

An Experimental Study on the Aging of Sands

Christopher D. P. Baxter

Dissertation submitted to the Faculty of the
Virginia Polytechnic Institute and State University
in partial fulfillment of the requirements for the degree of

Doctor of Philosophy
in
Civil Engineering

Committee:

Dr. James K. Mitchell, Chair

Dr. Thomas L. Brandon

Dr. J. Michael Duncan

Dr. George M. Filz

Dr. J. Donald Rimstidt

July 15, 1999
Blacksburg, Virginia

Keywords: aging, time-dependent, cone penetration resistance, ground modification

An Experimental Study on the Aging of Sands

by

Christopher D. P. Baxter

Dr. James K. Mitchell, Chairman

Via Department of Civil Engineering

ABSTRACT

There are numerous examples in the literature of time-dependent changes in the properties of sands, or aging effects. Most of these aging effects are of increases in the cone penetration resistance. Time-dependent increases in penetration resistance have been measured in hydraulically placed fills and freshly densified deposits, with the largest increases following the use of ground modification techniques such as vibrocompaction, dynamic compaction, and blast densification. It is not known what causes these increases in penetration resistance to occur.

The objective of this research was to gain an understanding of the possible mechanisms responsible for aging effects in sands. Current hypotheses to explain what causes aging effects in sands include increased interlocking of particles, internal stress arching, and precipitation of silica or carbonate minerals at the contacts between grains. To date, no unambiguous evidence has been presented to support these hypotheses. A laboratory testing program was developed to study the influence of different variables on the presence and magnitude of aging effects. Three different sands were tested in rigid wall cells and buckets. Samples were aged under different effective stresses, densities, tempera-

tures, and pore fluids. In every rigid wall cell, three independent measurements were made to monitor property changes during the aging process: small strain shear modulus using bender elements, electrical conductivity, and mini-cone penetration resistance. At the end of each test, detailed mineralogical tests were performed to assess changes in the chemistry of the samples and pore fluids. A total of 22 tests in rigid wall cells were performed with periods of aging ranging from 30 to 118 days. Mini-cone penetration resistances were measured in the buckets before and at various times during the aging process.

Increases in the small strain shear modulus were measured with time. It was found that sand type and pore fluid composition greatly influenced the amount of increase in small strain shear modulus. Density was also found to influence the amount of increase in small strain shear modulus. Temperature was found to have little influence on the increase in small strain shear modulus with time.

Changes in the chemistry of the samples were also measured with time. The dissolution and precipitation of minerals in solution was monitored with electrical conductivity measurements. In most of the tests, there was continual dissolution of minerals with time. Mineralogical studies and conductivity measurements indicated precipitation of carbonates and silica in two of the tests; however, scanning electron micrographs showed no visible evidence of precipitation.

Despite the measured increases in small strain shear modulus and evidence of mineral precipitation, there were no increases in the mini-cone penetration resistance with time. This finding is significant and suggests that small-scale laboratory experiments do not capture the mechanism(s) that are responsible for time-dependent increases in penetration resistance in the field.

ACKNOWLEDGEMENTS

I wish to express my deepest gratitude to my advisor, Dr. James K. Mitchell, for an outstanding experience here at Virginia Tech. It has been a great experience working with him, and the lessons I have learned will stay with me throughout my career. Thank you for the constant inspiration and encouragement throughout this difficult project.

This research was truly supported by the help of many other people, without which it would have been impossible to complete this research.

Thank you to my committee members Dr. Thomas Brandon, Dr. J. Michael Duncan, Dr. George Filz, and Dr. J. Donald Rimstidt for their review of my dissertation and helpful comments. Special thanks to Dr. Rimstidt for help with the mineralogical tests and chemical analyses and to Dr. Brandon for the help with the equipment setup and for loaning me 500 pounds of lead.

Special thanks to Charlie Dell for taking on an apprentice in his machine shop for the construction of the rigid wall cells, and to Clark Brown who taught me how to wire my equipment without electrocuting myself.

Julie Petruska helped me a great deal with the chemical titrations and the conductivity tests and loaned me a conductivity meter. Todd Solberg helped me perform the scanning electron microscopy. Their help is greatly appreciated.

