. It is observable that even though there are similar patterns between both, they are not identical. Due to this discrepancy, it is important to calibrate this model so the final structural conditions are as closely representative of those observed in the field and mapped from the laser scans.

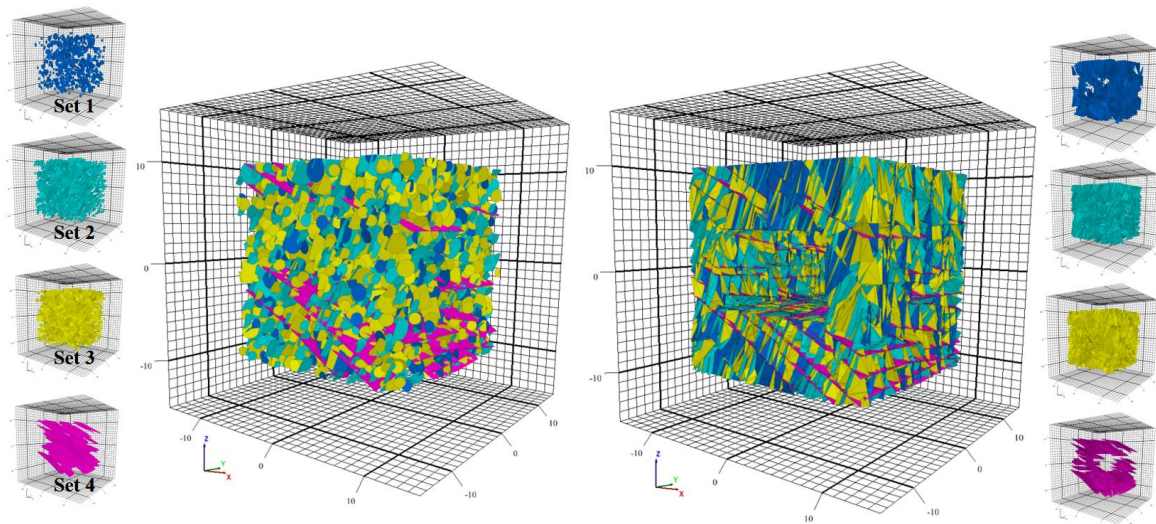


Figure 3.1. Comparison between DFNs and actual fractures in the model.

DFNs are a geometrical representation of the structures mapped from the rock mass, based on statistical information of their orientation, size and frequency (Pierce, 2017). Their nature is merely statistical. If further stability analyses are intended, these require a stochastic modeling approach (Monsalve J. , Baggett, Bishop, & Ripepi, 2019). Different authors have performed stochastic stability analyses of failure in fractured rock masses using DFNs; however, they have used LEM analysis. Grenon & Hadjigeorgiou (2003) evaluated the stability of wedges in underground mines at the periphery of mining stopes by integrating a 3D joint network generation model with a 3D limit equilibrium software package. Fu & Ma (2012) presented an algorithm that evaluates progressive failure in rock masses based on the LEM and the key block theory. This method would be extended later to perform a stochastic analysis of progressive failure in rock slopes and underground excavation, obtaining probability density functions indicating the amount of failed blocks (key blocks) and their volumes (Fu, Ma, Qu, & Huang, 2016). Grenon, Landry, Hadjigeorgiou, & Lajoie (2017) compared different fracture network models and evaluated the stability of a drift in an underground mine with stochastic limit equilibrium analysis based on previous work. Similarly, Rogers, Bewick, Brzovic, & Gaudreau (2017) performed stochastic LEM analysis in an underground excavation, obtaining the DFN's information from photogrammetric surveys. As is shown by these limited number of studies, there is a lack of research in stochastic methods using the DEM.

The following paper presents the stability analysis of a previously defined case study mine based on a stochastic discrete element model approach. This analysis method is performed by running multiple iterations of a discrete element model in 3DEC that simulates a 20-meter long section of the excavation. This method resulted in an estimate of probability for the number of failed blocks and the volume of the unstable mass of rock.

2. GEOTECHNICAL CONDITIONS

In previous work, Monsalve et al. described the geotechnical conditions of the CSM. This operation extracts a 30 m thick and 30° dipping limestone body with a room and pillar mining method with eventual stoping. The stopes are conformed along the strike of the body, when the seam is consistent and thick enough. These are conformed by top and bottom drifts, 12.8 m wide and 7.6 m high, which are separated by a 15 m sill which is extracted by vertical long hole drilling (Sill and pillar mining). The stopes are supported by 24 m by 24 m pillars. This rock has a UCS of 159.2 MPa ± 21.25 MPa, a tensile strength of 6.3 MPa ± 1.99 MPa and a Young's modulus of 64.11 GPa ± 2.37 MPa. In addition to this, an average GSI value of 75 was reported throughout the area of interest. The deepest point in the mine is approximately 700 m below ground surface and is exposed to a maximum vertical stress close to 20 MPa.

As described in the above mentioned previous work, according to the risk/Hazard Assessment Chart proposed by Martin, Kaiser, & Christiansson (2003), the main cause of instability in this excavation is due to gravity-induced structurally controlled block movement, which agrees with the observed failure conditions in the mine (Monsalve J. , Baggett, Bishop, & Ripepi, 2018). If the horizontal stress is assumed to be the principal maximum stress and 1.2 times higher than the vertical stress, for the mine to be under stress induced failure the excavations should be deeper than 750 m, as it is presented in Figure 3.2. Considering that the deepest level in the mine is 700 m below ground surface, stress induced failure mechanisms are unlikely to be encountered in this mine.

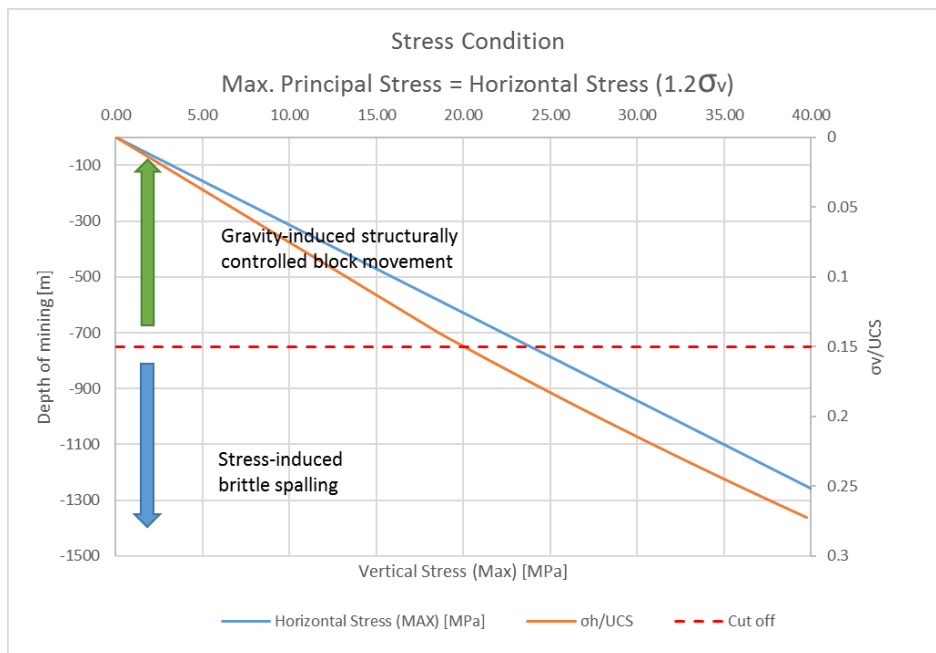


Figure 3.2. Failure mechanisms depending on the depth of the mine based on the risk/Hazard Assessment chart proposed by Martin, Kaiser, & Christiansson (2003).

The ore body is located in the limb of a regional syncline structure. From terrestrial laser scanning and virtual discontinuity mapping, the authors were able to identify four main discontinuity sets. These discontinuity sets were classified as: Set 4, which corresponds to the bedding planes and contacts between rock units, which are almost parallel to the tunnel orientation and has a mean dip of 29°; Set 1, which is almost perpendicular to the tunnel orientation and presents a sub-vertical dip; and Sets 2 and 3, which are oblique joints with a steep dip. These four identified structural sets agree with the model of joints in folded sediments proposed by some authors, where Set 1 corresponds to Dip joints, Set 2 and 3 could be classified as oblique or shear joints, and Set 4 corresponds to bedding planes, as can be observed in Figure 3.3 (Brady & Brown, 1985) (Blyth & de Freitas, 1984).

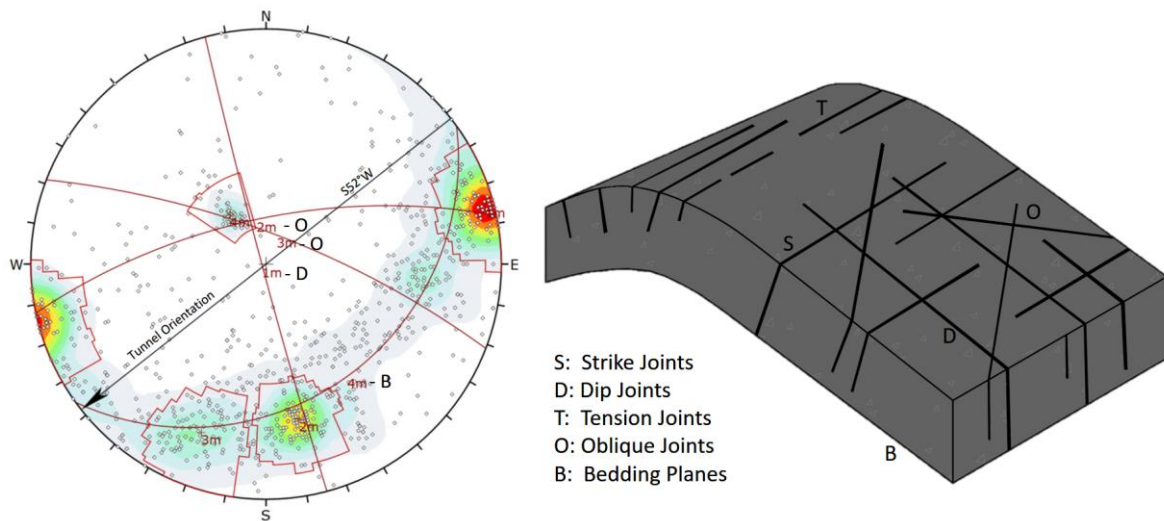


Figure 3.3. Relation between structural sets defined in the CSM and Jointing model in folded sediments.

In addition to this, additional field work aimed at measuring the mechanical properties of the discontinuities was carried out. In general, it was determined that all discontinuity sets are mainly very closed. The weathering conditions ranged between decolored and fresh for all the discontinuity sets. The majority of the discontinuities were completely damp, except for some discontinuities located closer to karst features, which were encountered wet and a few of them were dripping. Fifty-four percent of the mapped discontinuities did not present any filling material, while 22% were filled with calcite and the other 24% were filled with mud, mainly coming from the karsts. In addition, 64 % of the fractures from all the families presented a joint roughness coefficient (JRC) between 2 and 4, while the other 38 % of the fractures presented higher JRCs between 6 and 10. The bedding planes were the fractures that presented the highest JRCs. In addition to this, tilt tests were performed which obtained friction angle values ranging from 30° to 35°.

3. ANALYSIS METHODOLOGY

Initially, preliminary models were developed considering simple tunnel geometries and simplified DFNs. These initial models were calibrated and improved until the model closely represented the rock mass structure and the excavation section. The final model represents a 20-m long section of the excavation and takes into account the four mapped discontinuity sets. The model was run for 40000 cycles after excavation, with a time step of 3.81×10^{-6} seconds, taking 6 hours of processing time. The hardware consisted on a MSI laptop computer with 16 Gb of RAM and a 7th generation Core i7 processor. Once the model was run, a function was written to mark failed blocks (Fekete & Diedrichs, 2013). The failed blocks were defined as those blocks that had displaced more than 2 cm and presented velocities higher than 5×10^{-5} mm/s indicating that the blocks were in movement (Curden & Varnes, 1996). Then information about the blocks was stored in a text file. This information included block number, block volume, block mass and velocity.

As mentioned earlier, due to the stochastic nature of the DFNs, the results obtained just from one simulation are not representative of the overall response of the rock mass under excavation. Due to this, the model was run 30 times, reporting the same information for each iteration. This was done by generating a master file which ran the model several times and varying the random seed number to ensure every result was different. The iteration number was stored in the text file along with the block information. This enabled the performance of further analyses on the extracted information. In order to validate the models, the results were compared with the 3 dimensional point clouds obtained from the laser scans. Two dimensional sections from both models were extracted and compared.

The text file was imported into an excel file where information was processed and analyzed. The parameters that were evaluated were total number of failed blocks and total volume of failed blocks. These two parameters give insight into the amount of rock fall that can occur in the excavation given certain structural condition in the rock mass. The statistical analysis software JMP was used to perform a statistical analysis on the results (Proust, 2018). Probability Density Functions were fitted and validated for these two parameters, allowing the probability of block failure in the 20 m long section to be estimated. On the other hand, the x, y and z velocity components were converted to stereographical data. This information allowed the kinematics of the failed blocks to be better interpreted.

4. 3DEC MODEL

This section explains how the 3DEC model was programed and describes the conditions and assumptions considered in this study.

Initially, a DFN was generated for each discontinuity set present in the rock mass. These DFNs were defined within a control volume of 20 m x 20 m x 20 m. The input parameters considered for this were obtained from the authors' (Monsalve J. , Baggett, Bishop, & Ripepi, 2019) previous research . The parameters required to generate the DFNs were orientation,

size and density. For orientation, a fisher distribution was considered for all the discontinuity sets. Dip, dip direction and K values are presented for all discontinuity sets in Table 3.1. In order to simplify the model, the y axis was defined parallel to the tunnel orientation (N52°E). Due to this, all discontinuity sets were rotated 52° counterclockwise to maintain their orientations respective to the excavation. For Sets 1, 2 and 3, a log-normal distribution for the size was considered based on the values obtained from the laser scans. A function was written to consider log-normal distributions for joint size, since 3DEC DFN generator does not include these distributions for this parameter. The values measured from the laser scans were not considered for Set 4, since it corresponds to the bedding, which extends consistently along the rock mass. Instead of this, a Gaussian distribution, with a mean of 9 m and a standard deviation of 1 m, was used to generate this DFN, allowing it to better represent this family. The area of fractures per unit volume (P_{32}) was considered as the density parameter. The P_{32} value was calibrated until the measured number of fractures per unit length of scan line (P_{10}) in the resulting fracture model reached values close to those obtained from the virtual discontinuity mapping.

Table 3.1. Statistical Summary of the joint properties for each joint set.

SET		S1	S2	S3	S4 (Bedding)
PARAMETERS		N=157	N=127	N=97	N=45
Orientation	Dip [°]	88	68	75	29
	Dip Direction [°]	255	348	21	144
	K (Fisher)	103.9	102.4	69.5	197.3
Size	Distribution	Log-normal	Log-normal	Log-normal	Normal
	Mean	0.353	0.318	0.018	9
	Standard deviation	0.659	0.772	0.749	1
Density	Input area of fractures per unit volume (P_{32})	0.1	0.4	0.4	0.5

The geometry of the excavation was based on the general mine design. The section is rectangular with a width of 12.8 m and a height of 7.6 m. This geometry was modeled in Rhinoceros and imported into 3DEC using Griddle (ITASCA, 2017). Two sections in the geometry were considered: 1) an inner section, considered as the fractured rock mass which contained the excavation; and 2) an external section, considered as the massive rock mass for the sake of the simplicity for the model. After the geometry was imported, the blocks were cut with the DFNs. The cutting order was a relevant parameter to simulate the rock mass structure, since it altered significantly the fracturing model. For this study, the set bedding planes were used to cut the model first, followed by Set 1 which corresponded to dip joints, followed by Sets 2 and 3 which corresponded to shear joints. Figure 3.4.a) shows the jointing model after the DFNs cut the initial block. Figure 3.4.b) illustrates the inner section, representing the fractured rock mass with the excavation. In addition to this, Figure 3.4.c) displays a two dimensional section of the excavation, where the fractures can be noticed.

