

Experimental and Analytical Investigations of Piles and Abutments of Integral Bridges

Sami Arsoy

Dissertation submitted to the
Faculty of the Virginia Polytechnic Institute and State University
in partial fulfillment of the requirements for the degree of

Doctor of Philosophy

In

Civil Engineering

Committee:

Dr. J. Michael Duncan, Chair

Dr. Richard M. Barker

Dr. Thomas M. Murray

Dr. T. Kuppusamy

Dr. George M. Filz

Dr. Rakesh K. Kapania

December 15, 2000

Blacksburg, Virginia

Keywords: Integral bridges, piles, abutments, semi-integral abutment, temperature effects, finite element analyses

© Copyright 2000, Sami Arsoy

Experimental and Analytical Investigations of Piles and Abutments of Integral Bridges

by Sami Arsoy

Dr. J. Michael Duncan, Chair

The Charles E. Via, Jr. Department of Civil and Environmental Engineering

ABSTRACT

Bridges without expansion joints are called “integral bridges.” Eliminating joints from bridges creates concerns for the piles and the abutments of integral bridges because the abutments and the piles are subjected to temperature-induced cyclic lateral loads. As temperatures change daily and seasonally, the lengths of integral bridges increase and decrease, pushing the abutment against the approach fill and pulling it away. As a result the bridge superstructure, the abutment, the approach fill, the foundation piles and the foundation soil are all subjected to cyclic loading, and understanding their interactions is important for effective design and satisfactory performance of integral bridges.

The ability of piles to accommodate lateral displacements is a significant factor in determining the maximum possible length of integral bridges. In order to build longer integral bridges, pile stresses should be kept low.

This research project investigated the complex interactions that take place between the structural components of the integral bridge and the soil through experimental and analytical studies. A literature review was conducted to gain insight into the integral bridge/soil interactions, and to synthesize the information available about the cyclic loading damage to piles of integral bridges. The ability of the piles and the abutments to withstand cyclic loads was investigated by conducting large-scale cyclic load tests. Three pile types and three semi-integral abutments were tested in the laboratory. Experiments simulated 75 years of bridge life for each specimen by applying over 27,000 displacement cycles. Numerical analyses were conducted to investigate the interactions among the abutment, the approach fill, the foundation soil, and the piles.

The original VDOT semi-integral abutment hinge experienced shear key failure as observed in two large-scale laboratory tests. The revised hinge detail did not exhibit any sign of damage. Both abutments tolerated 75-year worth of displacement cycles without any appreciable change in their behavior. Semi-integral abutments are recommended for longer integral bridges because they can reduce pile stresses. As the need to build longer integral bridges grows, the role of the semi-integral abutments is expected to become more important.

The data from the experimental program indicates that steel H-piles are the best pile type for support of integral abutment bridges. Concrete piles are not recommended because under repeated lateral loads, tension cracks progressively worsen and significantly reduce vertical load carrying capacity of these piles. Pipe piles have high flexural stiffness, which results in an undesired condition for the shear stresses in the abutment. For this reason, stiff pipe piles are not recommended for support of integral bridges.

Numerical analyses indicate that the interactions between the approach fill and the foundation soils create favorable conditions for stresses in piles supporting integral bridges. Because of these interactions, the foundation soil acts as if it were softer, resulting in reduction in pile stresses compared to a single pile in the same soil without the approach fill above it.

ACKNOWLEDGMENTS

Throughout this study, numerous people provided support and encouragement. I would like to thank Prof. J. M. Duncan for his deep insight and guidance during the course of my studies at Virginia Tech. I would also like to thank Prof. R. M. Barker for his efforts towards making this project a success.

Prof. Thomas M. Murray contributed significantly to this research program by reviewing construction plans and recommending solutions to many difficulties encountered.

I extend my sincere appreciation to my other committee members Prof. T. Kuppusamy, Prof. George M. Filz, and Prof. Rakesh K. Kapania for their valuable contributions. I also thank Prof. J. K. Mitchell for serving on my committee until his retirement. A special thanks goes to Prof. T. E. Cousins for his help with the data acquisition system.

Contributions of the technicians Dennis Hoffman, Brett Farmer, and Clark Brown are greatly acknowledged.

A special note of recognition goes to Virginia Tech graduate students (former and current) Bob Mokwa, Emmet Sumner, Angela Terry, Micheal DeFreese, David Mokarem, Diane Baxter, Harry Cooke for their assistance at various stages of this project.

Celal Bayar University, Manisa-Turkey sponsored the first half of my studies at Virginia Tech. The Virginia Transportation Research Council and the Virginia Department of Transportation provided funding for this project. These supports are greatly acknowledged.

Finally, I sincerely express my appreciation to my wife Dr. Aysen Arsoy for her companionship, understanding and continuous encouragement throughout this challenging endeavor.