Dr. Richard Weyers and Dr. Panos Diplas provided the laboratory space to work in, and their help is greatly appreciated.

Many thanks to Dr. Dale Schutt and Dr. Sedki Riad for lending me two oscilloscopes and a function generator that were used in the course of this research.

Special thanks to Dr. Michael Riemer, Dr. Kenneth Stokoe, and Dr. Carlos Santamarina for their helpful suggestions in ironing out difficulties with the bender element setup.

Thanks to Jim Coffey for help in the lab and in using the lathe. Thanks also to the numerous students who helped pluviate samples for the laboratory testing program.

Financial support for this research was provided by the Via Department of Civil Engineering and an equipment grant from Schnabel Engineering. This support is greatly appreciated.

A large part of what made my experience here at Virginia Tech so enjoyable was the discussions and friendships with fellow students John Bonita, Harry Cooke, Jesus Gomez, Matt Helmers, Laura Henry, Carmine Polito, Jake Stephens, and Monica Wang.

I am grateful for the love and support of my parents, Dave and Thelma Baxter, my brother, Ian, and Herbert and Barbara Yamane. You have all showed patience, guidance, and acceptance of my endeavors.

Lastly, I would like to thank my wife, Diane, for being my partner, colleague, editor, and best friend. Her comments and suggestions have made this research better, and her companionship has made me a better person.

TABLE OF CONTENTS

1.0 INTRODUCTION

1.1 Statement of the Problem	1
1.2 Objective and Scope of Research.....	3

2.0 REVIEW OF PREVIOUS WORK

2.1 Introduction	7
2.2 Aging Effects on the Small Strain Shear Modulus of Sands.....	7
2.3 Aging Effects on the Thermal and Electrical Conductivity of Sands	10
2.4 Aging Effects on Liquefaction Resistance	11
2.5 Aging Effects on the Stiffness and Shear Strength of Sands	13
2.6 Aging Effects on the Penetration Resistance of Sands	16
2.6.1 Jebba Dam Project (Mitchell and Solymar 1984)	16
2.6.2 Laboratory Study of Blast Densification (Dowding and Hryciw 1986) ...	18
2.6.3 Field Study of Blast Densification (Hryciw 1986).....	19
2.6.4 St. Johns River Power Park (Schmertmann et al. 1986; Schmertmann 1987).....	20
2.6.5 Pointe Noire Deep Sea Harbor (Dumas and Beaton 1988).....	21
2.6.6 Tarsiut P-45 Caisson, Beaufort Sea (Jefferies et al. 1988).....	22
2.6.7 Amauligak F-24 Caisson, Beaufort Sea (Jefferies and Rogers 1993; Rogers et al. 1990)	22
2.6.8 Field Study of Blast Densification (Thomann 1990)	23
2.6.9 Compaction of a Deep Hydraulic Fill (Massarch and Heppel 1991).....	23
2.6.10 Cone Penetration Testing Following the Loma Prieta Earthquake (Human 1994)	24
2.6.11 Field Blasting Experiment in Greeley, Colorado (Charlie et al. 1992).....	25
2.6.12 Field Blasting Experiment in Kelowna, B. C. (Gohl et al. 1994)	27
2.6.13 Blast Densification at the SM-3 site (AGRA 1995; Ground Engineering 1995).....	27
2.6.14 Laboratory Test on the Penetration Resistance of Sand (Joshi et al. 1995).....	28
2.6.15 Chek Lap Kok Airport, Hong Kong (Ng et al. 1996)	31
2.7 Trends	31

3.0 PROPOSED MECHANISMS FOR AGING EFFECTS IN SANDS

3.1 Introduction	61
3.2 Schmertmann's Hypothesis.....	61
3.2.1 Hypothesis.....	61

3.2.2	Dispersive Particle Movements.....	62
3.2.3	Increased Interlocking	62
3.2.4	Internal Stress Arching.....	63
3.2.5	Evidence Supporting Hypothesis	64
3.2.6	Summary	68
3.3	Mesri's Hypothesis.....	68
3.3.1	Introduction	68
3.3.2	Hypothesis	69
3.3.3	Evidence Supporting Hypothesis	69
3.3.4	Summary	74
3.4	Bonding Hypothesis	75
3.4.1	Introduction	75
3.4.2	Dissolution/Precipitation Reactions	75
3.4.3	Hypothesis.....	78
3.4.4	Evidence Supporting Hypothesis	79
3.5	Other Potential Mechanisms	82
3.5.1	Introduction	82
3.5.2	Blast Gas Dissipation	83
3.5.3	Biological Activity	84
3.5.4	Pressure Solution.....	85
3.6	Summary and Conclusions.....	85