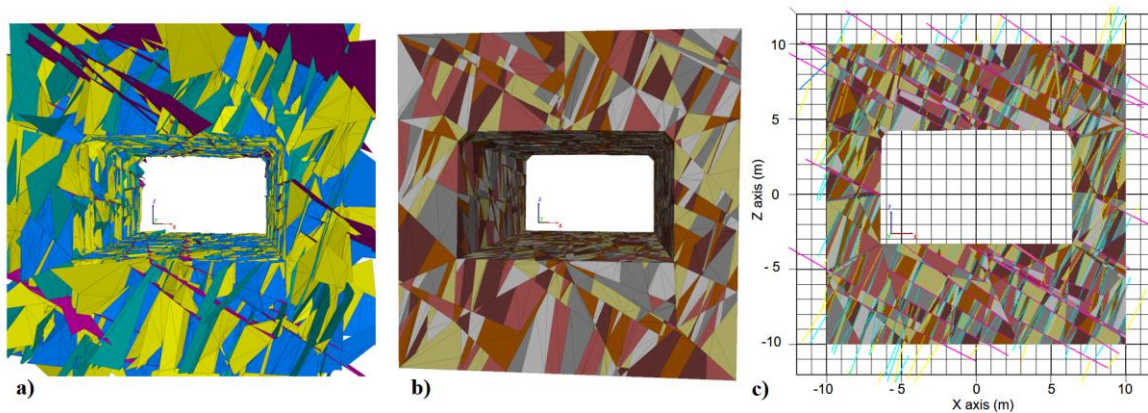


Figure 3.4. a) Jointed model b) Inner Section of the Geometry indicating the Jointed rock mass. c) XZ 2-Dimensional displaying the excavation the rock blocks and joints.

Considering that the main failure mechanism defined was gravity-induced, structurally controlled block movement, and that there was no evidence of failure resulting from high stresses, it was decided to use rigid blocks in the model. This means that the only deformation observed in the simulation occurs along discontinuity planes and the blocks do not change their geometries. The gravity was set as 9.81 m/s^2 . Additionally, the external section of the geometry was fixed, preventing it from displacing. Thus, the only blocks that were displaced were the ones belonging to the fractured rock mass section.

The intact rock properties were obtained from previous laboratory testing performed by the mine operator. Blocks were assigned a density of 2700 kg/m^3 and were assumed to be infinitely stiff (rigid). Furthermore, discontinuity properties were assumed to have a friction angle of 30° and no cohesion based on tilt tests and field observations. These values represented the worst discontinuity strength conditions. A joint shear stiffness of 30 MPa/mm and a joint normal stiffness of 300 MPa/mm were used. These values were obtained based on the work performed by Bandis, Lumsden, & Barton (1983).

Once the model was set up, it was cycled 500 times to reach initial equilibrium before excavation. After these initial cycles were run, the material inside the tunnel was excavated and run for an additional 40000 cycles. Once, these cycles were finished, the failed blocks analysis stage proceeded.

5. RESULTS AND DISCUSSION

Figure 3.5 shows the results obtained from one of the simulations in 3DEC. In the model, red indicates blocks that defined as failed blocks, while grey indicates those blocks that have not yet failed. Velocity vectors also are marked, indicating blocks that are still displacing. In this particular simulation the maximum displacements obtained are 4.62 cm and the maximum block velocities obtained are 0.6 m/s . Additionally, the figure shows that the majority of failed blocks on the roof presented geometries ranging from oblique triangular pyramids to

irregular prisms, which are shown isolated in Figure 3.6. Not only did working with rigid blocks showed an acceptable rock mass behavior, but this assumption significantly reduced the processing time as well.

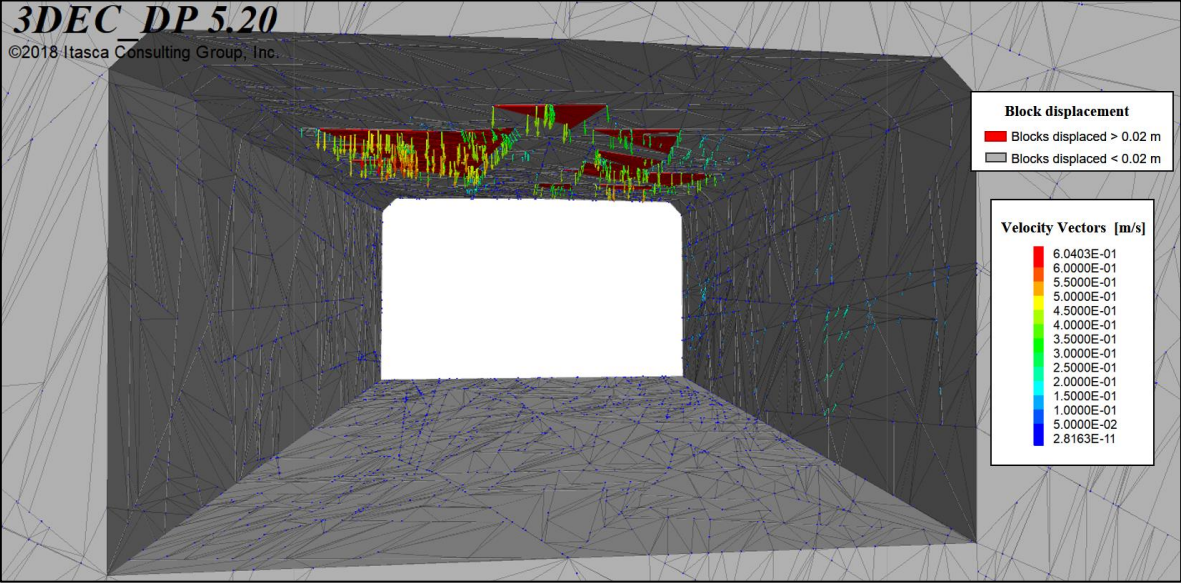


Figure 3.5. Discrete element model of the study area indicating failed blocks.

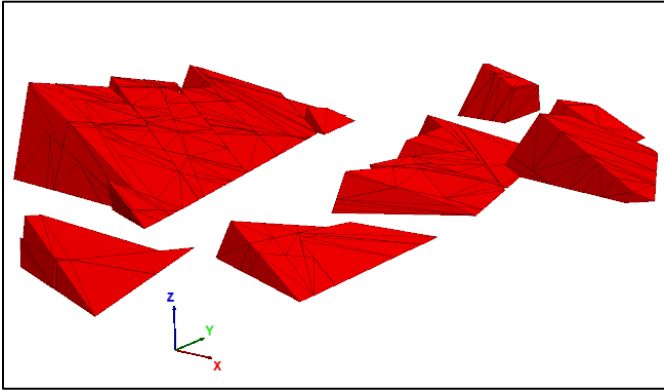


Figure 3.6. Typical geometries of failed blocks in the discrete element model.

Results obtained from the simulations were compared with previously Laser-scanned sections. Taking into account that the 3DEC models were stochastic, they were not meant to match exactly the laser scanned sections. However, the blocks formed in the numerical model still shared similar volumes and shapes to those observed in the laser-scanned sections. Figure 3.7 compares two 2-dimensional sections, one extracted from laser scans and the other extracted from numerical models. Results from both show similarities between the locations, sizes and shapes of the blocks. Figure 3.8 shows a failed block from 3DEC matching the gap left from a failed block in a laser scanned section in the study area. These results indicate that the structural model obtained from the virtual discontinuity mapping somehow adequately represents the structure of the rock mass.

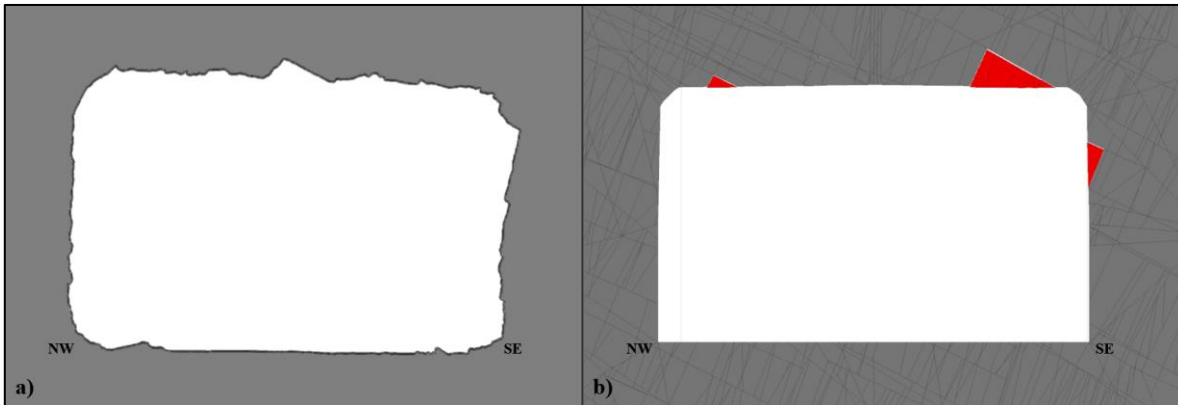


Figure 3.7. Comparison between 2 dimensional section obtained from a) Laser Scanning Point clouds and b) Discrete Element Model.

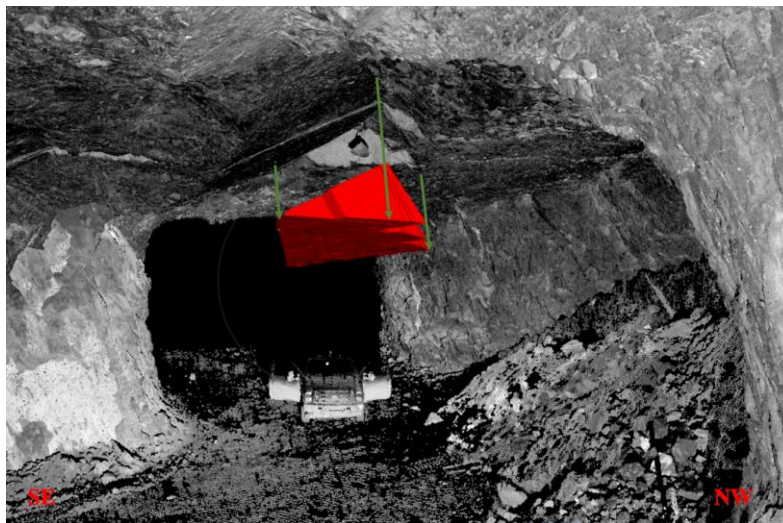


Figure 3.8. 3DEC failed block matching a gap left from a failed block in a laser scanned section.

The stochastic modeling stage resulted in 30 discrete element models and took around 180 hours. Figure 3.9 shows some of the models obtained from this analysis. As mentioned earlier, information for each of the 3,146 failed blocks was recorded. Two main parameters were evaluated from the models: the number of failed blocks per iteration, and the total volume of failed blocks per iteration. This information was analyzed in the statistical software JMP, where probability density functions were defined for these parameters. The volume of failed blocks was fitted to a log-normal distribution, with a mean of 2.60 m^3 and a standard deviation of 0.49 m^3 . This distribution was validated with the Kolmogorov's D goodness of fit test. On the other hand, the number of failed blocks was distributed normally with a mean value of 104.86 and a standard deviation of 47.58. Similarly, this distribution was validated using the Shapiro-Wilk W goodness of fit test. These results indicate that considering the present structural condition in this section of the mine, there is 35% probability for a total volume of 10 m^3 of rock blocks to fall in 20 m of tunneling advance. Results obtained from this methodology offer engineers an accurate tool to estimate the mass of failed blocks in excavations under structurally controlled failure mechanism.

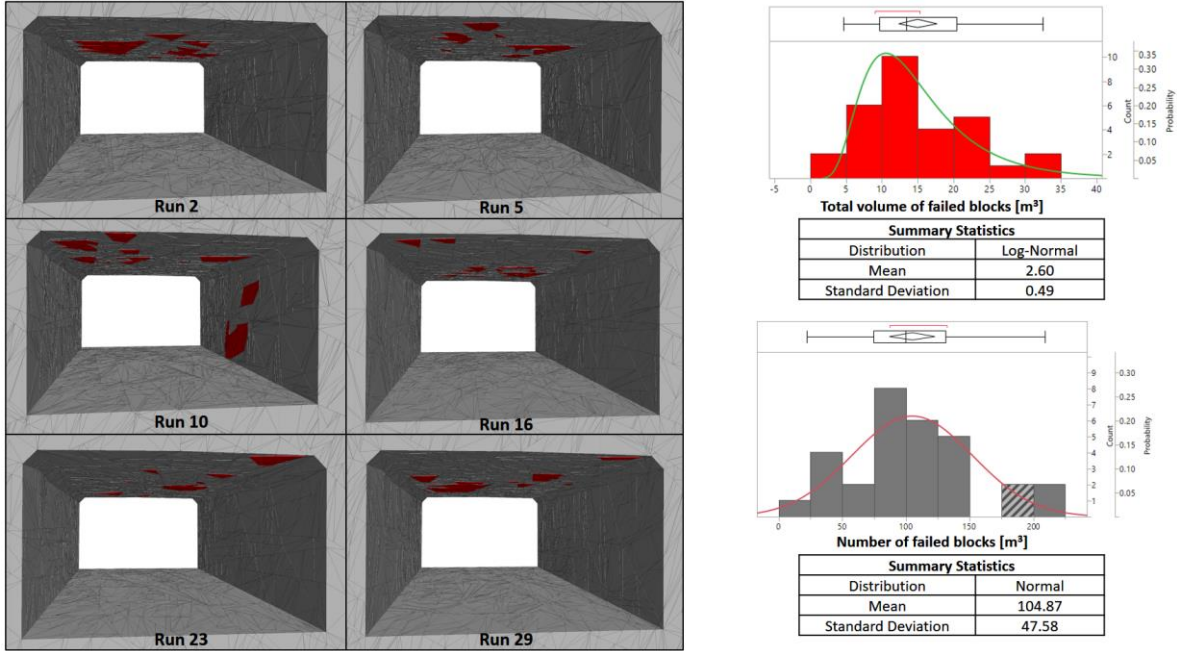


Figure 3.9. Stochastic approach to block failure analysis.

Once the failed blocks probability was estimated, as described in previous section, a block kinematics analysis was performed. The velocity vectors from all the models were transformed into spherical coordinates, allowing them to be presented in a stereonet. Figure 3.10 shows a model of failed blocks in the roof and in the south-east wall of the excavation, as well as the velocity vectors for all failed blocks. The figure demonstrates that the majority of the blocks were falling vertically from the roof; however, other vectors were concentrated more towards the NW. These vectors represent blocks falling from the south-east wall towards the excavation.