TABLE OF CONTENTS

ABSTRACT	ii
ACKNOWLEDGMENTS	iv
TABLE OF CONTENTS	v
LIST OF FIGURES	ix
LIST OF TABLES	xiv
CHAPTER 1 - INTRODUCTION	1
1.1. Integral Bridge Concept.....	1
1.2. Objectives and Scope of Research.....	2
1.3. Contents of Dissertation	3
CHAPTER 2 – LITERATURE REVIEW	5
2.1. Introduction	5
2.2. Cyclic Loading Damage to Piles of Integral Abutment Bridges	6
2.3. Performance of Piles under Cyclic Lateral Loading	8
2.4. Governing Differential Equation for the Laterally Loaded Pile Problem	10
2.5. Closed Form Solution.....	11
2.5.1. Long Piles	11
2.5.2. Short Piles	12
2.6. Approximate Solution Methods	13
2.6.1. Series Expansion.....	13
2.6.2. Finite Difference Method.....	13
2.6.3. Finite Element Method	15
2.7. Empirical Methods	18
2.7.1. The Method of p-y Curves.....	18
2.7.2. Evans and Duncan (1982) Method	19
2.7.3. SALLOP: Simple Approach for Lateral Loads on Piles (1997)	19
2.8. Equivalent Cantilever Method.....	20
2.9. Conclusions	20
CHAPTER 3 – THERMAL BRIDGE DISPLACEMENTS	22
3.1. Introduction	22

3.2. Factors Affecting Bridge Temperatures	23
3.3. Practical Treatment of Temperature Effects.....	24
3.4. Prediction of Thermal Bridge Displacements	25
3.5. Daily and Seasonal Variation of Thermal Bridge Displacements.....	27
3.6. Proposed Modeling of Daily Temperature Variations in Experimental Research	28
3.6.1. Selection of Temperature Data	29
3.6.2. Mathematical Representation of Temperature-Induced Displacements	29
3.6.3. Proposed Model	31
CHAPTER 4 - CYCLIC LOAD TESTING OF SEMI-INTEGRAL BRIDGE	
ABUTMENTS	40
4.1. Introduction	40
4.2 Design and Construction	41
4.3 Material properties.....	42
4.3.1 Dowels	42
4.3.2 Concrete	42
4.4 Load Test Setup.....	43
4.4.1 Mounting specimens on reaction floor	43
4.4.2 Application of vertical load	44
4.4.3 Application of lateral load in static loading.....	44
4.4.4 Application of lateral load in cyclic loading.....	45
4.4.5 Instrumentation and Data Acquisition	45
4.5 Test Program.....	46
4.5.1 Static Testing	47
4.5.2 Application of displacement cycles	47
4.6 Shear Key Failure	49
4.7 Results of Static Load Tests	49
4.7.1 Lateral load displacement relationship	50
4.7.2 Point of rotation of abutments.....	51
4.7.3 Rotational stiffness of specimens.....	52
4.8 Results of Cyclic Load Tests.....	52
4.9 Conclusions and Recommendations.....	53
CHAPTER 5 - CYCLIC LOAD TESTS ON PILES.....	88

5.1. Introduction	88
5.2. Design and Construction	89
5.3. Material Properties	90
5.3.1. Piles.....	90
5.3.2. Concrete	90
5.4. Load Test Setup.....	91
5.4.1. Mounting pile caps on reaction floor	91
5.4.2. Application of vertical load	92
5.4.3. Application of lateral load	93
5.4.4. Instrumentation and data acquisition	93
5.5. Test Program.....	95
5.5.1. Steel H-pile	96
5.5.2. Steel Pipe Piles.....	98
5.5.3. Prestressed Reinforced Concrete Pile	98
5.6. Results	99
5.6.1. H-pile Tests.....	99
5.6.2. Pipe Pile Tests.....	100
5.6.3. Prestressed concrete pile tests.....	101
5.7. Discussion.....	103
5.7.1. Steel H-pile	103
5.7.2. Pipe pile	103
5.7.3. Prestressed reinforced concrete pile.....	104
5.8. Conclusions and Recommendations.....	104
CHAPTER 6 – NUMERICAL INVESTIGATIONS OF INTEGRAL BRIDGE/SOIL INTERACTIONS.....	132
6.1 Introduction	132
6.2 Objectives of Numerical Analysis.....	133
6.3 Expected Soil/Abutment/Pile Interactions.....	133
6.4 Software Selection.....	134
6.4.1 SAGE	134
6.4.2 LPILE.....	135
6.5 Bridge Geometry	135