4.0 LABORATORY TESTING PROGRAM

4.1	Introduction	100
4.2	Equipment Setup	101
4.2.1	Rigid Wall Cells.....	101
4.2.2	Load Frames.....	103
4.2.3	Constant Temperature Water Baths	104
4.3	Properties of Sands Tested	104
4.4	Test Measurements.....	107
4.4.1	Small Strain Shear Modulus Measurements Using Bender Elements	107
4.4.1.1	Motivation for Measuring Small Strain Shear Modulus during Aging.....	110
4.4.1.2	Influence of Cementation on the Small Strain Shear Modulus..	110
4.4.1.3	Equipment Details	111
4.4.2	Electrical Conductivity of Sands.....	115
4.4.2.1	Influence of Pore Fluid Conductivity on Electrical Conductivity	117
4.4.2.2	Influence of Soil Fabric on Electrical Conductivity.....	118
4.4.2.3	Influence of Cementation on Electrical Conductivity.....	119
4.4.2.4	Motivation for Measuring Electrical Conductivity during Aging	119

4.4.2.5	Equipment Details	120
4.4.2.6	Determination of Cell Constants	121
4.4.2.7	Measuring Soil-Water Conductivity	122
4.4.2.8	Measuring Pore Fluid Conductivity	122
4.4.3	Mini-Cone Penetration Tests.....	123
4.4.3.1	Motivation for Performing Mini-Cone Penetration Tests during Aging	123
4.4.3.2	Equipment Details	124
4.5	Laboratory Testing Program	125
4.5.1	Variables Tested	125
4.5.1.1	Sand Type.....	126
4.5.1.2	Vertical Stress	126
4.5.1.3	Temperature	126
4.5.1.4	Relative Density	127
4.5.1.5	Pore Fluid Composition	127
4.5.2	Sample Preparation	128
4.5.3	Schedule of Rigid Wall Tests.....	130
4.5.4	Mineralogical Tests	131
4.5.5	Bucket Tests	132

5.0 RESULTS

5.1	Introduction	159
5.2	Time-Dependent Changes in Small Strain Shear Modulus.....	160
5.2.1	Summary of Small Strain Shear Modulus Data	169
5.3	Time-Dependent Changes in the Electrical Conductivity of Sands.....	170
5.4	Mineralogical Tests	175
5.5	Mini-Cone Penetration Tests.....	183
5.6	Bucket Tests	189
5.7	Summary and Conclusions.....	190

6.0 SYNTHESIS OF ALL WORK AND REVISED HYPOTHESIS

6.1	Introduction	219
6.2	Review of Significant Laboratory Studies on the Aging of Sands	219
6.2.1	Laboratory Study of Blast Densification (Dowding and Hryciw 1986) ...	220
6.2.2	Laboratory Test on the Penetration Resistance of Sand (Joshi et al. 1995)	223
6.3	Can Aging Effects be Accounted for Using Analytical Techniques?.....	228
6.3.1	Estimating Cone Penetration Resistance Using Cavity Expansion Theory.....	228
6.4	Revised Hypothesis	233
6.5	Summary and Conclusions.....	238