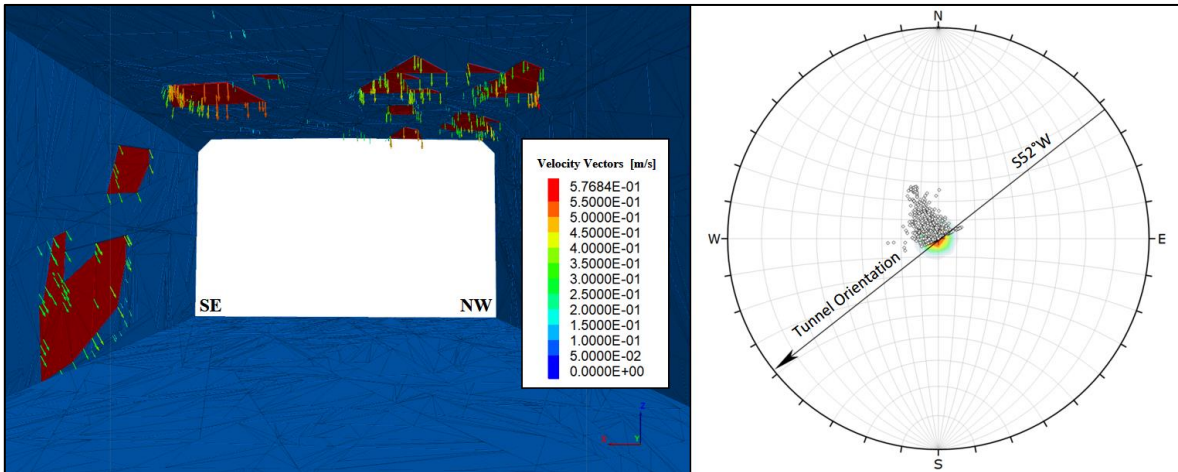


Figure 3.10. Stereographical analysis of block kinematics.

Results obtained from this stochastic model can be extended to any sector of the mine sharing similar structural condition as that mapped on the study area. If a different structural condition is reported, these models must be repeated considering the new structural setting. The methodology described in this work can be applied in any underground limestone mine presenting a structurally controlled failure mechanism, and the results obtained from these analyses can be used to define probability of block failure. Similarly, the data and information resulting from this work can be integrated into a risk management system to control risks associated with rock fall in a mine, ultimately improving safety in the operation.

6. FURTHER WORK

Based on the results of this work, a number of different research opportunities are identified. This methodology can be extended to rock support design under structurally controlled instability. Pillar strength on fractured pillars could also be analyzed by using stochastic discrete element modeling with 3DEC. In underground mines where stresses can be an additional issue, this methodology may also be applied if deformable blocks are considered, even though, the analysis time may significantly increase.

7. CONCLUSIONS

This work presented a methodology for analyzing the stability of an underground limestone mine under structurally controlled failure. This methodology integrated results from terrestrial laser scanning with Discrete Fracture Networks and Discrete Element Modeling software. The following conclusions can be drawn from this work:

1. Terrestrial laser scanning was successfully integrated with Discrete Element Modeling by applying discrete fracture networks in the model. The information obtained from virtual discontinuity mapping in the laser scans was used as inputs for generating the DFNs which were then used to build the 3DEC model.
2. It is important to have a general understanding of the geological model of the rock mass. This knowledge dictates how the 3DEC model is built. In this work, the joint set

corresponding to the bedding planes was used to first cut the blocks than the other sets. This enhanced the results of the rock mass model.

3. In this study, working with rigid blocks was an acceptable assumption, which yielded results similar to those observed in the field. Should this methodology be used in an underground excavation presenting both structurally controlled and stress controlled failure mechanisms, it is recommended to work with deformable blocks, which may increase significantly the processing time.
4. The results obtained from the model agreed with the failure mechanisms observed in the field. The results were also comparable with those obtained from laser scans, indicating that the parameters considered in the model were acceptable.
5. Considering the stochastic nature of DFNs, it is necessary to extend the discrete element models to a stochastic approach in order to obtain significant results. A stochastic modeling approach allows engineers to measure the probability of block failure in a section of the excavation. This methodology could also be extended to rock support design and pillar strength determination.
6. The present methodology can be used as a method for rock fall hazard identification in underground limestone mines. It can be easily integrated into a risk management system, allowing engineers and mining operators to have greater control over possible block failures, to reduce ground control related accidents, and to improve the safety of these operations.

8. ACKNOWLEDGEMENTS

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Chapter 4 Conclusions, Recommendations and Future Work

Throughout this work a methodology for analyzing the stability of an underground limestone mine under structurally controlled failure was presented and tested. This methodology integrated results from terrestrial laser scanning (TLS), virtual discontinuity mapping (VDM), Discrete Fracture Network (DFN) generation and Discrete Element Modeling (DEM). This methodology was tested in an underground limestone mine that extracts a 30° dipping limestone body with a room and pillar mining method with eventual stoping. The primary failure mechanism of this mine was structurally controlled block movement. A study area was defined along with mine personal and researchers. In this area 9 laser scans were performed. A point cloud was extracted from this scans, which were used later for VDM. During the VDM process information such as spatial location, orientation, trace length, and spacing was extracted for each discontinuity set. These information was used to generate DFNs for each fracture set. These allowed us to generate fractured rock mass models in 3DEC a DEM software. The DEM allowed us to estimate the probability of rock block failure in a 20 m long tunnel considering the structural parameters obtained from the study area. The following lines present some of the conclusions that arose from this work.

- 1) Similar to other authors mentioned in this work TLS was proved to be a powerful tool for rock mass characterization. Not only this technology was time efficient, but it also provided detailed information that allowed to generate DFNs which were used later to generate a fractured rock mass model that represented accurately the rock mass behavior under excavation.
- 2) Planning the scanning project was fundamental in order to save time during the scanning process, reduce errors on the resulting point cloud, and obtain the necessary information required for virtual discontinuity mapping. While there is no existing guideline for performing an underground laser-scanning project, the experiences from other authors were valuable in defining the conditions of the scanning.
- 3) DEM software such as 3DEC is a powerful tool to interpret and analyze structurally controlled instability in underground excavations. This software with detailed structural information such as the one obtained from TLS and VDM provided not only deterministic but also stochastic analysis, which allowed us to understand better the behavior of the rock mass and provide better estimations of the probability of rock block failure in the case study mine.
- 4) TLS was successfully integrated with DEM by applying discrete fracture networks in the model. The information obtained from VDM in the laser scans was used as inputs for generating the DFNs which were then used to build the 3DEC model. The integration of both technologies (TLS and DEM) was successfully used in order to estimate the

probability of rock falls in an underground excavations and can be used to enhance worker's safety.

- 5) A clear understanding of the geological model of the site is important in order to improve the results from virtual discontinuity mapping and the fractured rock mass model. The geologist or the engineer in charge of the structural mapping and the numerical modeling are required to expend adequate time in the field in order to avoid any misleading interpretation and to ensure the results concur with those observed in the field.
- 6) A rigid block model was utilized based on an empirical risk/hazard assessment and field observations. This assumption yielded results that agreed with the field observations and laser scanning results. Additionally, the computing time was significantly reduced by implementing this assumption. This rigid block model is only applicable if stresses are not a main concern in the excavation. Should stress controlled failure mechanisms start to play a role in the stability of the excavation, a deformable block approach needs to be considered.
- 7) A stochastic Discrete element modeling approach was selected for the analysis due to the stochastic nature of DFNs. This modeling approach offered broader inference on the rock mass behavior under excavation. The results obtained from this analysis can be applied to any part of the mine where the structural setting is similar to that of the laser scanned area in this study. If a different structural setting is identified or the orientation and dimensions of the excavation changes, it is required to update the stochastic numerical analyses.
- 8) The present methodology can be used as a method for rock fall hazard identification in underground limestone mines. It can be easily integrated into a risk management system, allowing engineers and mining operators to have greater control over possible block failures, to reduce ground control related accidents, and to improve the safety of these operations.
- 9) The case study mine offered an ideal environment to apply both technologies (TLS and DEM), and to develop risk analysis methodologies based on these technological advances, ultimately reducing accidents related to rock falls in the underground stone mine industry.
- 10) The methodology here presented required the integration of different software packages (SCENE, I-Site, DIPS, 3DEC, JMP). Due to this, it was important to define an adequate workflow, identifying the inputs and outputs required and produced for each program. Furthermore, it is also important to understand the limitations of each software and how these limitations can affect the results of the models.

In addition to the conclusions obtained from this work, a series of recommendations that should be followed in order to implement this methodology in any underground limestone mine presenting similar failure mechanism are made. These recommendations are detailed in the following lines.

- 1) Considering that structurally controlled failure mechanism is governed by the structural setting of the rock mass, it is important for mining operators to keep track of these conditions. An adequate mapping and reporting of these sets of fractures and their properties will allow engineers and miners to be aware of the possible rock block fall that can occur in certain sectors of the mine.
- 2) If the structural mapping is decided to be done by using TLS, it is not necessary to perform these laser scans after every advance. In order to avoid collecting excessive information, periodical weekly surveys are proposed. These surveys could also be complemented with conventional rock mass characterization. The information obtained from this mapping process should be used mainly to keep track of changes in the structural sets along the mine.
- 3) Considering that underground limestone mines are generally continuous and regular deposits, and the majority of the excavations are made in the same rock type, the structural geology can be used as main parameter for defining the different geotechnical units throughout the mine. TLS and virtual discontinuity mapping offer a useful methodology to keep track of these changes and mapping these geotechnical units.
- 4) Not only this methodology can be applied in already on going mining projects, but also it can be implemented from the beginning of a mining project since exploration and prefeasibility studies. Current technologies such as borehole scoping and borehole discontinuity mapping, can be used for generating preliminary structural models and Discrete Fracture Network models for the areas where the tunnels will be projected. These models will allow planning engineers to evaluate rock fall risk in early stages of the project.

This work sets the bases for more detailed research on application of Laser Scanning and Discrete Element Modeling for hazard recognition and failure analysis in underground stone mines. A series of additional research are proposed from this work:

- 1) A comparison between Stochastic Discrete Element Modeling with Conventional Limit Equilibrium Methods.

- 2) A comparison between Automated Discontinuity Extraction Software versus Manual Virtual discontinuity Mapping.
- 3) Application of Stochastic Discrete Element Modeling for support design in underground excavations.
- 4) Stochastic Discrete Element Modeling extended to structurally and stress controlled underground mines.
- 5) Stochastic Discrete Element Modeling for pillar strength determination and comparison with empirical pillar strength estimation methods.

APPENDIX A. Mapped Structural Data

# Format is <X/E (m)>	<Y/N (m)>	<Z/RL (m)>	<Dip (°)>	<Dip direction (°)>	<Strike (°)>	<Length>	<Area>	SET
-12.834	23.132	2.647	73.481	352.586	262.586	1.42	0.794	2
-12.713	23.665	-0.124	82.978	296.001	206.001	0.844	0.33	
-12.386	24.048	0.064	77.927	345.873	255.873	0.399	0.07	2
-12.53	23.802	0.809	73.735	342.894	252.894	1.084	0.126	2
-20.427	11.803	3.911	77.609	357.703	267.703	1.88	0.263	2
-20.181	12.43	1.757	89.308	84.99	354.99	4.645	1.214	1
-21.225	10.075	3.93	65.352	320.047	230.047	1.842	0.24	
-19.727	13.771	3.661	88.171	257.817	167.817	3.339	1.562	1
-19.767	14.031	3.755	67.801	319.448	229.448	0.954	0.024	
-12.49	23.588	1.509	73.128	351.057	261.057	1.066	0.301	2
-19.703	14.803	3.015	83.882	16.761	286.761	2.615	0.221	3
-19.748	14.394	3.886	83.453	0.507	270.507	1.24	0.096	
-18.299	17.576	1.763	77.315	329.984	239.984	3.025	1.203	
-18.876	16.769	1.793	69.69	349.2	259.2	1.925	0.182	2
-16.148	19.27	1.633	78.894	55.709	325.709	0.963	0.052	
-16.6	18.578	0.979	80.037	328.887	238.887	2.323	1.271	
-10.974	1.427	3.105	85.51	257.409	167.409	5.113	5.92	1
-10.601	2.584	3.075	80.102	285.278	195.278	2.835	1.4	
-9.669	4.338	4.768	88.211	256.065	166.065	2.923	1.237	1
-11.117	0.24	4.103	83.506	248.003	158.003	4.055	1.377	1
-12.611	23.129	4.242	74.352	5.552	275.552	1.467	0.302	
-11.58	-0.374	4.52	85.584	243.164	153.164	3.273	0.938	1
-11.841	-0.603	5.899	29.88	144.958	54.958	0.959	0.186	4
-8.632	5.546	0.868	68.419	265.784	175.784	5.241	1.848	
-9.356	4.711	3.654	87.224	58.344	328.344	2.566	0.509	
-8.009	5.792	3.747	75.265	274.777	184.777	1.585	0.239	
-6.3	9.926	0.766	66.468	349.015	259.015	4.321	1.251	2
-7.309	8.314	3.247	66.476	321.46	231.46	1.066	0.236	
-7.423	7.159	3.139	78.723	265.347	175.347	2.816	1.812	1
-5.963	10.621	4.218	85.477	243.559	153.559	4.278	1.033	1
-4.619	12.29	3.973	79.277	183.787	93.787	0.614	0.103	
-12.634	22.813	5.54	76.476	8.058	278.058	0.788	0.159	3
-4.53	12.56	3.998	89.424	256.908	166.908	1.243	0.329	1
-2.054	14.636	1.431	63.712	280.56	190.56	1.426	0.175	
-1.432	15.321	1.166	57.318	270.417	180.417	1.796	0.302	
-0.994	15.831	-0.383	57.374	290.162	200.162	0.541	0.034	
-2.128	15.304	4.838	68.233	295.333	205.333	2.361	1.341	
-3.424	13.319	3.607	89.24	77.553	347.553	1.014	0.154	1
-2.784	14.224	3.841	72.73	242.993	152.993	0.798	0.089	

-1.412	15.837	3.02	54.836	308.225	218.225	0.803	0.111	
-1.4	15.726	2.435	61.48	300.094	210.094	1.548	0.216	
-2.045	14.782	2.14	55.328	311.883	221.883	0.495	0.07	
-13.501	22.038	4.036	68.383	24.185	294.185	5.505	1.328	3
-3.14	13.459	0.205	58.448	131.734	41.734	2.324	0.313	
-2.485	14.44	-0.205	70.817	271.502	181.502	0.62	0.134	
-1.617	15.152	1.007	72.994	27.999	297.999	1.317	0.141	3
-3.004	14.138	4.625	66.85	342.843	252.843	0.643	0.152	2
-3.121	15.391	6.227	84.942	79.077	349.077	2.241	0.446	1
-2.784	16.289	6.562	50.433	319.762	229.762	0.84	0.081	
-12.186	22.47	6.559	69.686	353.965	263.965	0.869	0.192	2
-12.883	22.587	5.605	83.966	112.885	22.885	0.657	0.165	
-12.741	23.394	1.238	87.746	288.266	198.266	0.733	0.216	
-16.005	19.046	5.306	76.437	1.495	271.495	1.836	0.24	2
-4.796	11.713	2.507	67.824	279.609	189.609	1.457	0.199	
-16.009	5.156	7.546	31.084	147.152	57.152	3.822	2.949	4
-14.452	4.986	7.191	64.809	347.708	257.708	2.629	0.381	2
-15.251	8.016	7.292	76.565	75.927	345.927	0.55	0.092	
-15.504	8.367	7.258	56.449	10.04	280.04	0.355	0.066	
-12.187	7.548	7.402	89.931	256.487	166.487	3.77	1.364	1
-13.667	1.791	7.458	25.568	143.729	53.729	1.951	0.757	4
-13.813	2.436	7.379	41.087	23.579	293.579	0.773	0.26	
-14.106	2.705	7.216	37.747	20.343	290.343	0.598	0.126	
-14.381	2.394	7.351	41.214	322.034	232.034	0.379	0.083	
-13.574	3.154	7.085	58.406	34.968	304.968	0.276	0.021	
-8.224	15.064	7.333	82.929	258.152	168.152	2.721	0.365	1
-13.105	7.982	7.664	74.138	308.2	218.2	0.492	0.109	
-12.919	8.067	7.804	73.199	28.024	298.024	0.205	0.016	3
-13.356	7.738	7.623	65.545	299.17	209.17	0.744	0.121	
-14.299	7.749	7.284	38.981	324.029	234.029	0.361	0.063	
-14.057	7.622	7.356	54.042	52.987	322.987	0.364	0.048	
-13.056	6.078	7.399	88.975	77.31	347.31	1.484	0.103	1
-16.798	9.913	7.413	87.467	257.901	167.901	1.147	0.106	1
-16.135	12.234	7.191	44.762	75.175	345.175	0.802	0.139	
-15.275	11.26	7.257	64.88	342.016	252.016	1.629	0.155	2
-14.457	11.115	7.217	62.062	341.321	251.321	0.615	0.077	2
-18.165	10.84	7.316	65.387	327.853	237.853	0.437	0.041	
-18.666	10.988	7.178	50.566	230.704	140.704	0.239	0.025	
-10.254	19.498	8.116	50.223	260.937	170.937	2.435	1.478	
-8.589	20.5	8.849	59.869	280.698	190.698	1.249	0.422	
-10.011	20.198	8.313	8.615	84.34	354.34	0.693	0.165	
-10.267	20.73	7.976	63.123	334.44	244.44	0.863	0.255	2
-15.375	15.327	7.097	43.992	24.139	294.139	0.693	0.173	