6.6 Material Properties	136
6.6.1 SAGE	136
6.6.2 LPILE.....	136
6.7 Assumptions and Sources of Errors in Numerical Modeling.....	138
6.8 Parametric Analyses	139
6.8.1 Effects of Approach Fill.....	140
6.8.2 Effects of abutment type: fully vs. semi-integral.....	141
6.9. Proposed Method to Account for Soil Structure Interaction Effects in Design of Piles Supporting Integral Abutment Bridges	143
6.10 Summary of Findings	144
6.11 Recommendations for Future Work	145
CHAPTER 7 – CONCLUSIONS AND RECOMMENDATIONS.....	160
7.1. Conclusions	161
7.2. Recommendations for Design of Integral Abutment Bridges	163
7.3. Recommendations for Future Work	163
APPENDIX A - THE BEHAVIOR OF INTEGRAL ABUTMENT BRIDGES....	164
APPENDIX B - CONSTRUCTION PLANS OF THE SEMI-INTEGRAL ABUTMENT SPECIMENS.....	199
APPENDIX C - CONSTRUCTION PLANS OF THE PILE/PILE CAP SPECIMENS.....	209
REFERENCES.....	223
VITA.....	231

LIST OF FIGURES

Figure 1.1. Simplified geometry of an integral abutment bridge..... 34

Figure 3.1. Factors controlling temperature distribution over the depth of the
superstructure (After Emerson, 1977)..... 33

Figure 3.2. Relationship between air temperature and longitudinal bridge displacement
for Maple River Bridge (From Girton et al., 1991). 34

Figure 3.3. Temperature variation in Charlottesville, VA..... 35

Figure 3.4. Monthly temperature variation in Virginia..... 36

Figure 3.5. Illustration of parameters needed to define a sine function..... 37

Figure 3.6. Representation of temperature-induced displacements by a combination of
two sine-functions..... 38

Figure 3.7. Representation of temperature-induced displacements by two separate sine
functions..... 39

Figure 4.1. Schematic illustration of the initial VDOT semi integral abutment hinge.... 55

Figure 4.2. Schematic illustration of the revised VDOT semi integral abutment hinge.. 56

Figure 4.3. Pictures from construction of the initial VDOT semi integral abutment hinge
..... 57

Figure 4.4. Pictures from construction of the revised VDOT semi integral abutment
hinge..... 58

Figure 4.5. Increase in strength of concrete with time..... 59

Figure 4.6. Schematic illustration of mounting pile caps on reaction floor..... 60

Figure 4.7. Setup used to apply vertical load to specimens 61

Figure 4.8. Roller and tilting-plate assembly used between ram and specimen 62

Figure 4.9. Setup used to apply static lateral load 63

Figure 4.10. Setup used to apply cyclic lateral load 64

Figure 4.11. Locations of displacement measuring devices 65

Figure 4.12. Picture of failed shear key	66
Figure 4.13. Lateral load/displacement relation of Specimen A for various vertical loads	67
Figure 4.14. Lateral load/displacement relation of Specimen B for various vertical loads	68
Figure 4.15. Lateral load/displacement relation of Specimen C for various vertical loads	69
Figure 4.16. Comparison of lateral load/displacement relations of Specimens A and B	70
Figure 4.17. Comparison of lateral load/displacement relations of Specimens A and C	71
Figure 4.18. Comparison of lateral load/displacement relations of Specimens B and C.	72
Figure 4.19. Bending strains measured on dowels of Specimen C.....	73
Figure 4.20. Point of rotation of Specimen A during static load tests	74
Figure 4.21. Point of rotation of Specimen B during static load tests	75
Figure 4.22. Point of rotation of Specimen C during static load tests	76
Figure 4.23. Rotational stiffness of Specimen A during static load tests	77
Figure 4.24. Rotational stiffness of Specimen B during static load tests	78
Figure 4.25. Rotational stiffness of Specimen C during static load tests	79
Figure 4.26. Lateral load/displacement relationship of Specimen B for zero vertical load during cyclic tests	80
Figure 4.27. Lateral load/displacement relationship of Specimen B for 35-kip vertical load during cyclic tests.....	81
Figure 4.28. Lateral load/displacement relationship of Specimen C for zero vertical load during cyclic tests	82
Figure 4.29. Lateral load/displacement relationship of Specimen C for 35-kip vertical load during cyclic tests.....	83