7.0 SUMMARY AND CONCLUSIONS

7.1 Recommendations for Future Work..... 254

REFERENCES..... 257

APPENDIX A – DIGITAL FILTER FOR RECEIVED SIGNALS..... 263

**APPENDIX B – SAMPLES OF RECEIVED SIGNALS FROM SMALL STRAIN
SHEAR MODULS MEASUREMENT 266**

**APPENDIX C – CONE PENETRATION RECORDS FROM EXAMPLES OF
AGING EFFECTS IN THE FIELD..... 289**

Vita..... 303

LIST OF FIGURES

Figure 1.1	Example of aging effects following vibrocompaction (after Mitchell and Solymar 1984).....	5
Figure 1.2	Example of aging effects following blast densification (after Solymar 1984).....	6
Figure 2.1	Increase in shear modulus with time (Afifi and Woods 1971).	39
Figure 2.2	Thermal resistivity of three sands with time (Brandon and Mitchell 1989).....	39
Figure 2.3	Increased resistance to liquefaction with time (Seed 1979).....	40
Figure 2.4	Comparison of cyclic strength between undisturbed and reconstituted samples of Niigata sand (Ishihara 1985).	41
Figure 2.5	Comparison of pore pressure generation in cyclic tests of undisturbed and reconstituted Tapo Canyon sands (Arango and Miguez 1996).	42
Figure 2.6	Field cyclic strengths of aged sand deposits relative to the strength of Holocene (<10,000 years) sand (Arango and Miguez 1996).	43
Figure 2.7	Schematic of the vibrating plate experiment (Denisov and Reltov 1961).....	44
Figure 2.8	Results from the vibrating test experiment (Denisov and Reltov 1961)...	44
Figure 2.9	Results of triaxial tests on aged samples of Ham River sand (Daramola 1980).....	45
Figure 2.10	Example of quasi-preconsolidation pressure in sand (Schmertmann 1991).	46
Figure 2.11	Results of three triaxial tests showing (a.) increase in shear wave velocity with time, and (b.) stress-strain relationships (Human 1992).	47
Figure 2.12	Aging effects in simple shear showing (a) an increase in modulus after periods of rest, and (b) changes in density (Pender et al. 1992).	48
Figure 2.13	Effect of Xanthum gum on the strength of a silty soil (Martin et al. 1996).....	49

Figure 2.14	Effect of aging after blast densification at Jebba Dam (after Mitchell and Solymar 1984).	50
Figure 2.15.	Effect of aging after vibrocompaction at Jebba Dam (after Mitchell and Solymar 1984).	51
Figure 2.16	Effect of aging on a hydraulic fill at Jebba Dam (after Mitchell and Solymar 1984).	52
Figure 2.17	Results of aging effects on a laboratory blasting experiment (Dowding and Hryciw 1986).	53
Figure 2.18	Aging effects after dynamic compaction at power park site (Schmertmann et al. 1986).	54
Figure 2.19	Aging effects after dynamic compaction at Pointe Noire deep sea harbor (Dumas and Beaton 1988).	55
Figure 2.20	Aging effects after blast densification at Molipak Amauligak F-24 (Jefferies and Rogers 1993).	56
Figure 2.21	Normalized penetration resistance vs. time (Jefferies and Rogers 1993).	57
Figure 2.22	Rate of increase in penetration resistance as a function of temperature (Jefferies and Rogers 1993).	57
Figure 2.23	Aging effects after blasting at SM-3 Site in Quebec (AGRA 1995).	58
Figure 2.24	Load displacement curves for penetration resistance tests in the laboratory (Joshi et al. 1995).	59
Figure 2.25	Increase in penetration resistance with time for laboratory specimens (Joshi et al. 1995).	59
Figure 2.26	Effect of aging after vibrocompaction at Chek Lap Kok airport (after Ng et al. 1996).	60
Figure 3.1	Development of load chains in a granular assembly of photoelastic disks (Dresher and De Josselin De Jong 1972).	88
Figure 3.2	Numerical simulation of load chains using discrete element modeling (Kuhn 1987).	89