-11.516	18.894	7.974	77.1	331.167	241.167	7.815	3.731	
-7.765	18.975	8.577	89.951	66.119	336.119	1.428	0.703	1
-7.942	20.061	8.669	89.473	260.062	170.062	3.996	2.619	1
-8.102	17.767	8.091	89.537	78.318	348.318	0.635	0.106	1
-5.122	16.778	7.033	64.397	347.612	257.612	1.284	0.173	2
-5.094	18.334	7.491	56.899	321.439	231.439	1.944	0.695	
8.871	46.253	2.991	37.517	144.25	54.25	7.055	2.78	4
16.837	28.633	1.183	31.138	183.354	93.354	2.371	0.564	
16.98	28.448	4.995	89.472	254.424	164.424	0.804	0.167	1
17.304	28.194	4.651	82.46	256.703	166.703	0.589	0.148	1
16.645	28.944	4.861	80.28	54.837	324.837	1.555	0.97	
16.742	30.036	3.487	85.18	251.228	161.228	0.817	0.268	1
16.65	30.042	4.433	28.171	137.6	47.6	0.713	0.133	4
18.817	33.648	4.565	76.17	330.579	240.579	10.114	8.699	
22.976	37.371	0.509	73.311	300.653	210.653	1.867	1.301	
22.742	36.93	3.005	85.2	129.803	39.803	1.489	0.276	
18.485	33.158	1.933	85.71	296.275	206.275	1.862	0.701	
9.891	47.074	1.58	76.365	357.569	267.569	2.449	0.793	2
17.821	32.406	1.316	74.751	275.482	185.482	0.324	0.036	
15.965	31.454	0.237	67.389	272.444	182.444	1.441	0.318	
16.154	29.997	0.937	54.909	313.45	223.45	0.485	0.05	
15.906	31.166	0.249	42.475	331.91	241.91	1.272	0.115	
16.355	30.842	1.285	78.396	263.533	173.533	0.85	0.178	1
19.382	34.222	0.759	85.378	346.035	256.035	3.817	2.802	
26.194	39.909	1.315	66.606	45.559	315.559	1.069	0.105	
26.218	39.968	0.764	76.304	62.489	332.489	0.969	0.116	
17.1	45.503	9.598	54.302	327.291	237.291	1.348	0.24	
15.755	42.868	9.955	54.677	343.676	253.676	2.569	1.21	2
8.413	45.934	5.099	76.752	330.828	240.828	0.904	0.114	
19.953	41.198	8.927	78.774	82.828	352.828	1.919	0.455	1
14.557	38.222	9.716	68.737	346.541	256.541	9.882	3.189	2
8.146	45.739	4.813	67.347	346.999	256.999	0.727	0.052	2
8.075	45.763	1.262	82.495	332.626	242.626	3.373	2.522	
10.978	47.835	5.617	88.919	343.234	253.234	1.87	0.839	
14.313	50.126	6.171	64.999	0.633	270.633	1.499	0.294	2
14.251	50.476	4.843	68.499	345.698	255.698	1.435	0.44	2
16.826	52.453	5.331	55.505	39.802	309.802	3.194	0.365	
-2.802	33.938	4.787	82.229	345.07	255.07	4.866	1.897	
-6.421	29.036	6.735	74.737	273.038	183.038	1.109	0.171	
12.931	24.159	8.08	87.366	76.697	346.697	1.394	0.211	1
12.914	25.68	8.272	87.261	75.172	345.172	1.589	0.442	1
-6.491	28.979	6.617	70.732	344.288	254.288	1.27	0.105	2
-8.605	27.47	1.498	63.703	335.615	245.615	1.58	0.46	2

-8.291	27.853	2.545	60.005	339.055	249.055	0.9	0.289	2
-8.545	27.462	3.22	57.711	340.793	250.793	0.669	0.062	2
-10.176	25.631	2.669	73.362	342.785	252.785	1.731	0.686	2
-10.25	25.645	1.961	73.571	355.375	265.375	0.535	0.108	2
-10.198	25.716	1.556	72.859	357.883	267.883	0.377	0.044	2
-10.188	25.737	1.352	72.039	0.149	270.149	0.36	0.051	2
-10.044	25.53	4.485	73.395	327.214	237.214	0.743	0.181	
1.981	38.639	6.1	65.313	343.366	253.366	1.497	0.477	2
-9.696	25.999	4.147	68.122	326.602	236.602	1.176	0.353	
-6.568	30.055	2.023	73.635	348.926	258.926	2.675	0.929	2
-6.616	30.057	3.6	71.086	340.965	250.965	1.501	0.363	2
-6.632	30.063	4.162	32.928	140.963	50.963	2.082	0.32	4
-4.04	32.873	3.847	35.824	140.302	50.302	7.147	1.161	4
2.074	39.107	5.135	74.656	347.055	257.055	2.19	0.566	2
-0.123	36.158	6.532	85.361	50.194	320.194	3.452	1.139	
-2.761	33.962	4.781	80.791	330.85	240.85	5.695	3.825	
1	38.666	0.526	78.109	334.157	244.157	3.194	1.108	
-1.575	36.072	1.581	30.494	144.597	54.597	7.566	2.041	4
-0.256	36.362	6.059	87.716	257.601	167.601	2.511	0.686	1
-0.57	36.926	2.059	32.348	140.716	50.716	1.404	0.24	4
1.954	38.612	6.155	65.751	345.658	255.658	1.476	0.469	2
2.474	39.232	6.072	85.996	258.789	168.789	2.557	0.526	1
1.084	37.228	7.145	86.525	160.083	70.083	0.897	0.278	
3.04	18.918	1.952	82.423	31.838	301.838	1.893	0.694	3
2.357	19.079	3.945	79.383	29.767	299.767	1.125	0.2	3
1.415	19.205	5.195	88.025	188.532	98.532	1.591	0.1	
1.025	18.815	4.897	77.925	23.635	293.635	1.027	0.106	3
1.481	18.678	3.79	83.581	203.762	113.762	0.57	0.09	
1.609	18.602	3.38	86.216	202.252	112.252	0.231	0.015	
2.492	39.209	6.191	86.945	256.955	166.955	2.602	0.492	1
3.603	18.811	1.702	78.035	27.591	297.591	0.832	0.184	3
-0.952	17.283	5.407	62.648	330.025	240.025	1.281	0.168	
-1.228	16.635	4.674	71.203	341.997	251.997	0.485	0.078	2
0.764	18.547	4.579	51.87	346.263	256.263	0.809	0.057	
0.042	18.024	5.029	63.971	323.65	233.65	0.398	0.047	
2.985	20.026	5.473	56.027	352.521	262.521	1.293	0.232	2
-0.341	16.416	1.396	61.489	306.221	216.221	1.494	0.318	
5.802	17.914	0.479	72.367	32.439	302.439	2.009	0.335	3
5.095	18.132	1.691	63.245	333.273	243.273	0.713	0.073	2
4.733	18.698	0.654	66.812	332.657	242.657	0.369	0.032	2
0.855	38.555	0.751	79.684	332.94	242.94	2.377	1.054	
4.708	18.57	0.943	62.624	335.374	245.374	0.373	0.022	2
4.115	18.866	0.913	72.979	347.073	257.073	1.107	0.272	2

4.657	19.027	-0.051	62.632	325.066	235.066	1.015	0.098	
4.427	18.983	0.016	71.478	39.493	309.493	0.387	0.013	
4.308	19.123	-0.246	76.43	44.932	314.932	0.335	0.009	
3.614	19.806	4.839	89.581	229.9	139.9	0.643	0.08	
3.371	19.915	4.917	46.598	150.611	60.611	0.573	0.052	
3.068	19.892	4.9	44.761	133.266	43.266	0.723	0.105	
6.668	17.655	5.381	82.004	27.709	297.709	1.263	0.225	3
4.83	18.815	4.605	70.818	15.879	285.879	0.708	0.068	3
-0.855	36.718	1.645	30.499	144.577	54.577	7.428	1.721	4
4.977	19.075	5.188	80.386	157.082	67.082	0.639	0.11	
5.253	19.114	5.422	62.808	13.766	283.766	0.625	0.112	3
5.654	19.518	6.344	83.96	9.525	279.525	1.238	0.456	3
6.265	19.271	6.17	58.538	225.159	135.159	0.881	0.345	
4.652	19.6	6.109	66.352	3.044	273.044	0.313	0.031	2
4.224	19.568	5.618	66.206	147.021	57.021	0.265	0.016	
2.404	20.087	5.691	68.6	356.413	266.413	0.466	0.069	2
5.758	17.752	3.812	78.818	330.948	240.948	1.66	0.116	
6.089	17.29	2.279	74.444	25.442	295.442	1.636	0.547	3
5.273	18.101	1.412	63.246	347.663	257.663	0.432	0.027	2
-6.245	29.675	6.042	73.812	342.623	252.623	2.431	0.773	2
4.16	19.035	3.613	78.992	221.764	131.764	2.339	1.789	
3.104	19.125	3.434	73.941	341.86	251.86	1.429	0.317	2
0.385	17.915	3.673	80.024	281.388	191.388	0.561	0.073	
0.929	18.933	5.327	89.321	123.716	33.716	1.027	0.177	
5.276	20.715	8.095	79.606	18.628	288.628	3.27	2.978	3
5.173	20.748	8.086	80.555	17.777	287.777	4.084	3.894	3
2.328	21.572	8.033	82.425	16.527	286.527	0.801	0.108	3
-4.813	21.527	8.725	85.949	255.343	165.343	4.123	1.615	1
-3.605	23.864	8.637	76.861	29.55	299.55	1.053	0.188	3
0.104	25.152	8.92	60.933	330.696	240.696	1.395	0.363	
-6.295	29.341	6.624	72.144	276.109	186.109	1.287	0.309	
-1.744	24.13	8.693	61.553	333.19	243.19	2.376	0.445	2
-4.27	22.943	8.92	67.226	334.254	244.254	2.829	0.706	2
-6.088	22.234	8.905	76.364	7.969	277.969	0.592	0.024	3
-6.277	22.543	8.864	74.589	352.503	262.503	0.559	0.097	2
-7.893	23.444	8.84	78.355	259.983	169.983	0.722	0.096	1
-7.352	23.774	9.041	77.654	261.208	171.208	1.171	0.428	1
2.894	27.661	9.084	66.456	345.851	255.851	5.42	1.729	2
10.437	27.315	9.158	64.878	349.391	259.391	5.062	1.529	2
12.05	28.289	8.819	28.692	143.384	53.384	7.943	17.577	4
7.693	26.27	9.291	65.881	278.567	188.567	2.277	0.562	
-6.347	29.159	6.64	74.698	358.442	268.442	1.045	0.11	2
9.186	24.249	8.945	86.584	75.266	345.266	3.066	1.195	1

-2.976	25.473	8.493	86.195	218.234	128.234	3.763	0.696	
2.707	38.329	8.585	86.492	261.966	171.966	2.452	1.135	1
1.951	35.965	9.189	80.075	318.853	228.853	2.004	0.396	
1.791	37.544	7.995	81.504	263.016	173.016	1.4	0.201	1
1.972	37.626	8.233	63.353	351.589	261.589	1.621	0.309	2
2.607	35.571	9.503	65.001	248.518	158.518	1.185	0.199	
2.432	31.258	9.443	76.751	172.972	82.972	2.4	0.963	
1.51	32.124	9.349	69.922	251.691	161.691	1.748	0.624	
8.979	22.704	8.507	71.092	14.865	284.865	2.807	0.585	3
-23.279	6.282	1.174	64.961	2.199	272.199	2.607	0.432	2
-25.239	1.205	5.378	69.783	358.081	268.081	1.191	0.264	2
-18.174	-5.615	6.951	42.292	354.926	264.926	1.049	0.225	
-21.308	-9.427	6.934	54.119	305.381	215.381	0.862	0.15	
-21.016	-8.889	6.817	50.319	317.288	227.288	0.427	0.049	
-21.867	-9.976	6.937	49.18	321.764	231.764	1.207	0.156	
-25.136	-10.382	7.025	52.417	324.397	234.397	0.691	0.076	
-15.875	-3.534	7.225	54.046	266.359	176.359	0.911	0.134	
-15.626	-2.569	7.258	39.648	340.021	250.021	1.448	0.398	
-26.085	0.634	4.578	69.644	359.742	269.742	2.496	0.46	2
-22.295	8.337	2.709	68.842	327.295	237.295	1.253	0.276	
-22.286	8.364	1.916	78.232	330.249	240.249	1.28	0.299	
-23.829	5.333	1.279	70.744	81.337	351.337	0.937	0.143	
-23.657	5.565	0.559	64.175	80.537	350.537	0.232	0.018	
-23.721	5.353	0.566	72.374	75.393	345.393	0.38	0.05	
-25.272	2.167	2.888	80.668	2.708	272.708	1.056	0.054	
-24.64	2.543	2.349	56.539	67.191	337.191	0.717	0.155	
-24.57	2.844	3.534	84.545	18.289	288.289	0.708	0.042	3
-22.858	7.294	0.746	51.535	317.406	227.406	0.854	0.133	
-24.765	2.404	1.97	49.274	303.538	213.538	0.406	0.083	
-26.467	0.837	1.498	89.406	196.927	106.927	1.857	0.51	
-26.349	0.82	2.767	82.144	10.522	280.522	0.746	0.085	3
-27.749	0.436	1.707	85.392	313.322	223.322	0.246	0.011	
-26.948	0.801	2.314	85.171	3.026	273.026	0.476	0.099	
-26.79	0.335	0.486	78.991	99.961	9.961	0.527	0.092	
-26.485	-0.262	5.602	67.732	16.46	286.46	0.49	0.054	3
-26.562	-0.014	5.263	71.235	17.221	287.221	0.371	0.062	3
-26.167	-0.088	5.477	73.571	9.698	279.698	0.195	0.014	3
-27.148	-0.45	5.752	60.814	18.643	288.643	0.704	0.288	3
-25.098	2.364	1.501	86.55	30.655	300.655	2.849	0.898	3
-27.113	-0.037	5.128	70.054	348.79	258.79	0.929	0.427	2
-27.291	0.619	2.03	80.544	311.787	221.787	1.353	0.301	
-25.51	1.881	2.981	73.606	8.816	278.816	0.404	0.05	3
-25.693	1.679	2.092	82.954	16.856	286.856	0.699	0.022	3