Figure 4.30. Bending strain/displacement relationship of Specimen C for zero vertical load during cyclic tests.....	84
Figure 4.31. Bending strain/displacement relationship of Specimen C for 35-kip vertical load during cyclic tests.....	85
Figure 4.32. Lateral load/displacement relationship of Specimen C for zero vertical load during large displacement cyclic tests	86
Figure 4.33. Bending strain/displacement relationship of Specimen C for zero vertical load during large displacement cyclic tests	87
Figure 5.1. Static equivalent of the pile/pile cap system in the laboratory	106
Figure 5.2. Schematic illustration of mounting pile caps on reaction floor.....	107
Figure 5.3. Mounting pile caps to reaction floor by L-shaped steel elements	108
Figure 5.4. Vertical load application to piles.....	109
Figure 5.5. Gravity load simulator (GLS).....	110
Figure 5.6. Views of the MTS actuator during cyclic lateral loading.....	111
Figure 5.7. Picture of the steel stand used to measure rotation of the pile cap.....	112
Figure 5.8. Picture of a typical wire pot transducer used to measure displacements	113
Figure 5.9. Locations of strain gages used in the experiments	114
Figure 5.10. Lateral load vs. displacement relationship in HP-AA series.....	115
Figure 5.11. Lateral load vs. displacement relationship in HP-BB series	116
Figure 5.12. Lateral load vs. displacement relationship in HP-CC series	117
Figure 5.13. Lateral load vs. displacement relationship in HP-DD series.....	118
Figure 5.14. Experimental vs. theoretical strains of the H-pile test.....	119
Figure 5.15. Displacements along the H-pile for selected lateral loads.....	120
Figure 5.16. Pictures of the damage observed in the pile cap during testing	121
Figure 5.17. Lateral load vs. displacement relationship in pipe pile test.....	122

Figure 5.18. Displacements along the pipe pile for selected lateral loads in kips	123
Figure 5.19. Bending strains along the pipe pile for selected lateral loads in kips.....	124
Figure 5.20. Failure of a strain gage because of tension cracks in the pile	125
Figure 5.21. Drop in tension strain due to tension cracks as detected by a strain gage.	126
Figure 5.22. Pictures of the tension cracks developed in the first load cycle	127
Figure 5.23. Lateral load vs. displacement relationship in CP-AA series	128
Figure 5.24. Progressively developed tension cracks in concrete pile	129
Figure 5.25. Lateral load vs. displacement relationship in CP-BB series.....	130
Figure 5.26. Lateral load vs. displacement relationship in CP-CC series.....	131
Figure 6.1. Geometry of bridge considered in finite element analyses	146
Figure 6.2. Enlarged views of fully-integral and semi-integral abutment details.....	147
Figure 6.3. Finite element mesh used for the bridge analyzed	148
Figure 6.4. Finite element mesh of the area of the fully-integral abutment.....	149
Figure 6.5. Finite element mesh of the area of the semi integral abutment.....	150
Figure 6.6. Comparison of results for SAGE and LPILE for a fully fixed head pile	151
Figure 6.7. Comparison of results for SAGE and LPILE for a partially fixed head pile	153
Figure 6.8. Sketches of the geometry of the bridge analyzed w/ and w/o the approach fill	155
Figure 6.9. Displacement behaviors of fully- and semi-integral bridges in expansion mode of the bridge in medium dense sand.....	156
Figure 6.10. Initial and rotated shape of the semi-integral abutment in expansion mode of the bridge in medium dense sand (displacements shown to true scale) ...	157
Figure 6.11. Displacement behaviors of fully- and semi-integral bridges in contraction mode of the bridge in medium dense sand.....	158

Figure 6.12. Initial and rotated shape of the semi-integral abutment in contraction mode of the bridge in medium dense sand (displacements shown to true scale) ... 159

LIST OF TABLES

Table 2.1. Summary of behavior of piles supporting integral abutment bridges.....	8
Table 2.2. Performance of piles under cyclic lateral loading.	9
Table 3.1. Proposed representation of temperature-induced displacement cycles for one year for a maximum expected displacement range from -0.5 to +0.5 inches.	32
Table 4.1. Strength properties of concrete.	43
Table 4.2. Representation of temperature-induced displacement cycles for one year for a maximum expected displacement range from -0.5 to +0.5 inches.	48
Table 5.1. Properties of the piles used in the experiments.....	90
Table 5.2. Strength properties of concrete of pile caps.....	91
Table 5.3. Locations of transducers used to measure the lateral deflections of piles	94
Table 5.4. Estimated natural frequencies of piles for 70 kips of vertical load	96
Table 5.5. Description of the tests conducted on the steel H-pile.....	97
Table 5.6. Representation of one year of temperature effects in the laboratory	97
Table 5.7. Description of the tests conducted on the concrete pile.....	98
Table 5.8. Representation of one year of temperature effects in the laboratory	99
Table 5.9. Maximum measured displacement and strain in H-pile	99
Table 5.10. Tests conducted on the concrete pile	102
Table 6.1. Properties of structural components	137
Table 6.2. Hyperbolic stress-strain and strength parameters for approach fill and foundation soils.....	137
Table 6.3. Parameters used to define p-y curves in LPILE	138
Table 6.4. Comparison of Shear and Moment as calculated by SAGE and LPILE	139
Table 6.5. Reductions in maximum pile stresses due to approach fill.....	142

Table 6.6. Calculated p-multipliers to account for the interactions between the approach fill and the foundation soil 144