Figure 3.3	Two samples at different stress states but having “identical” structures (after Schmertmann 1991).....	90
Figure 3.4	Mobilized c' and ϕ' for an IDS test during shear (after Schmertmann 1991).....	91
Figure 3.5	Mobilization of c' and ϕ' as a function of strain (after Schmertmann 1991).....	92
Figure 3.6	Conceptual stress-strain curve for IDS tests using one sample.....	92
Figure 3.7	Development of the frictional and cohesive components of strength with strain during IDS triaxial tests (after Schmertmann 1991).	93
Figure 3.8	IDS test results comparing a sample aged for five weeks to a sample mixed with dispersants and aged for one day (after Schmertmann 1991).....	94
Figure 3.9	Dissolution of silica with time (after Wilding et al. 1977).....	95
Figure 3.10	The effect of pH on the solubility of silica (after Dove and Rimstidt 1994).....	96
Figure 3.11	The effect of pH on the solubility of calcium carbonate (after Langmuir 1997).....	97
Figure 3.12	Scanning electron micrographs showing the effect of aging on (a) freshly deposited river sand, (b) freshly deposited Beaufort Sea sand, (c) river sand after dry aging, (d) Beaufort Sea sand after dry aging, (e) river sand aged in distilled water, (f) Beaufort Sea sand aged in distilled water, (g) river sand aged in seawater, (h) Beaufort Sea sand aged in seawater (Joshi et al. 1995).....	98
Figure 3.13	Effect of the degree of saturation on cone penetration resistance (Hyrciw 1986).	99
Figure 4.1	Fixed wall cell used for this study.....	134
Figure 4.2	Creep of PVC and steel cell with time.	135
Figure 4.3	Construction of composite cell on a lathe.	136
Figure 4.4	Press-fitting PVC sleeve into a steel pipe.	136

Figure 4.5	Rigid wall cell showing the dial gauge setup for measuring settlements.....	137
Figure 4.6	Two load frames used for this study.	138
Figure 4.7	The lever arm system used to load the rigid wall cells.	139
Figure 4.8	Linear motion bearing.	139
Figure 4.9	Grain size distributions for Evanston sand, Density sand, and Lightcastle sand.	140
Figure 4.10	Scanning electron micrographs of Evanston sand (a) scale = 1000 μm , (b) scale = 100 μm	141
Figure 4.11	Energy dispersive spectrum for Evanston sand showing (a) dolomite, and (b) quartz.	142
Figure 4.12	Scanning electron micrographs of Density sand (a) scale = 1000 μm , (b) scale = 100 μm	143
Figure 4.13	Energy dispersive spectrum of Density sand indicating quartz.	144
Figure 4.14	Scanning electron micrographs of Lightcastle sand (a) scale = 1000 μm , (b) scale = 100 μm	145
Figure 4.15	Schematic of a piezoceramic (a) single sheet and (b) double sheet “bender element”.....	146
Figure 4.16	Schematic of bender element setup.....	147
Figure 4.17	Influence of cementation on the small strain shear modulus (Acar and El-Tahir 1986).	148
Figure 4.18	Waveforms generated by the bender element setup used in this study... ..	149
Figure 4.19	Different wiring setups for bender elements, (a) series connection, and (b) parallel connection (Dyvik and Madshus 1985).	150
Figure 4.20	Oscilloscope screen image showing the transmitted half sine pulse and the received signal.....	151

Figure 4.21	Influence of salt concentration on the electrical conductivity of water (Sadek 1993).	152
Figure 4.22	Flow of electrons through a sample for (a) 1-D conditions, and (b) 2-D conditions.	153
Figure 4.23	Determination of cell constants, K, for electrical conductivity.....	154
Figure 4.24	Variation of cell constant, K, with temperature.	154
Figure 4.25	Schematic of electrical conductivity setup.....	155
Figure 4.26	Schematic of mini-cone penetration tests.....	156
Figure 4.27	PVC “extenders” used during pluviation of samples.	157
Figure 4.28	Mini-cone penetration test for bucket tests.	158
Figure 5.1	Typical increase in the small strain shear modulus with time plotted (a) semi-logarithmically, and (b) arithmetically.....	194
Figure 5.2	Small strain shear modulus as a function of time for Evanston sand in distilled water, $\sigma'_v = 100$ kPa.	195
Figure 5.3	Small strain shear modulus as a function of time for Evanston sand in ethylene glycol, $\sigma'_v = 100$ kPa.	195
Figure 5.4	Small strain shear modulus as a function of time for Evanston sand in CO ₂ -saturated water and dry, $\sigma'_v = 100$ kPa.	196
Figure 5.5	Small strain shear modulus as a function of time for Density sand in distilled water, $\sigma'_v = 100$ kPa.	197
Figure 5.6	Small strain shear modulus as a function of time for Density sand in ethylene glycol, $\sigma'_v = 100$ kPa.	197
Figure 5.7	Small strain shear modulus as a function of time for Density sand in CO ₂ -saturated water and dry, $\sigma'_v = 100$ kPa.	198
Figure 5.8	Results of regression analyses for (a) all 22 rigid wall tests, and (b) Evanston sand only.....	199