-25.496	1.947	1.773	70.437	348.364	258.364	0.565	0.026	2
-25.525	1.982	2.436	67.781	275.544	185.544	0.597	0.069	
-25.406	1.961	2.991	71.113	267.474	177.474	0.373	0.045	
-23.688	4.883	3.189	76.902	335.178	245.178	0.774	0.161	
-24.699	1.721	5.949	89.165	39.288	309.288	0.951	0.063	
-24.633	1.969	6.18	87.381	39.879	309.879	0.406	0.025	
-23.902	4.269	5.303	55.857	137.649	47.649	0.873	0.13	
-24.634	1.537	6.216	84.623	29.634	299.634	0.417	0.017	3
-24.979	1.897	5.088	77.266	21.878	291.878	0.329	0.018	3
-24.821	2.415	4.16	76.51	350.101	260.101	0.543	0.083	2
-13.149	-5.68	2.804	82.436	261.46	171.46	3.585	2.899	1
-12.937	-4.256	4.776	84.801	259.744	169.744	2.368	1.382	1
-12.264	-2.802	3.306	77.864	261.499	171.499	2.454	1.892	1
-12.437	-2.961	2.174	74.32	266.762	176.762	1.605	0.889	
-12.113	-1.849	2.768	85.987	255.39	165.39	1.113	0.282	1
-12.37	-2.898	4.427	80.048	270.414	180.414	1.104	0.55	
-12.519	-3.025	5.346	73.86	280.437	190.437	1.315	0.33	
-23.882	4.297	4.939	47.062	147.801	57.801	0.134	0.007	
-12.795	-3.322	6.365	82.874	252.482	162.482	1.021	0.316	1
-12.684	-2.852	6.257	87.443	305.806	215.806	0.274	0.034	
-13.17	-5.251	5.499	89.235	258.301	168.301	1.586	0.413	1
-13.209	-6.039	3.691	78.396	279.264	189.264	1.528	0.21	
-13.811	-8.433	2.396	81.004	241.725	151.725	4.662	3.793	
-13.964	-9.598	1.51	63.038	246.449	156.449	1.985	0.552	
-14.023	-9.93	2.048	73.659	262.519	172.519	1.356	0.218	
-14.316	-10.562	2.521	84.694	248.167	158.167	2.571	0.767	1
-13.819	-7.343	0.97	81.653	51.694	321.694	0.634	0.099	
-13.953	-7.701	1.508	86.929	57.633	327.633	0.663	0.068	
-24.026	4.11	4.863	50.065	146.08	56.08	0.138	0.006	
-14.004	-7.856	1.515	67.524	345.479	255.479	0.474	0.048	2
-13.964	-7.624	1.031	67.58	348.039	258.039	0.426	0.023	2
-14.055	-8.052	1.411	84.842	257.057	167.057	0.826	0.143	1
-13.707	-9.544	2.098	84.294	244.378	154.378	0.566	0.064	1
-13.872	-9.676	2.939	51.442	62.312	332.312	0.513	0.085	
-13.47	-6.096	0.02	78.99	259.366	169.366	1.519	0.897	1
-14.092	-7.474	-0.001	82.759	244.267	154.267	1.066	0.328	1
-12.104	-2.004	1.121	81.409	250.12	160.12	1.319	0.278	1
-12.454	-2.773	0.766	78.39	263.399	173.399	0.952	0.396	1
-13.236	-5.554	0.925	81.708	249.425	159.425	1.282	0.419	1
-23.745	4.5	5.1	47.079	140.097	50.097	0.916	0.098	
-14.192	-8.76	5.15	80.453	256.038	166.038	2.814	1.259	1
-14.236	-9.69	5.108	80.843	254.567	164.567	0.668	0.162	1
-12.874	-4.044	1.205	77.191	79.544	349.544	0.539	0.109	1

-13.172	-4.766	0.128	80.428	259.147	169.147	1.315	0.472	1
-12.482	-2.423	6.019	87.055	270.348	180.348	0.893	0.479	1
-12.579	-3.066	5.637	75.066	284.708	194.708	0.785	0.189	
-13.237	-5.57	5.625	69.828	345.824	255.824	0.835	0.102	2
-28.605	-5.114	7.391	27.985	146.124	56.124	12.274	28.811	4
-34.604	-5.002	8.203	82.623	259.062	169.062	1.503	0.346	1
-22.236	0.913	7.071	27.997	133.593	43.593	4.283	2.536	4
-23.232	6.131	4.554	42.633	123.804	33.804	0.442	0.065	
-22.631	1.415	7.235	61.139	307.465	217.465	0.437	0.105	
-22.258	-0.459	7.069	87.16	44.14	314.14	2.192	0.657	
-20.946	3.539	7.285	61.568	344.774	254.774	5.223	2.099	2
-22.531	4.099	6.929	67.907	347.943	257.943	2.796	0.981	2
-18.992	4.551	7.561	66.075	346.807	256.807	3.426	1.324	2
-20.149	1.308	7.283	22.333	140.075	50.075	6.865	9.118	4
-28.621	-10.961	7.116	26.301	146.612	56.612	3.822	3.589	4
-21.398	-6.343	6.751	25.071	137.96	47.96	5.369	5.887	4
-18.922	-3.764	6.997	24.055	153.144	63.144	1.917	1.366	4
-17.042	-1.954	7.39	29.263	139.597	49.597	1.652	0.799	4
-24.211	3.781	4.79	55.876	107.644	17.644	0.369	0.031	
-16.279	-0.278	7.311	39.247	27.514	297.514	1.269	0.363	
-16.279	-0.278	7.311	39.247	27.514	297.514	1.269	0.363	
-17.172	-0.864	7.243	36.325	25.436	295.436	1.21	0.508	
-19.395	-0.196	7.143	70.753	354.049	264.049	1.92	0.472	2
-19.316	0.346	6.975	71.249	328.831	238.831	0.511	0.074	
-18.83	0.919	7.068	52.651	353.413	263.413	1.129	0.144	
-19.014	1.313	7.095	61.736	356.958	266.958	0.941	0.211	2
-23.119	-13.124	7.069	64.792	311.602	221.602	2.045	0.524	
-20.025	-11.566	7.045	49.096	327.76	237.76	0.875	0.258	
-19.819	-12.158	7.208	46.931	9.221	279.221	1.095	0.258	
-27.133	-19.41	1.997	86.652	55.145	325.145	4.038	8.218	
-33.321	-14.107	5.125	83.267	45.91	315.91	2.225	1.822	
-31.951	-14.743	2.52	81.998	40.612	310.612	7.734	16.477	
-31.662	-15.386	3.917	36.255	79.651	349.651	1.824	0.191	
-31.837	-15.437	5.141	32.839	186.071	96.071	1.373	0.273	
-31.419	-11.201	7.271	89.007	55.702	325.702	3.939	1.272	
-29.61	-17.823	7.471	78.398	41.848	311.848	10.764	11.111	
-36.757	-7.036	8.147	86.326	261.676	171.676	2.485	1.711	1
-34.935	-8.283	7.748	21.666	147.615	57.615	4.175	8.603	4
-0.846	-23.535	0.105	67.714	278.686	188.686	2.472	0.561	
-1.545	-24.418	3.424	72.075	263.093	173.093	0.679	0.078	
-13.255	-14.966	-0.263	74.258	342.398	252.398	0.983	0.303	2
-13.337	-14.326	4.933	66.944	356.768	266.768	1.193	0.261	2
-13.741	-14.314	5.268	76.578	243.838	153.838	0.326	0.032	

-14.426	-12.447	1.383	75.872	330.968	240.968	1.159	0.094	
-25.539	-24.625	0.795	71.667	345.088	255.088	1.641	0.301	2
-26.12	-25.235	3.381	68.154	349.714	259.714	4.126	2.342	2
-25.036	-22.916	5.724	74.067	29.333	299.333	0.496	0.108	3
-24.951	-24.041	0.817	82.103	265.658	175.658	0.503	0.088	1
-24.913	-24.25	0.377	65.336	105.138	15.138	0.336	0.035	
-26.329	-25.565	1.708	83.014	54.167	324.167	1.32	0.309	
-2.322	-24.971	4.44	61.032	303.145	213.145	1.242	0.357	
-25.658	-24.714	2.961	70.284	339.556	249.556	0.711	0.11	2
-25.008	-24.554	-0.139	28.28	144.759	54.759	0.949	0.297	4
-24.916	-24.137	4.208	63.194	48.94	318.94	0.523	0.057	
-25.183	-22.883	4.484	78.977	56.697	326.697	0.725	0.125	
-25.409	-24.772	4.171	73.063	346.27	256.27	1.367	0.436	2
-25.32	-25.366	5.327	61.987	344.13	254.13	0.918	0.151	2
-25.984	-25.418	4.597	66.312	329.533	239.533	0.743	0.158	
-25.368	-24.439	0.949	81.642	108.869	18.869	0.523	0.085	
-25.856	-25.082	1.019	86.504	72.406	342.406	0.4	0.039	1
-25.043	-24	3.274	70.949	70.133	340.133	0.636	0.131	
-0.96	-23.55	4.269	69.988	271.017	181.017	0.337	0.036	
-11.277	-14.876	6.101	64.915	349.716	259.716	4.098	1.767	2
-8.612	-21.244	6.315	68.679	350.15	260.15	4.046	1.183	2
-11.39	-21.146	6.294	57.961	346.005	256.005	1.72	0.351	2
-11.895	-21.79	6.279	86.591	63.465	333.465	0.977	0.076	1
-12.177	-22.078	6.257	71.176	355.347	265.347	0.828	0.128	2
-13.133	-19.196	6.398	85.439	72.524	342.524	0.905	0.103	1
-10.915	-24.778	6.3	64.61	351.941	261.941	5.944	1.29	2
-7.925	-18.891	6.468	88.012	37.44	307.44	0.859	0.086	
-9.234	-18.694	6.451	72.305	95.389	5.389	1.064	0.242	
-9.42	-17.949	6.514	86.087	40.654	310.654	0.854	0.163	
-1.803	-24.506	4.013	65.146	276.705	186.705	0.55	0.087	
-9.553	-19.288	6.508	81.585	87.601	357.601	0.63	0.105	
-8.636	-19.395	6.411	87.898	63.645	333.645	0.83	0.109	1
-11.048	-20.142	6.317	86.647	246.379	156.379	0.446	0.054	1
-11.213	-20.222	6.523	24.266	235.781	145.781	0.265	0.038	
-11.337	-20.097	6.404	51.368	301.598	211.598	0.413	0.069	
-11.133	-20.172	6.456	82.946	69.012	339.012	0.391	0.051	1
-17.019	-23.886	6.109	50.388	35.779	305.779	1.004	0.103	
-15.778	-23.324	6.227	80.34	25.185	295.185	2.211	0.199	3
-16.117	-24.789	6.172	53.628	9.614	279.614	1.171	0.189	
-5.66	-23.67	6.038	48.578	303.543	213.543	0.431	0.054	
-1.518	-24.076	4.346	54.13	283.597	193.597	0.5	0.051	
-6.507	-23.558	6.136	49.363	306.713	216.713	1.303	0.083	
-7.35	-23.562	6.227	54.722	305.96	215.96	1.205	0.104	

-4.402	-23.786	5.914	84.545	44.364	314.364	2.155	0.354	
-4.043	-25.805	5.752	89.151	251.293	161.293	0.894	0.267	1
-5.562	-25.346	5.987	50.446	22.391	292.391	1.169	0.268	
-4.999	-24.604	5.864	38.081	12.783	282.783	0.978	0.196	
-14.501	-21.321	6.427	65.532	31.654	301.654	1.104	0.135	3
-14.525	-21.655	6.386	38.427	332.927	242.927	0.447	0.058	
-14.073	-21.144	6.334	42.784	328.948	238.948	0.494	0.087	
-13.969	-21.605	6.318	60.942	103.89	13.89	0.767	0.061	
-1.451	-24.142	3.887	53.249	289.472	199.472	0.822	0.116	
-13.392	-21.35	6.26	61.794	98.699	8.699	0.393	0.025	
-13.101	-22.883	6.199	67.167	352.352	262.352	0.626	0.062	2
-12.306	-22.945	6.359	52.404	13.98	283.98	0.419	0.104	
-12.117	-22.842	6.28	65.221	10.252	280.252	0.453	0.058	3
-12.519	-22.614	6.217	66.134	11.353	281.353	0.595	0.049	3
-14.017	-24.027	6.149	20.687	146.378	56.378	1.43	0.332	4
-13.856	-18.167	6.437	71.729	33.84	303.84	0.776	0.102	3
-13.572	-18.576	6.417	70.229	30.468	300.468	0.961	0.161	3
-13.71	-18.655	6.488	65.041	29.987	299.987	0.97	0.126	3
-14.994	-18.014	6.556	64.978	22.793	292.793	0.299	0.02	3
-1.324	-24.185	0.876	88.157	170.516	80.516	0.646	0.112	
-15.106	-18.496	6.592	41.375	3.75	273.75	0.381	0.033	
-15.748	-18.815	6.626	50.868	332.559	242.559	0.545	0.038	
-15.992	-19.232	6.581	24.624	156.49	66.49	1.066	0.18	4
-16.006	-17.714	6.786	66.949	17.418	287.418	0.674	0.079	3
-24.777	-23.208	6.231	75.235	27.247	297.247	1.15	0.34	3
-14.782	-27.945	5.976	48.375	359.028	269.028	1.208	0.199	
-17.114	-27.337	5.923	65.692	325.885	235.885	2.558	0.415	
-18.708	-25.977	6.225	67.105	352.796	262.796	5.06	1.95	2
-24.001	-24.124	6.163	88.36	254.464	164.464	0.924	0.212	1
-23.258	-25.212	6.287	58.683	244.693	154.693	1.369	0.139	
-1.542	-24.328	1.037	89.424	169.803	79.803	0.303	0.041	
-22.468	-27.123	6.445	79.638	71.227	341.227	1.491	0.184	1
-22.463	-28.076	6.302	79.717	232.025	142.025	0.747	0.119	
-20.616	-29.797	6.263	86.77	65.963	335.963	3.977	0.821	1
-19.75	-26.692	6.132	35.126	235.506	145.506	0.817	0.136	
-19.566	-23.77	6.317	58.736	276.15	186.15	1.905	0.22	
-18.632	-24.735	6.251	66.804	290.059	200.059	0.527	0.054	
-16.547	-21.921	6.423	58.193	51.269	321.269	1.144	0.061	
-16.085	-21.637	6.495	45.88	295.509	205.509	0.27	0.018	
-16.098	-21.459	6.51	68.234	343.652	253.652	1.146	0.08	2
-14.228	-30.785	5.663	84.878	248.464	158.464	0.976	0.097	1
-1.724	-24.544	0.658	80.277	255.403	165.403	0.561	0.048	1
-14.918	-31.826	5.73	35.984	356.712	266.712	0.813	0.235	