Figure 5.9	Variation of the water, vertical, and horizontal conductivity for both loose and dense Evanston and Density sand at 25° C.	200
Figure 5.10	Variation of water, vertical, and horizontal conductivity for both loose and dense Evanston and Density sand at 40° C.	201
Figure 5.11	Variation of water, vertical, and horizontal conductivity for Evanston and Density sand with CO ₂ saturated water at ~22° C.	202
Figure 5.12	Variation of pH with time for samples in distilled water.	203
Figure 5.13	Variation of pH with time for samples in CO ₂ -saturated water.	203
Figure 5.14	Variation of the formation factors and the anisotropy index for both loose and dense Evanston and Density sand at 25° C.	204
Figure 5.15	Variation of the formation factors and the anisotropy index for both loose and dense Evanston and Density sand at 40° C.	205
Figure 5.16	Variation of the formation factors and the anisotropy index for Evanston and Density sand with CO ₂ saturated water at ~22° C.	206
Figure 5.17	ICP test results for samples tested in rigid wall cells in distilled water. .	207
Figure 5.18	ICP test results for samples tested in rigid wall cells in CO ₂ -saturated water.	208
Figure 5.19	Scanning electron micrographs after aging for (a) Evanston sand in CO ₂ -saturated water, and (b) Density sand in distilled water at 40° C.	209
Figure 5.20	Plan view of a rigid wall cell showing the holes through which the mini-cone penetration tests are performed.	210
Figure 5.21	Typical penetration test results for Evanston sand in distilled water showing the two penetration tests through the top cap of the rigid wall cell.	211
Figure 5.22	Penetration test results for samples of dense Evanston sand in distilled water and ethylene glycol.	212
Figure 5.23	Mini-cone penetration resistance for loose and dense Evanston sand in distilled water at 25° C and 40° C.	213

Figure 5.24	Mini-cone penetration resistance for loose and dense Evanston sand in ethylene glycol at 25° C and 40° C.	213
Figure 5.25	Mini-cone penetration resistance for loose and dense Density sand in distilled water at 25° C and 40° C.	214
Figure 5.26	Mini-cone penetration resistance for loose and dense Density sand in ethylene glycol at 25° C and 40° C.	214
Figure 5.27	Mini-cone penetration resistance for loose and dense Evanston sand in CO ₂ -saturated water at 25° C and 40° C.	215
Figure 5.28	Mini-cone penetration resistance for loose and dense Density sand in CO ₂ -saturated water at 25° C and 40° C.	215
Figure 5.29	Comparison of load deformation behavior for Density sand in a rigid wall cell and an oedometer.	216
Figure 5.30	Mini-cone penetration test results in a 5-gallon bucket containing Evanston sand saturated with distilled water.	217
Figure 5.31	Mini-cone penetration test results in a 5-gallon bucket containing Evanston sand saturated with a 0.1 molar NaCl solution.	217
Figure 5.32	Mini-cone penetration test results in a 5-gallon bucket containing Lightcastle sand saturated with distilled water.	218
Figure 5.31	Mini-cone penetration test results in a 5-gallon bucket containing Lightcastle sand saturated with a 0.1 molar NaCl solution.	218
Figure 6.1	Diagram of liquefaction tank used by Dowding and Hryciw (1986).	241
Figure 6.2	Increase in penetration resistance with time in a laboratory blasting experiment (Dowding and Hryciw 1986).	242
Figure 6.3	Penetration test results after detonation of 2 charges (Dowding and Hryciw 1986).	243
Figure 6.4	Penetration test results for freshly deposited sand (Dowding and Hryciw 1986).	244
Figure 6.5	Percent increase in q_c after blasting (Dowding and Hryciw 1986).	245

Figure 6.6	Percent increase in q_c in freshly deposited fill (Dowding and Hryciw 1986).....	245
Figure 6.7	Section and plan view of equipment used by Joshi et al. (1995).	246
Figure 6.8	Penetration test data for Joshi et al. (1995).	247
Figure 6.9	Penetration test data from Joshi et al. (1995) showing cumulative displacements and loading-unloading cycles.	247
Figure 6.10	Evanston sand saturated with (a) seawater, and (b) calcium rich water, and then air-dried without drainage.	248
Figure 6.11	Zones of different stress-strain behavior used in cavity expansion theory to model cone penetration resistance (after Salgado 1993).....	249
Figure 6.12	Example of increasing heterogeneity with time following blast densification (after Mitchell and Solymar 1984).	250
Figure B.1	Test No. 1: Received signal for Evanston sand in distilled water, $Dr_o = 40\%$, $T = 25^\circ C$	267
Figure B.2	Test No. 2: Received signal for Evanston sand in ethylene glycol, $Dr_o = 40\%$, $T = 25^\circ C$	268
Figure B.3	Test No. 3: Received signal for Evanston sand in carbon dioxide-saturated water, $Dr_o = 40\%$, $T = 25^\circ C$	269
Figure B.4	Test No. 4: Received signal for Evanston sand in distilled water $Dr_o = 80\%$, $T = 25^\circ C$	270
Figure B.5	Test No. 5: Received signal for Evanston sand in ethylene glycol $Dr_o = 80\%$, $T = 25^\circ C$	271
Figure B.6	Test No. 6: Received signal for Evanston sand in carbon dioxide-saturated water, $Dr_o = 80\%$, $T = 25^\circ C$	272
Figure B.7	Test No. 7: Received signal for dry Evanston sand $Dr_o = 80\%$, $T = 22^\circ C$	273
Figure B.8	Test No. 8: Received signal for Evanston sand in distilled water $Dr_o = 40\%$, $T = 40^\circ C$	274

Figure B.9	Test No. 9: Received signal for Evanston sand in ethylene glycol, Dr _o = 40%, T = 40° C.....	275
Figure B.10	Test No. 10: Received signal for Evanston sand in distilled water, Dr _o = 80%, T = 40° C.....	276
Figure B.11	Test No. 11: Received signal for Evanston sand in ethylene glycol, Dr _o = 80%, T = 40° C.....	277
Figure B.12	Test No. 12: Received signal for Density sand in distilled water, Dr _o = 40%, T = 25° C.....	278
Figure B.13	Test No. 13: Received signal for Density sand in ethylene glycol, Dr _o = 40%, T = 25° C.....	279
Figure B.14	Test No. 14: Received signal for Density sand in carbon dioxide- saturated water, Dr _o = 40%, T = 25° C.....	280
Figure B.15	Test No. 15: Received signal for Density sand in distilled water, Dr _o = 80%, T = 25° C.....	281
Figure B.16	Test No. 16: Received signal for Density sand in ethylene glycol, Dr _o = 80%, T = 25° C.....	282
Figure B.17	Test No. 17: Received signal for Density sand in carbon dioxide- saturated water, Dr _o = 80%, T = 25° C.....	283
Figure B.18	Test No. 18: Received signal for dry Density sand Dr _o = 80%, T = 22° C.....	284
Figure B.19	Test No. 19: Received signal for Density sand in distilled water, Dr _o = 40%, T = 40° C.....	285
Figure B.20	Test No. 20: Received signal for Density sand in ethylene glycol, Dr _o = 40%, T = 40° C.....	286
Figure B.21	Test No. 21: Received signal for Density sand in distilled water, Dr _o = 80%, T = 40° C.....	287
Figure B.22	Test No. 22: Received signal for Density sand in ethylene glycol, Dr _o = 80%, T = 40° C.....	288
Figure C.1	Example of cone penetration test results following blast densification (AGRA 1995).....	290