-16.669	-29.821	5.902	52.983	321.152	231.152	0.656	0.085	
-16.625	-30.801	5.967	65.541	322.815	232.815	0.373	0.029	
-17.632	-29.48	5.938	25.845	14.278	284.278	0.487	0.076	
-1.556	-24.335	0.469	80.175	249.516	159.516	0.499	0.026	1
-0.573	-23.117	-0.444	69.345	277.947	187.947	0.961	0.139	
-0.433	-23.351	2.005	84.121	242.758	152.758	0.423	0.039	1
-0.09	-23.095	0.993	81.842	250.159	160.159	1.032	0.127	1
-1.151	-24.029	3.308	56.26	266.262	176.262	1.513	0.194	
-3.18	-25.76	2.643	56.955	281.212	191.212	1.26	0.187	
-4.282	-26.502	2.992	83.495	247.075	157.075	4.166	0.835	1
-11.434	-30.937	3.048	87.521	79.849	349.849	3.17	0.984	1
-4.488	-26.62	3.524	76.323	260.306	170.306	1.47	0.16	1
-4.926	-26.886	2.277	60.503	247.795	157.795	0.573	0.046	
-4.73	-26.789	3.491	71.856	249.488	159.488	0.771	0.071	
-5.146	-26.987	3.556	89.402	189.019	99.019	0.896	0.174	
-1.187	-23.9	-0.017	67.197	267.051	177.051	1.511	0.266	
-3.919	-26.204	2.841	59.47	269.245	179.245	0.767	0.112	
-4.092	-26.385	2.656	61.051	268.116	178.116	0.724	0.079	
-3.974	-26.259	3.181	55.679	18.345	288.345	0.405	0.033	
-3.008	-25.596	2.105	67.723	279.994	189.994	0.525	0.034	
-3.02	-25.578	4.12	64.605	249.36	159.36	0.36	0.026	
-6.115	-27.594	2.939	63.446	270.231	180.231	0.933	0.146	
-5.682	-27.467	3.975	63.06	272.679	182.679	0.638	0.096	
-2.597	-25.351	2.178	86.813	59.533	329.533	3.893	0.572	
-10.581	-30.077	0.813	64.218	272.33	182.33	1.278	0.19	
-10.79	-30.313	0.916	64.955	273.738	183.738	1.703	0.24	
-1.961	-24.964	1.083	70.025	267.064	177.064	2.105	0.612	
-11.083	-30.584	0.613	68.966	278.971	188.971	0.891	0.109	
-10.705	-30.25	1.131	66.309	350.691	260.691	1.593	0.266	2
-11.151	-30.7	1.033	62.243	278.996	188.996	0.786	0.11	
-11.288	-30.81	1.159	70.308	282.96	192.96	1.015	0.157	
-10.656	-30.101	2.229	76.885	286.071	196.071	1.092	0.094	
-11.469	-31.054	2.911	70.942	284.686	194.686	1.694	0.515	
-12.311	-31.731	3.695	64.42	350.266	260.266	1.681	0.376	2
-12.904	-32.116	3.986	63.186	297.131	207.131	0.828	0.098	
-13.134	-32.289	2.075	89.57	256.837	166.837	3.101	0.529	1
-12.986	-32.16	2.645	86.121	264.47	174.47	1.248	0.212	1
-1.249	-24.115	1.915	63.901	262.545	172.545	0.54	0.098	
-13.05	-32.135	1.575	78.182	279.771	189.771	0.701	0.095	
-13.577	-32.539	0.79	80.302	267.798	177.798	1.189	0.143	1
-15.107	-34.235	0.711	78.311	283.175	193.175	2.453	0.639	
-14.479	-33.618	1.14	80.229	289.107	199.107	1.91	0.619	
-14.133	-33.131	2.339	88.344	281.302	191.302	2.102	0.609	

-12.639	-31.834	1.367	88.557	277.943	187.943	0.919	0.177	
-12.041	-31.316	-0.305	66.473	282.592	192.592	0.535	0.073	
-12.422	-31.53	-0.444	65.517	291.11	201.11	1.074	0.132	
-11.881	-31.089	-0.401	75.463	268.187	178.187	1.167	0.199	
-9.357	-29.157	1.083	88.906	249.295	159.295	1.108	0.12	1
-0.901	-23.77	1.724	77.482	235.585	145.585	0.517	0.032	
-9.866	-29.527	1.076	69.204	284.412	194.412	1.421	0.163	
-9.31	-29.291	2.507	79.584	279.797	189.797	0.616	0.07	
-9.903	-29.545	1.918	64.51	259.552	169.552	0.612	0.048	
-9.423	-29.403	2.719	64.431	298.157	208.157	1.261	0.132	
-9.915	-29.584	2.487	66.999	266.789	176.789	0.345	0.025	
-8.381	-28.587	-0.355	25.386	151.27	61.27	0.681	0.102	4
-9.24	-29.112	0.198	39.408	148.156	58.156	0.746	0.038	4
-11.566	-30.822	-0.402	66.368	284.491	194.491	0.89	0.094	
-6.594	-27.947	-0.348	79.831	31.011	301.011	0.796	0.16	3
-6.359	-28.003	0.472	76.619	16.924	286.924	0.411	0.041	3
-1.127	-24.014	1.176	69.09	355.118	265.118	1.021	0.079	2
-6.831	-27.914	-0.42	65.962	269.013	179.013	0.546	0.04	
-6.514	-28.054	0.423	71.733	275.568	185.568	0.429	0.021	
-6.766	-28.055	0.141	71.215	218.044	128.044	0.361	0.014	
-6.705	-28.106	0.521	72.564	202.726	112.726	0.264	0.014	
-6.914	-28.148	0.81	58.762	274.599	184.599	0.25	0.01	
-7.553	-28.137	0.046	71.979	235.846	145.846	0.791	0.048	
-7.973	-28.307	0.233	78.522	13.509	283.509	0.724	0.13	3
-7.94	-28.473	1.16	67.511	328.916	238.916	1.008	0.463	
-7.883	-28.56	2.298	69.095	256.816	166.816	1.38	0.235	
-6.608	-27.889	1.476	66.493	257.327	167.327	0.509	0.047	
-2.069	-24.765	4.063	74.957	17.479	287.479	0.572	0.059	3
-7.023	-28.029	2.351	69.461	255.672	165.672	0.506	0.064	
-8.191	-28.764	4.041	66.735	294.484	204.484	0.843	0.106	
-8.18	-28.669	4.355	64.761	282.844	192.844	0.76	0.085	
-7.97	-28.467	4.714	60.424	301.388	211.388	0.763	0.069	
-14.9	-34.061	2.976	63.46	295.278	205.278	1.029	0.168	
-14.858	-33.85	3.695	71.8	292.438	202.438	0.766	0.201	
-12.774	-14.332	3.687	49.624	241.33	151.33	2.295	0.879	
-13.747	-14.497	1.628	62.542	247.976	157.976	1.992	1.22	
-14.116	-12.347	3.308	88.209	65.994	335.994	2.002	0.791	1
-10.674	-13.768	3.528	65.104	345.424	255.424	1.453	0.842	2
-1.719	-24.395	3.91	71.854	178.783	88.783	0.403	0.054	
-11.055	-13.987	3.043	81.851	25.372	295.372	1.341	0.337	3
-14.406	-12.215	4.552	87.135	40.016	310.016	1.558	0.436	
-13.313	-14.937	0.207	67.929	332.295	242.295	2.194	0.711	2
-12.886	-15.033	-0.305	85.086	236.841	146.841	0.708	0.113	

-12.697	-14.835	0.595	85.889	238.01	148.01	0.974	0.141	
-12.4	-14.784	1.63	89.522	49.444	319.444	1.24	0.225	
-11.606	-14.449	0.381	82.399	224.75	134.75	0.736	0.121	
-11.811	-14.545	1.242	74.933	232.516	142.516	0.681	0.086	
-12.775	-14.51	2.813	83.25	241.816	151.816	2.8	0.724	1
-11.896	-14.567	5.251	64.655	351.056	261.056	3.76	6.025	2
17.241	-13.409	1.584	89.066	259.469	169.469	2.337	0.385	1
16.056	-14.304	-0.633	60.21	92.597	2.597	0.497	0.042	
-4.747	-15.524	6.504	67.956	147.057	57.057	1.397	0.177	
-1.812	-19.981	5.808	54.87	327.883	237.883	0.592	0.071	
-3.43	-19.338	6.291	88.353	253.709	163.709	1.621	0.427	1
-3.441	-19.073	6.326	64.929	19.781	289.781	0.788	0.189	3
-3.999	-18.753	6.528	63.781	23.445	293.445	1.118	0.299	3
-3.233	-19.479	6.08	57.806	61.535	331.535	0.524	0.127	
-3.111	-19.867	6.043	72.636	21.875	291.875	0.926	0.15	3
-2.409	-19.274	5.886	49.376	336.489	246.489	1.639	0.265	
-2.564	-18.719	6.054	48.958	94.284	4.284	1.587	0.305	
8.618	-8.976	5.792	46.916	8.08	278.08	2.425	0.383	
16.948	-13.708	0.793	81.724	78.736	348.736	0.368	0.044	1
9.283	-8.869	5.697	89.611	73.45	343.45	1.413	0.156	1
13.127	-8.613	5.415	56.082	80.038	350.038	2.022	0.324	
6.974	-4.632	6.051	63.107	352.029	262.029	3.795	1.213	2
13.57	-14.002	5.045	26.909	139.422	49.422	5.561	4.266	4
15.67	-12.783	5.049	41.708	320.928	230.928	1.226	0.369	
16.764	-12.377	5.183	34.388	27.433	297.433	0.512	0.046	
16.39	-12.401	5.143	33.549	295.215	205.215	0.399	0.084	
17.201	-11.939	5.115	78.78	68.931	338.931	1.471	0.188	1
16.62	-10.898	5.139	49.847	345.597	255.597	0.313	0.034	
16.242	-11.187	5.146	52.649	304.113	214.113	0.439	0.034	
16.957	-13.786	0.568	48.467	297.869	207.869	0.614	0.049	
16.171	-10.66	5.073	45.552	342.906	252.906	0.367	0.057	
15.71	-11.378	5.1	49.898	339.164	249.164	0.302	0.023	
15.944	-11.325	5.124	48.313	317.9	227.9	0.182	0.019	
11.906	-8.036	5.556	67.325	252.946	162.946	5.15	0.94	
6.185	-7.335	6.098	26.686	138.15	48.15	3.235	2.287	4
13.223	-7.424	5.559	53.377	273.007	183.007	1.097	0.134	
17.68	-13.247	1.071	45.146	316.34	226.34	0.536	0.034	
16.268	-14.158	-0.215	55.057	297.874	207.874	1.016	0.097	
15.527	-14.616	-0.409	88.658	281.246	191.246	2.128	0.392	
16.243	-13.81	2.123	86.546	78.685	348.685	1.336	0.167	1
16.336	-13.577	2.997	61.902	299.848	209.848	0.668	0.113	
17.192	-13.027	4.361	56.639	308.654	218.654	0.59	0.137	
17.458	-12.818	3.979	47.512	334.205	244.205	0.353	0.032	

17.615	-13.501	-0.228	88.767	80.86	350.86	1.331	0.117	1
16.435	-13.429	4.304	48.576	330.778	240.778	0.72	0.062	
11.403	-16.348	2.241	50.768	281.553	191.553	0.627	0.111	
11.126	-16.534	1.386	49.137	280.032	190.032	0.62	0.087	
11.661	-16.388	-0.713	50.193	233.873	143.873	0.659	0.046	
11.982	-16.059	2.853	89.596	260.275	170.275	0.722	0.062	1
11.308	-16.412	3.11	36.739	282.126	192.126	0.817	0.087	
11.376	-16.425	2.744	54.568	345.303	255.303	0.395	0.033	2
10.591	-16.79	2.566	88.804	258.361	168.361	0.881	0.063	1
11.214	-16.548	-0.601	54.793	277.508	187.508	0.746	0.075	
9.867	-17.091	0.428	49.152	294.883	204.883	0.457	0.049	
17.266	-13.712	-0.132	42.382	67.919	337.919	0.355	0.035	
5.066	-20.243	0.171	89.821	225.456	135.456	1.371	0.295	
5.895	-19.251	-0.005	75.364	209.461	119.461	0.875	0.104	
5.796	-19.303	-0.677	81.622	29.902	299.902	0.311	0.015	3
5.957	-19.283	-0.576	83.774	22.308	292.308	0.351	0.012	3
5.606	-19.412	-0.331	77.291	217.923	127.923	0.328	0.02	
5.513	-19.497	0.086	75.783	243.159	153.159	0.21	0.01	
0.592	-22.685	2.113	70.643	358.953	268.953	0.507	0.092	2
0.987	-22.575	2.171	78.224	3.216	273.216	0.648	0.142	
15.114	-14.784	0.981	87.189	76.159	346.159	0.89	0.085	1
15.788	-14.158	1.773	84.734	86.064	356.064	0.733	0.069	1
17.534	-13.607	-0.676	87.74	81.833	351.833	0.211	0.014	1
13.527	-15.246	2.402	71.167	347.578	257.578	0.992	0.112	2
15.008	-14.645	1.652	68.398	347.338	257.338	1.895	0.306	2
14.223	-15.289	0.922	59.355	344.177	254.177	1.082	0.212	2
14.522	-15.062	1.346	52.836	318.257	228.257	0.647	0.069	
14.621	-14.692	2.281	89.738	140.546	50.546	0.361	0.052	
15.197	-14.698	0.812	84.13	100.686	10.686	1.62	0.209	
14.091	-14.836	2.832	85.068	231.159	141.159	0.678	0.037	
12.84	-15.637	1.803	49.344	280.523	190.523	0.699	0.066	
12.566	-15.769	2.402	13.631	6.153	276.153	0.295	0.029	
12.932	-15.483	2.462	53.68	80.773	350.773	0.322	0.031	
16.976	-13.914	-0.477	89.508	81.86	351.86	0.277	0.025	1
13.034	-15.467	2.189	58.305	86.368	356.368	0.254	0.019	
12.688	-15.964	0.384	72.6	349.604	259.604	0.297	0.03	2
13.517	-15.524	0.775	89.028	232.024	142.024	0.341	0.026	
10.719	-16.438	4.079	77.443	244.677	154.677	0.637	0.048	1
3.91	-4.633	0.344	53.548	347.896	257.896	1.217	0.525	2
2.144	-5.618	2.55	71.986	8.121	278.121	2.243	0.37	3
2.487	-5.323	2.302	77.517	13.156	283.156	0.749	0.114	3
2.686	-5.307	2.152	81.741	26.725	296.725	0.581	0.103	3
2.89	-5.198	1.562	66.044	18.828	288.828	0.812	0.174	3

2.131	-5.807	3.897	68.807	6.901	276.901	1.314	0.21	
17.298	-13.762	-0.441	43.658	99.453	9.453	0.148	0.011	
2.363	-5.764	4.209	75.314	8.032	278.032	0.62	0.076	3
2.438	-5.798	4.573	47.467	188.205	98.205	0.199	0.014	
2.267	-5.845	4.386	77.083	35.662	305.662	0.578	0.042	3
-7.965	-12.082	1.867	82.01	34.217	304.217	0.424	0.066	3
-9.237	-12.775	1.955	65.352	278.519	188.519	1.877	0.414	
-8.143	-12.506	4.908	74.339	258.681	168.681	0.952	0.149	1
-7.445	-11.829	3.614	82.71	255.442	165.442	2.303	0.279	1
-7.718	-12.319	5.16	52.035	353.019	263.019	0.511	0.13	
-9.254	-12.859	2.968	82.954	26.62	296.62	0.529	0.094	3
-10.022	-13.235	2.739	64.857	349.659	259.659	1.48	0.466	2
17.103	-13.828	-0.329	57.115	78.405	348.405	0.177	0.014	
-9.494	-13.179	4.314	66.974	233.266	143.266	0.863	0.106	
-6.996	-11.514	3.179	88.292	37.704	307.704	1.684	0.336	
-8.694	-12.57	3.772	39.053	155.559	65.559	0.4	0.07	4
-8.525	-12.678	4.952	70.539	11.036	281.036	0.842	0.212	3
6.565	-3.495	2.882	64.822	351.297	261.297	1.97	0.564	2
7.178	-3.208	2.562	70.75	348.642	258.642	1.167	0.281	2
8.886	-2.586	2.136	67.063	350.319	260.319	1.256	0.699	2
10.292	-2.122	1.606	33.147	147.331	57.331	3.649	0.459	4
8.175	-2.825	1.017	89.955	84.006	354.006	2.407	0.228	1
8.451	-2.764	0.976	42.295	135.745	45.745	0.94	0.112	4
16.656	-13.952	-0.048	42.617	101.095	11.095	0.254	0.013	
5.039	-4.127	4.91	73.499	24.571	294.571	1.032	0.221	3
5.457	-4.009	3.992	69.12	349.651	259.651	1.778	0.361	2
4.146	-4.744	5.358	84.715	247.654	157.654	1.442	0.383	1
4.789	-4.138	0.982	67.884	14.514	284.514	1.274	0.226	3
4.393	-4.452	2.094	77.962	19.615	289.615	0.602	0.101	3
3.96	-4.522	1.854	57.446	352.398	262.398	0.822	0.219	2
1.919	-5.861	1.31	68.051	221.177	131.177	1.893	0.246	
1.226	-6.455	0.672	28.949	126.969	36.969	0.579	0.105	4
1.364	-6.356	0.725	77.823	229.632	139.632	0.601	0.059	
1.036	-6.57	0.638	67.069	222.901	132.901	0.437	0.041	
16.901	-13.883	-0.166	36.277	111.1	21.1	0.1	0.003	
4.875	-4.45	2.591	64.74	275.426	185.426	0.633	0.081	
-5.59	-16.738	6.465	33.964	148.566	58.566	2.149	1.697	4
-6.879	-14.773	6.365	83.281	252.193	162.193	4.576	2.068	1
-6.424	-14.644	6.575	70.38	350.345	260.345	2.56	0.564	2
-7.786	-15.081	6.605	61.64	287.83	197.83	0.454	0.048	
-7.351	-14.588	6.536	58.262	27.756	297.756	0.526	0.073	
-7.677	-14.555	6.613	55.127	25.599	295.599	0.504	0.071	
-8.457	-14.504	6.552	63.653	350.926	260.926	0.933	0.117	2