Figure C.2	Example of cone penetration test results following blast densification (after Charlie et al. 1992)	291
Figure C.3	Example of cone penetration test results following blast densification (after Gohl et al. 1994)	292
Figure C.4	Example of cone penetration test results following blast densification (after Jefferies and Rogers 1993)	293
Figure C.5	Example of cone penetration test results following blast densification (after La Fosse and von Rosenvinge 1992)	294
Figure C.6	Example of cone penetration test results following blast densification, Blast Zone 2, CPT Area 101(after Mitchell and Solymar 1984)	295
Figure C.7	Example of cone penetration test results following blast densification, Test Blast 17 (after Mitchell and Solymar 1984)	296
Figure C.8	Example of cone penetration test results following blast densification, Test Blast 3 (after Solymar 1984)	297
Figure C.9	Example of cone penetration test results following dynamic compaction (after Dumas and Beaton 1988)	298
Figure C.10	Example of cone penetration test results following dynamic compaction (after Mitchell and Solymar 1984)	299
Figure C.11	Example of cone penetration test results following vibrocompaction (after Mitchell and Solymar 1984)	300
Figure C.12	Example of cone penetration test results following vibrocompaction (after Ng et al. 1996)	301
Figure C.13	Example of cone penetration test results hydraulically placed fill (after Mitchell and Solymar 1984)	302

LIST OF TABLES

Table 2.1	Values of N_G for various soils (after Jamiolkowski 1996).....	9
Table 2.2	Properties of Jebba sand.	17
Table 2.3	Properties of Evanston beach sand.....	19
Table 2.4	Time improvement factors for q_c at power plant site.....	21
Table 2.5	Properties of Douglas Lake sand.....	23
Table 2.6	Properties of sand from Greeley, Colorado.....	25
Table 2.7	Empirical constants for aging effects in penetration resistance (Charlie et al. 1992).....	26
Table 2.8	Properties of sand from the Saint Marguerite River.	28
Table 2.9	Properties of River sand and Beaufort Sea sand.	29
Table 2.10	Summary of aging effects in sands reported in the literature.....	35
Table 2.11	Main points of examples of aging effects in sands.	37
Table 2.12	Main points of examples of aging effects involving cone penetration tests.	38
Table 3.1	C_D values back-calculated from the literature.....	73
Table 3.2	Solubility of quartz and amorphous silica for different temperatures.....	76
Table 3.3	Solubility of SiO_2 in some natural systems (Langmuir 1997).	77
Table 3.4	Ca^{2+} concentrations, in ppm, for varying temperature and partial pressure of CO_2 (Langmuir 1997).	78
Table 4.1	Properties of sands used in this study.....	105
Table 4.2	Bulk chemical analyses of Evanston and Density sand.	106
Table 4.3	Electrical conductivity data for two sands (Arulanandan and Kutter 1978).....	118

Table 4.4	Variables tested in this laboratory study.	125
Table 4.5	Schedule of rigid wall tests performed in this study.	131
Table 4.6	Schedule of mini-cone penetration tests for bucket tests.	133
Table 5.1	Values of N_G for samples in rigid wall cells.	162
Table 5.2	Values of N_G for various soils from the literature (Afifi and Woods 1971; Anderson and Stokoe 1978; and Jamiolkowski 1996).	162
Table 5.3	Variables and level settings for regression analysis of N_G values.	166
Table 5.4	Influence of variables on N_G from results of regression analysis.	168
Table 5.5	Influence of variables on N_G from results of regression analysis using Evanston sand data only.	169
Table 5.6	ICP test results for samples soaked for 2 days in distilled water.	177
Table 5.7	Alkalinity titration data for rigid wall tests.	178
Table 5.8	Relative saturation of pore fluid with respect to calcite and dolomite for the rigid wall tests.	181
Table 5.9	Silica solubility data for rigid wall tests.	182
Table 6.1	Penetration resistance at 40 cm depth in liquefaction tank.	221
Table 6.2	Comparison between the current study and that of Joshi et al. (1995). ..	224
Table 6.3	Properties of Ticino sand.	230
Table 6.4	Results of the parametric study using CONPOINT for $Dr_o=30\%$	232
Table 6.5	Results of the parametric study using CONPOINT for $Dr_o=80\%$	232
Table 6.6	Changes in heterogeneity in cone penetration resistance associated with blast densification.	236
Table 6.7	Changes in heterogeneity in cone penetration resistance associated with dynamic compaction.	237

Table 6.8	Changes in heterogeneity in cone penetration resistance associated with vibrocompaction.....	237
Table 6.9	Changes in heterogeneity in cone penetration resistance associated with hydraulically placed fill.....	237