-8.582	-14.586	6.566	63.637	352.023	262.023	1.315	0.25	2
-8.121	-13.097	5.747	66.287	20.904	290.904	1.202	0.276	3
20.573	-11.946	1.293	87.809	262.453	172.453	0.974	0.066	1
32.016	-6.347	3.075	70.087	277.193	187.193	1.597	0.449	
34.591	-5.116	1.936	63.174	305.77	215.77	0.847	0.172	
33.79	-5.534	2.619	87.17	235.892	145.892	1.234	0.208	
34.809	-5.013	2.458	86.707	40.766	310.766	1.082	0.084	
29.197	-8.455	0.167	89.884	256.702	166.702	2.725	0.565	1
22.845	-11.401	0.319	89.536	260.8	170.8	2.107	0.117	1
23.258	-11.232	0.962	43.947	299.292	209.292	1.087	0.173	
21.466	-11.724	0.958	89.229	261.191	171.191	0.76	0.079	1
22.272	-11.375	1.938	89.262	31.246	301.246	1.223	0.17	
22.056	-11.198	3.158	89.718	225.679	135.679	1.245	0.154	
22.933	-11.094	2.588	34.879	342.287	252.287	0.61	0.125	
22.842	-11.137	2.907	81.823	196.841	106.841	0.878	0.144	
20.573	-11.937	1.327	87.861	262.817	172.817	0.98	0.07	1
20.968	-11.702	1.689	59.123	310.669	220.669	0.979	0.102	
19.04	-12.754	-0.237	25.69	141.815	51.815	0.931	0.204	4
20.376	-12.197	-0.266	58.027	159.126	69.126	0.803	0.216	
20.534	-11.642	3.718	30.985	145.401	55.401	1.228	0.208	4
27.883	-9.277	0.022	67.227	349.84	259.84	4.03	1.735	2
24.508	-10.598	1.696	87.513	89.236	359.236	1.759	0.187	1
18.291	-12.737	1.903	50.732	309.038	219.038	1.386	0.156	
18.752	-12.583	1.687	59.571	312.431	222.431	1.97	0.456	
18.968	-12.538	1.394	67.22	298.122	208.122	0.562	0.073	
31.87	-6.967	-0.871	83.329	260.824	170.824	0.9	0.156	1
30.886	-7.264	0.91	19.139	153.03	63.03	0.745	0.101	4
30.596	-7.666	0.475	29.919	161.791	71.791	0.708	0.084	4
29.632	-8.115	0.335	82.926	176.273	86.273	0.334	0.044	
30.052	-7.909	0.311	77.212	184.748	94.748	0.774	0.175	
14.972	-0.49	0.061	66.645	345.752	255.752	6.719	4.205	2
16.79	0.319	1.606	61.638	4.328	274.328	4.032	1.318	
19.8	4.635	4.188	84.197	34.582	304.582	1.823	0.772	
20.073	4.204	3.752	79.782	39.354	309.354	1.04	0.301	
19.898	3.096	2.787	88.883	24.263	294.263	1.299	0.159	3
19.482	4.342	1.281	57.722	236.554	146.554	1.315	0.661	
17.737	0.727	2.784	69.296	0.625	270.625	2.959	1.023	2
16.927	0.267	3.475	67.07	7.797	277.797	1.281	0.235	
16.722	0.127	4.062	74.482	4.43	274.43	2.446	0.726	
15.968	-0.409	5.138	64.678	8.759	278.759	1.384	0.796	3
15.466	-0.683	5.387	26.041	50.69	320.69	0.83	0.223	
15.801	-0.754	5.541	81.103	350.591	260.591	0.55	0.124	2
18.353	0.453	5.275	65.291	26.847	296.847	1.601	0.575	3

18.611	0.882	4.921	69.627	354.267	264.267	1.33	0.545	2
19.236	1.379	4.719	84.78	253.692	163.692	1.238	0.271	1
19.002	1.605	1.113	80.062	251.214	161.214	1.464	0.15	1
18.808	1.532	1.524	86.971	256.898	166.898	1.198	0.175	1
14.546	-0.762	5.004	87.979	2.755	272.755	0.77	0.175	
19.809	3.411	2.261	81.397	321.982	231.982	1.804	0.204	
20.547	2.372	5.202	55.812	11.108	281.108	0.816	0.129	
20.151	3.632	3.558	50.74	3.497	273.497	0.543	0.072	
20.029	1.756	5.141	55.96	4.758	274.758	0.911	0.204	
16.668	-7.766	5.545	82.415	51.153	321.153	4.174	1.156	
17.905	-9.616	5.703	23.543	105.493	15.493	0.922	0.21	
17.166	-10.768	5.15	60.748	343.993	253.993	1.018	0.133	2
17.004	-10.389	5.212	42.457	327.324	237.324	0.621	0.051	
17.307	-10.499	5.217	24.675	139.353	49.353	0.432	0.102	4
17.962	-11.107	5.199	25.985	356.625	266.625	0.306	0.019	
17.973	-11.236	5.195	13.829	169.921	79.921	0.288	0.04	
17.839	-11.218	5.163	67.616	276.981	186.981	0.214	0.01	
18.351	-11.364	5.112	65.408	252.74	162.74	0.625	0.043	
15.728	-3.235	5.836	72.761	350.606	260.606	5.423	1.152	2
14.266	-3.045	5.852	84.312	244.957	154.957	2.362	0.356	1
15.296	-2.054	5.819	23.363	150.356	60.356	1.855	0.51	4
15.831	-0.41	5.177	65.056	10.281	280.281	0.985	0.428	3
15.76	-0.76	5.532	81.115	349.947	259.947	0.573	0.136	2
15.516	-0.626	5.348	23.106	49.636	319.636	0.593	0.129	
16.373	-0.224	5.234	63.085	275.136	185.136	0.608	0.052	
13.776	-2.662	5.88	16.981	310.242	220.242	0.76	0.193	
13.091	-2.055	5.868	28.384	143.719	53.719	0.848	0.089	4
16.8	-4.827	5.681	89.033	257.658	167.658	1.419	0.237	1
16.578	-5.112	5.698	67.898	169.534	79.534	0.548	0.105	
16.196	-5.075	5.709	77.063	240.025	150.025	0.14	0.008	
19.108	-1.146	5.712	61.276	349.673	259.673	1.561	0.224	2
21.115	-1.65	5.613	67.781	349.546	259.546	2.599	0.658	2
21.692	-2.013	5.651	87.866	74.334	344.334	0.858	0.111	1
20.087	-2.098	5.747	79.073	259.822	169.822	1.42	0.191	1
20.484	-0.497	5.741	69.91	253.791	163.791	1.702	0.272	
20.054	1.708	5.191	50.631	0.081	270.081	1.169	0.307	
20.571	2.348	5.229	53.166	7.642	277.642	0.847	0.148	
22.958	3.794	5.589	28.071	141.749	51.749	1.873	1.809	4
22.503	-0.907	5.597	87.922	254.132	164.132	0.768	0.114	1
22.774	-0.549	5.521	46.806	247.349	157.349	0.343	0.039	
21.059	4.549	5.656	85.322	252.985	162.985	1.656	0.309	1
20.842	3.764	5.449	80.209	32.774	302.774	1.121	0.114	3
20.588	3.87	5.158	79.994	26.828	296.828	0.483	0.062	3

20.793	3.302	5.005	73.956	31.524	301.524	0.58	0.08	3
20.648	3.627	4.976	83.355	53.795	323.795	0.582	0.096	
20.745	3.707	5.212	78.09	30.534	300.534	0.315	0.023	3
20.591	4.484	5.473	25.174	75.545	345.545	0.797	0.088	
24.012	5.751	5.701	70.288	18.953	288.953	2.686	0.461	3
23.933	6.772	5.613	57.012	13.593	283.593	3.004	0.478	
28.233	6.841	5.288	29.444	147.274	57.274	1.1	0.674	4
28.138	3.545	5.121	67.414	349.163	259.163	3.881	0.63	2
27.676	2.064	5.312	80.863	74.163	344.163	2.29	0.387	1
28.364	2.274	5.211	80.238	76.276	346.276	2.954	0.644	1
29.344	5.951	5.206	87.634	254.665	164.665	1.649	0.299	1
28.54	4.799	5.085	47.916	106.418	16.418	1.635	0.127	
26.792	5.303	5.351	44.871	143.322	53.322	1.099	0.073	
27.846	5.657	5.233	54.688	158.385	68.385	0.643	0.026	
28.096	5.151	5.23	86.803	284.52	194.52	1.003	0.124	
31.326	2.706	4.981	88.573	74.869	344.869	2.338	0.234	1
31.326	2.706	4.981	88.573	74.869	344.869	2.338	0.234	1
29.533	2.958	4.995	84.495	80.375	350.375	1.13	0.157	1
32.178	-3.942	4.991	30.043	334.179	244.179	1.079	0.181	
30.811	-5.33	5.153	38.376	312.99	222.99	1.37	0.229	
31.88	2.868	4.92	83.626	75.209	345.209	1.743	0.142	1
32.908	-0.51	4.835	20.793	351.039	261.039	1.988	0.626	
23.946	-2.282	5.49	80.35	52.753	322.753	1.219	0.151	
29.631	-2.607	5.272	82.707	48.898	318.898	0.659	0.062	
30.374	-2.055	5.149	72.465	54.389	324.389	0.519	0.053	
30.803	0.727	5.141	68.437	268.842	178.842	1.135	0.123	
30.756	-1.707	5.08	79.307	270.362	180.362	0.848	0.058	
31.488	-1.165	5	74.587	227.426	137.426	1.518	0.141	
20.911	-7.183	5.352	82.511	67.898	337.898	2.353	0.453	1
19.39	-2.376	5.806	87.961	75.79	345.79	1.483	0.254	1
20.083	-3.895	5.709	67.123	49.061	319.061	1.01	0.123	
20.295	-7.914	5.387	52.468	352.957	262.957	1.411	0.326	
20.211	-7.992	5.4	78.871	55.557	325.557	1.714	0.223	
26.645	-3.031	5.215	57.284	85.276	355.276	0.867	0.105	
21.631	-7.033	5.375	44.068	349.845	259.845	1	0.173	
23.891	-6.975	5.317	84.612	52.106	322.106	0.692	0.094	
23.203	-4.731	5.561	86.194	247.469	157.469	0.632	0.091	1
13.908	11.08	4.395	64.951	351.845	261.845	1.211	0.134	2
12.874	11.886	2.2	65.026	351.186	261.186	3.765	1.191	2
12.277	12.935	-0.056	65.595	350.547	260.547	1.534	0.238	2
13.978	10.495	0.722	74.699	25.198	295.198	2.939	1.186	3
15.79	9.038	3.832	80.388	36.438	306.438	3.038	2.339	3
14.983	9.798	3.429	88.883	255.45	165.45	1.547	0.42	1

13.6	10.931	2.788	83.009	29.629	299.629	1.744	0.65	3
10.185	15.303	3.361	79.668	26.744	296.744	3.855	3.069	3
10.693	15.238	4.228	35.789	232.472	142.472	0.863	0.45	
7.12	17.088	1.072	73.929	23.804	293.804	3.881	4.836	3
10.3	14.996	3.498	28.621	148.239	58.239	4.924	2.44	4
9.043	16.364	5.282	76.791	20.499	290.499	2.545	1.569	3
10.584	15.816	5.919	81.134	20.91	290.91	2.838	1.54	3
7.255	17.31	5.722	77.595	23.909	293.909	2.621	1.231	3
11.392	13.876	2.265	76.658	35.021	305.021	0.734	0.291	3
12.624	13.355	5.583	70.637	26.048	296.048	1.529	0.281	3
12.558	13.592	5.663	61.185	319.656	229.656	0.744	0.144	
11.455	14.896	4.744	56.29	22.095	292.095	1.34	0.213	
26.159	16.617	1.932	89.924	73.659	343.659	5.645	10.035	1
27.037	15.232	4.551	67.227	231.146	141.146	1.618	1.304	
26.303	15.669	5.061	71.788	70.933	340.933	3.235	1.046	
28.059	14.208	0.226	88.174	61.736	331.736	3.022	2.513	1
23.835	19.778	5.044	68.01	349.619	259.619	2.955	1.051	2
23.696	20.469	3.425	56.527	62.011	332.011	1.343	0.417	
23.42	20.894	3.713	70.372	313.375	223.375	1.514	0.257	
23.232	21.053	2.567	72.523	247.956	157.956	1.648	0.627	
19.728	25.485	2.34	68.333	38.144	308.144	2.633	1.286	
20.492	24.808	2.983	80.495	257.308	167.308	0.873	0.227	1
19.93	25.146	1.942	62.358	281.144	191.144	1.061	0.207	
20.049	24.662	1.17	63.285	292.995	202.995	0.632	0.1	
20.161	24.297	5.513	68.309	90.046	0.046	1.591	0.248	
20.075	25.005	4.47	71.081	8.911	278.911	1.146	0.294	3
18.222	26.134	5.973	27.378	148.524	58.524	13.596	9.832	4
18.074	26.944	3.538	65.334	22.328	292.328	0.913	0.136	3
21.762	23.37	4.526	86.361	62.168	332.168	1.594	0.714	1
25.664	17.945	1.811	49.176	356.724	266.724	1.01	0.17	
21.992	23.053	3.382	75.416	21.356	291.356	0.256	0.023	3
22.222	22.784	4.237	73.269	4.282	274.282	0.82	0.117	
22.388	22.449	4.48	66.816	272.559	182.559	0.389	0.052	
24.204	19.41	0.065	74.139	22.941	292.941	0.737	0.079	3
24.851	18.405	-0.321	34.717	26.269	296.269	0.457	0.042	
17.819	26.902	0.535	88.116	77.519	347.519	0.862	0.231	1
17.266	16.924	8.389	65.254	348.478	258.478	12.299	11.849	2
16.468	20.026	9.082	44.722	106.19	16.19	1.57	0.949	
19.493	20.419	8.388	87.803	80.654	350.654	6.054	4.563	1
20.679	16.67	7.862	87.202	256.361	166.361	4.051	1.687	1
13.973	12.935	7.258	80.961	31.667	301.667	3.434	3.783	3
17.38	13.459	7.656	81.376	256.932	166.932	2.097	0.201	1
17.776	11.636	6.424	86.007	255.971	165.971	0.991	0.131	1

22.118	10.538	6.036	27.628	148.851	58.851	5.375	8.217	4
15.972	10.833	5.86	48.137	349.438	259.438	1.678	0.243	
18.708	11.126	6.352	32.134	326.046	236.046	1.26	0.179	
22.405	20.352	7.174	78.694	231.365	141.365	1.24	0.542	
22.073	20.85	6.731	80.11	234.207	144.207	0.793	0.204	
22.496	20.382	6.24	79.074	241.027	151.027	1.587	0.732	
14.692	17.819	9.081	28.273	145.879	55.879	2.785	2.211	4
16.591	17.659	8.564	26.183	154.26	64.26	2.654	1.505	4
18.023	12.339	7.014	65.983	353.804	263.804	0.861	0.374	2
17.102	12.148	6.851	55.769	4.916	274.916	0.468	0.097	
16.698	11.925	6.702	76.21	269.989	179.989	0.513	0.133	
5.35	20.69	8.076	79.997	18.305	288.305	2.181	2.037	3
9.265	19.574	7.239	74.9	15.267	285.267	4.52	1.748	3
9.333	21.813	7.878	78.741	198.631	108.631	1.24	0.069	
12.954	24.062	8.042	87.761	76.71	346.71	1.805	0.235	1
12.686	23.868	8.128	78.682	356.261	266.261	0.569	0.065	2
14.931	24.639	7.603	65.25	17.828	287.828	1.035	0.259	3
34.024	11.552	2.445	61.399	351.637	261.637	1.372	0.542	2
32.79	10.987	0.761	69.291	22.108	292.108	1.52	0.473	3
31.007	11.142	1.081	77.105	25.32	295.32	1.202	0.371	3
30.177	11.893	1.515	59.555	225.119	135.119	0.401	0.041	
39.697	17.904	4.04	77.364	38.115	308.115	0.72	0.107	
38.112	-2.468	-0.225	89.784	255.532	165.532	1.538	0.317	1
37.932	-2.47	0.974	87.836	76.379	346.379	0.876	0.192	1
38.226	-1.957	1.379	76.222	258.47	168.47	1.169	0.164	1
36.332	-3.743	1.337	84.155	244.097	154.097	1.006	0.237	1
39.615	-1.015	1.726	84.472	57.664	327.664	0.826	0.196	
39.918	-0.994	1.001	89.094	74.766	344.766	0.601	0.128	1
31.835	10.59	3.064	70.328	16.575	286.575	1.813	1.198	3
40.261	-0.773	1.07	80.158	7.123	277.123	0.848	0.243	
42.676	0.803	1.285	72.625	352.886	262.886	1.206	0.224	2
43.212	1.218	0.829	85.049	252.344	162.344	0.666	0.078	1
43.13	1.254	1.767	85.557	89.617	359.617	0.524	0.071	
44.129	2.23	0.306	57.074	277.373	187.373	1.347	0.218	
49.728	8.582	1.978	87.109	260.36	170.36	1.637	0.298	1
49.379	8.177	1.635	86.318	252.545	162.545	0.624	0.086	1
48.196	6.588	1.529	88.476	258.795	168.795	1.492	0.318	1
48.003	6.223	2.134	89.49	83.757	353.757	1.099	0.211	1
38.091	-2.028	2.609	75.552	254.039	164.039	0.942	0.213	1
31.131	10.918	2.644	87.471	256.414	166.414	2.317	1.245	1
39.132	-1.089	2.905	52.853	281.287	191.287	0.758	0.154	
39.077	-0.948	3.507	45.814	292.969	202.969	0.591	0.123	
33.893	7.026	4.962	87.81	76.974	346.974	2.192	0.326	1

34.23	6.18	4.798	79.673	76.096	346.096	0.592	0.034	1
33.617	9.122	4.739	68.059	16.877	286.877	4.74	2.32	3
37.058	7.548	4.692	88.5	253.173	163.173	6.973	1.724	1
38.322	6.188	4.629	87.744	75.322	345.322	2.328	0.346	1
35.861	0.092	4.698	39.433	0.306	270.306	1.34	0.419	
37.593	1.409	4.619	86.219	55.218	325.218	2.237	0.399	
33.947	5.17	4.88	71.195	359.969	269.969	1.001	0.142	2
30.481	11.33	-0.635	89.782	74.498	344.498	1.782	0.616	1
34.604	5.839	4.762	53.656	62.92	332.92	0.684	0.101	
35.24	5.531	4.808	63.659	348.82	258.82	0.608	0.051	2
35.969	6.424	4.685	57.531	352.108	262.108	0.957	0.093	2
31.344	4.873	5.059	88.755	74.669	344.669	2.051	0.456	1
29.397	12.418	-0.472	86.967	75.682	345.682	1.927	0.439	1
29.79	12.022	3.026	89.345	77.427	347.427	0.601	0.171	1
29.432	12.487	3.442	71.346	54.876	324.876	0.593	0.148	
30.012	11.811	2.748	66.386	43.373	313.373	0.972	0.356	
32.064	10.793	-0.056	89.873	75.471	345.471	1.616	0.332	1

APPENDIX B. MAIN VARIABLES DEFINITION CODE

```
def MainVariables
; Domain Extents
x1=-10
x2=10
y1=-10
y2=10
z1=-10
z2=10
; Number of Joint sets
JSN=4
; Joint Set 1 Properties
JS1dip= 88
JS1dd= 250-65
JS1Kfisher= 104
JS1P10= 0.000000009 ;1.011
JS1P32= 0.1
; Joint Set 2 Properties
JS2dip= 68
JS2dd= 348-65
JS2Kfisher= 102
JS2P10= 0.0000000001 ;1.18
JS2P32= 0.4
; Joint Set 3 Properties
JS3dip=75
JS3dd=21-65+360
JS3Kfisher=70
JS3P10= 0.0000003;0.928
JS3P32= 0.4
; Joint Set 4 Properties
JS4dip= 29
JS4dd= 144 - 65
JS4Kfisher= 197
JS4P10= 0.00000002
JS4P32= 0.5
end
def make_NormalScanlineToAVGPlaneJSET1
Mdip= (JS1dip)*pi/180
Mstrike= (JS1dd)*pi/180
s1x1=x1*sin(Mdip)*sin(Mstrike)
S1x2=x2*sin(Mdip)*sin(Mstrike)
S1y1=y1*sin(Mdip)*cos(Mstrike)
S1y2=y2*sin(Mdip)*cos(Mstrike)
S1z1=z1*cos(Mdip)
S1z2=z2*cos(Mdip)
command
geom edge (@S1x1,@S1y1,@S1z1) (@S1x2,@S1y2,@S1z2)
endcommand
end
```

```

def make_NormalScanlineToAVGPlaneJSET2
  Mdip= (JS2dip)*pi/180
  Mstrike= (JS2dd)*pi/180
  s2x1=x1*sin(Mdip)*sin(Mstrike)
  S2x2=x2*sin(Mdip)*sin(Mstrike)
  S2y1=y1*sin(Mdip)*cos(Mstrike)
  S2y2=y2*sin(Mdip)*cos(Mstrike)
  S2z1=z1*cos(Mdip)
  S2z2=z2*cos(Mdip)
  command
    geom edge (@S2x1,@S2y1,@S2z1) (@S2x2,@S2y2,@S2z2)
  endcommand
end
def make_NormalScanlineToAVGPlaneJSET3
  Mdip= (JS3dip)*pi/180
  Mstrike= (JS3dd)*pi/180
  s3x1=x1*sin(Mdip)*sin(Mstrike)
  S3x2=x2*sin(Mdip)*sin(Mstrike)
  S3y1=y1*sin(Mdip)*cos(Mstrike)
  S3y2=y2*sin(Mdip)*cos(Mstrike)
  S3z1=z1*cos(Mdip)
  S3z2=z2*cos(Mdip)
  command
    geom edge (@S3x1,@S3y1,@S3z1) (@S3x2,@S3y2,@S3z2)
  endcommand
end
def make_NormalScanlineToAVGPlaneJSET4
  Mdip= (JS4dip)*pi/180
  Mstrike= (JS4dd)*pi/180
  s4x1=x1*sin(Mdip)*sin(Mstrike)
  S4x2=x2*sin(Mdip)*sin(Mstrike)
  S4y1=y1*sin(Mdip)*cos(Mstrike)
  S4y2=y2*sin(Mdip)*cos(Mstrike)
  S4z1=z1*cos(Mdip)
  S4z2=z2*cos(Mdip)
  command
    geom edge (@S4x1,@S4y1,@S4z1) (@S4x2,@S4y2,@S4z2)
  endcommand
end
@MainVariables
@make_NormalScanlineToAVGPlaneJSET1
@make_NormalScanlineToAVGPlaneJSET2
@make_NormalScanlineToAVGPlaneJSET3
@make_NormalScanlineToAVGPlaneJSET4

```

APPENDIX C. DFN GENERATION CODE

```
call MainVariablesDefinition.3ddat
def DFN_DomainExtent
  command
    domain extent (@x1,@x2 @y1,@y2 @z1,@z2)
  endcommand
end

@DFN_DomainExtent

call LND_TraceLength.txt

; DFN SET 1
; Generating a 3d line perpendicular to the AVG plane generated from the Joint SET 1
; Average plane: dip 88 dd 255 - Reoriented dip 88 dd 190

@LogNorm_TraceLength1

dfn template create name 'SET_1_Template' ...
  size fish @LogNorm_TraceLength1 ...
  orientation fisher @JS1dip @JS1dd @JS1Kfisher ...
  position uniform

dfn generate name 'SET_1' id 1001 template name 'SET_1_Template' ...
  genbox @x1,@x2 @y1,@y2 @z1,@z2 density @JS1P32
  ;P10 @JS1P10 (@x1,@y1,@z1) (@x2,@y2,@z2)

; DFN SET 2
; Generating a 3d line perpendicular to the AVG plane generated from the Joint SET 2
; Average plane: dip 68 dd 348 - Reoriented dip 68 dd 283

@LogNorm_TraceLength2

dfn template create name 'SET_2_Template' ...
  size fish @LogNorm_TraceLength2 ...
  orientation fisher @JS2dip @JS2dd @JS2Kfisher ...
  position uniform

dfn generate name 'SET_2' id 1002 template name 'SET_2_Template' ...
  genbox @x1,@x2 @y1,@y2 @z1,@z2 density @JS2P32
  ;P10 @JS2P10 (@x1,@y1,@z1) (@x2,@y2,@z2)

; DFN SET 3
; Generating a 3d line perpendicular to the AVG plane generated from the Joint SET 3
; Average plane: dip 75 dd 21 - Reoriented dip 75 dd 316

@LogNorm_TraceLength3

dfn template create name 'SET_3_Template' ...
  size fish @LogNorm_TraceLength3 ...
  orientation fisher @JS3dip @JS3dd @JS3Kfisher ...
  position uniform

dfn generate name 'SET_3' id 1003 template name 'SET_3_Template' ...
```

```
genbox @x1,@x2 @y1,@y2 @z1,@z2 density @JS3P32
;P10 @JS3P10 (@x1,@y1,@z1) (@x2,@y2,@z2)

; DFN SET 4 - Bedding
; Generating a 3d line perpendicular to the AVG plane generated from the Joint SET 4
; Average plane: dip 29 dd 144 - Reoriented dip 29 dd 79

dfn template create name 'SET_4_Template' ...
size gauss (9,1) slimit 1.5 30.0 ...
orientation fisher @JS4dip @JS4dd @JS4Kfisher ...
position uniform

dfn generate name 'SET_4' id 1004 template name 'SET_4_Template' ...
genbox @x1,@x2 @y1,@y2 @z1,@z2 density @JS4P32
;P10 @JS4P10 (@x1,@y1,@z1) (@x2,@y2,@z2)

dfn cluster
```

APPENDIX D. 3DEC ANALYSIS CODE

```
call DFNs_Generation.3ddat
set atol 0.0001
call GVol.3ddat
jset dfn 1004
change dfn 1004 jmat 2 2
jset dfn 1001
change dfn 1001 jmat 2 2
jset dfn 1002
change dfn 1002 jmat 2 2
jset dfn 1003
change dfn 1003 jmat 2 2
;
;
;call p10_DFN1.3dfis
;call p10_DFN2.3dfis
;call p10_DFN3.3dfis
;call p10_DFN4.3dfis
;
;
grav 0 0 -9.81
fix range z -20 -30
;
;
; --- MAT 1: Rock ---
; density = 2.7 Ton/m3
; E=50000 MPa, Poisson's Ratio= 0.3
prop mat 1 dens 0.0027 k 41667 g 19231 ;ymod 50e3 pratio 0.3
; --- JMAT: Discontinuities ---
; Joint Shear Stiffness jks [MPa/m] = 30000; According to Bandis 1983
; Joint Normal Stiffnes jkn [MPa/m] = 300000
prop jmat 1 jks 50e5 jkn 50e5 jcoh 1e20 jfric 89 jten 50e5
prop jmat 2 jks 30000 jkn 300000 jfric 30 jcoh 0.0
change dfn 1002 jmat 2 2
change dfn 1003 jmat 2 2
change dfn 1004 jmat 2 2
change dfn 1001 jmat 2 2
;
;
; --- histories to monitor convergence ---
hist nc=1 unbal
; top of model
hist xdis 0 0 10 ydis 0 0 10 zdis 0 0 10
;
plot create plot Hist
plot hist 2 3 4 yaxis label 'Displacement'
;
cycle 500
;
reset disp
```

```

delete range group "10005"
delete range group "10002"
delete range group "10003"
damp local
cyc 40000
;-----
;
;identify blocks which have a displacement > 0.02 m
;
mark region 4 range displacement 0.02 10.0 velocity 0.001 5.0 ;mark it re 4 for failed
def block_vol
bb = get_array(500,6)
cc = get_array(500,3)
cc(1,1) = string(cycle) + ' steps'
r=0
bi = block_head
loop while bi # 0
    if b_region(bi) = 4
        if b_vol(bi) # 0 then ; and volume not zero
            r= r + 1
            bb(r,1) = r
            bb( r,2) = bi
            bb( r,3) = b_vol(bi)
            bb( r,4) = b_mass(bi)
            bb( r,5) = b_vel(bi)
            bb(r,6) = runname
            cc( r+1,1) = string(b_vol(bi))
        endif
    endif
    bi = b_next(bi)
endloop
end
@block_vol
def textsummary
dd = get_array(r+1)
loop n (1,r+1)
    dd(n)= string(bb(n,1))+"," + string(bb(n,2))+"," + string(bb(n,3))+"," + string(bb(n,4))+"," +
string(bb(n,5))+"," + string(bb(n,6))
endloop
end
@textsummary
def volcalc
volc= 0.0
ton= 0.0
loop mm (1,r)
    Tvolc= volc + bb(mm, 3)
    Tton= ton + bb(mm, 4)
endloop
end
@volcalc
list @Tvolc
list @Tton

```

```
def setup
  a_size = 20
  to_read = 0
  to_write = 2 ;appended
  to_fish = 0
  to_ascii = 1
filename = 'FailedBlockSummary.dat'
end
@setup

def fileme
  status=open(filename,to_write,to_ascii)
  status=write(dd,r+1)
  status=close
end
@fileme
save @runname
```


APPENDIX E. 3DEC STOCHASTIC ANALYSIS CODE

```
new
set random 10001
[runname='run1']
call FullModel
call PauseFunction
@PFunc
;
new
set random 10002
[runname='run2']
call FullModel
call PauseFunction
@PFunc
;
new
set random 10003
[runname='run3']
call FullModel
call PauseFunction
@PFunc
;
new
set random 10004
[runname='run4']
call FullModel
call PauseFunction
@PFunc
;
new
set random 10005
[runname='run5']
call FullModel
call PauseFunction
@PFunc
;
new
set random 10006
[runname='run6']
call FullModel
call PauseFunction
@PFunc
;
```

This code is repeated as many times as iterations are wanted in the stochastic model. The random number needs to be changed and also the run name, so each model is different and is stored with a unique name.