

A PERFORMANCE ANALYSIS OF THE HI-PLAN  
STRUCTURAL APPARATUS,

by

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Thesis submitted to the Graduate Faculty of the  
Virginia Polytechnic Institute and State University  
in partial fulfillment of the requirements for the degree of

MASTER OF SCIENCE

in

Civil Engineering

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December, 1985  
Blacksburg, Virginia

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ABSTRACT

Visual experience is a powerful pedagogic tool. Extensive use of experimental studies prior to design and construction has made conceptualization of complex structures possible. Experiments on reduced-scale structures and specimens are also vital tools for teaching structural mechanics. As such, the Department of Civil Engineering at Virginia Tech has acquired a new apparatus for use as an educational demonstration tool in the area of structural mechanics. This work presents the results of a detailed study on the performance of this device as related to its accuracy and operation.

To fulfill such objectives, two structural models (a continuous beam and a portal frame) were extensively tested under several loading and support configurations. The models were analysed using STRUDL as well as a computer program developed by the author. The comparison of the results (deformations) obtained in the two phases of the study have indicated that the apparatus is reasonably accurate to meet the requirements of a structural teaching model and adapting to a variety of structural models.

## ACKNOWLEDGEMENT

I wish to thank the chairman of the graduate committee Dr. Richard M. Barker for his guidance, sincere concern and his patience during my studies and thesis.

Thanks are also extended to the members of the committee, Prof. Don A. Garst and Dr. Joseph H. Moore for their fruitful suggestions and having patiently gone through every page of my thesis.

I am also thankful to Professor A.A. Pap for his valuable guidance in developing the computer program and  
for typing of the thesis.

Special thanks are extended to my parents and parents in-law for their support and help, which will always be remembered.

I would like to express my deepest appreciation to my wife for her love, continuous support and help during the course of my studies and my children, and for giving me hope.

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## CHAPTER I

### INTRODUCTION

#### 1.1 Modeling in Structural Engineering

The use of small-scale models by engineers and builders dates back many years. However, these early models were primarily aids for planning and constructing structures rather than a tool for predicting deformations and strength of full scale structure. The development of modeling as a practical tool for design and prediction of prototype's performance has been sharply influenced by advancement in area of experimental stress analysis techniques and introduction of measuring devices such as electrical resistance strain gages, linear variable differential transformers (LVDT), brittle and photelastic coatings and automated data acquisition systems.

Structural models can be defined and classified in a variety of ways. One is according to their intended function, such as strength model that is used to predict behavior of a prototype for all loads up to failure. Such a classification is presented in a text written by G.M. Sabnis, (5).

The type of model that is the concern of this thesis is an instructional or educational model that is used primarily as a complement to a mathematical model.

## 1.2 Educational Modeling

Barry Hilson in his book Basic Structural Behavior Via Models (2) says, "The concept of structural testing should be presented to the students at an early stage of their course of study so that they may freely apply their imagination and initiative without being too influenced by standard solutions to the problems."

Several other scholars in this field have emphasized the necessity of structural testing as a supplement to the more traditional subjects of theory and design of structures. But cost and availability of space for such a facility is usually a major problem in today's schools of engineering.

Unlike the apparatus used for research activities with a high degree of precision which usually requires a large amount of capital investment, educational model testing equipment should be very simple. This is for two main reasons: (1) no great skill will be necessary in manufacturing the models, (2) the observation and testing will be easier. Such equipment should be accurate enough to demonstrate the different aspects of the concept under study. Therefore similitude distortion that does not



markedly influence the desired behavior of the system is usually permissible.

The apparatus used during the course of this study satisfies such requirements, since it is a portable rigid stand that occupies very small space in the laboratory and serves as a universal base for variety of structural experiments. The testing models are easy to manufacture and testing arrangements are very simple to assemble.

### 1.3 Investigation Goals

The objective of this study is to provide a detailed description on the performance of a new apparatus purchased by the Department of Civil Engineering at Virginia Tech for use as an educational demonstration tool in the area of structural mechanics.

To familiarize the reader with the equipment, Chapter II provides a detailed description of the apparatus and its different fixtures. This chapter contains several pictures which should clarify the assembly process of the testing arrangements. Chapter III explains the testing procedures of several experiments that have been performed.

A FORTRAN computer program has been developed in order to have a quick way of checking the accuracy of experimental results. This program is capable of solving several beam bending problems. In Chapter IV the structural development of this program is presented.

Chapter V provides a brief discussion of the experimental results obtained and their differences with theoretical values. This section draws attention to possible sources of errors involved and in general terms presents the experience the author gained during experimental work with this apparatus. Finally Chapter VI contains the conclusion of this study. Appendixes include tabulated theoretical and experimental results, figures of testing arrangements, samples of hand calculated problems, and a listing of computer programs used for this study.

## CHAPTER II

### TESTING EQUIPMENT

#### 2.1 Introduction

The major piece of equipment used for experimental testing was a portable rigid frame which served as a foundation for a variety of structural model testing. This frame, structural testing models, and several other related accessories all have been designed and manufactured by Hi-Tech LTD, of England. The different sets of fixtures designed to be used with this frame for different structural experiments are all standardized and mostly multifunctional which gives a high degree of versatility to the system. For example, the dial gage assembly is so adaptable that it can be used at any point of the structure to monitor the deformation in any desired direction. Also, the apparatus is so simple that additional parts or models can be fabricated in case of future needs with very little cost. A brief description of different parts of the apparatus and their application in experimental work are presented in this chapter. The names used for the parts in the following discussion are according to the manufacturer's catalog (8).

## 2.2 Universal Frame and Stand

The apparatus shown in Figure 2.1 is an assemblage of two parallel rectangular frames 1.22 m wide by 0.76 m high. It is fabricated from 40 mm square heavy gage mild steel tubing clamped together with four aluminum corner blocks. These frames are bolted to a stand constructed of 25 mm square mild steel tubing with a shelf fitted over the two lower struts and standing on four adjustable mounting feet.

## 2.3 Model Beam and Its Fixtures

The beam specimen is a mild steel bar 1.21 m long with a rectangular cross section 25 mm wide and 5 mm thick.

The supports for the beam during experimental work are provided by end support brackets (Figure 2.2a) that are made of cast iron and act as rigid columns under the loaded beam. They can be fastened to the stand at any point for desired length of span or spans within the available length of the testing frame. Theoretical beam boundary conditions such as fixed or pin supports are simulated by specially designed devices on top of the end support brackets. These devices actually dictate the movement or degree of freedom of the beam under applied load at the resting point. Some of the devices that were used are as follows:

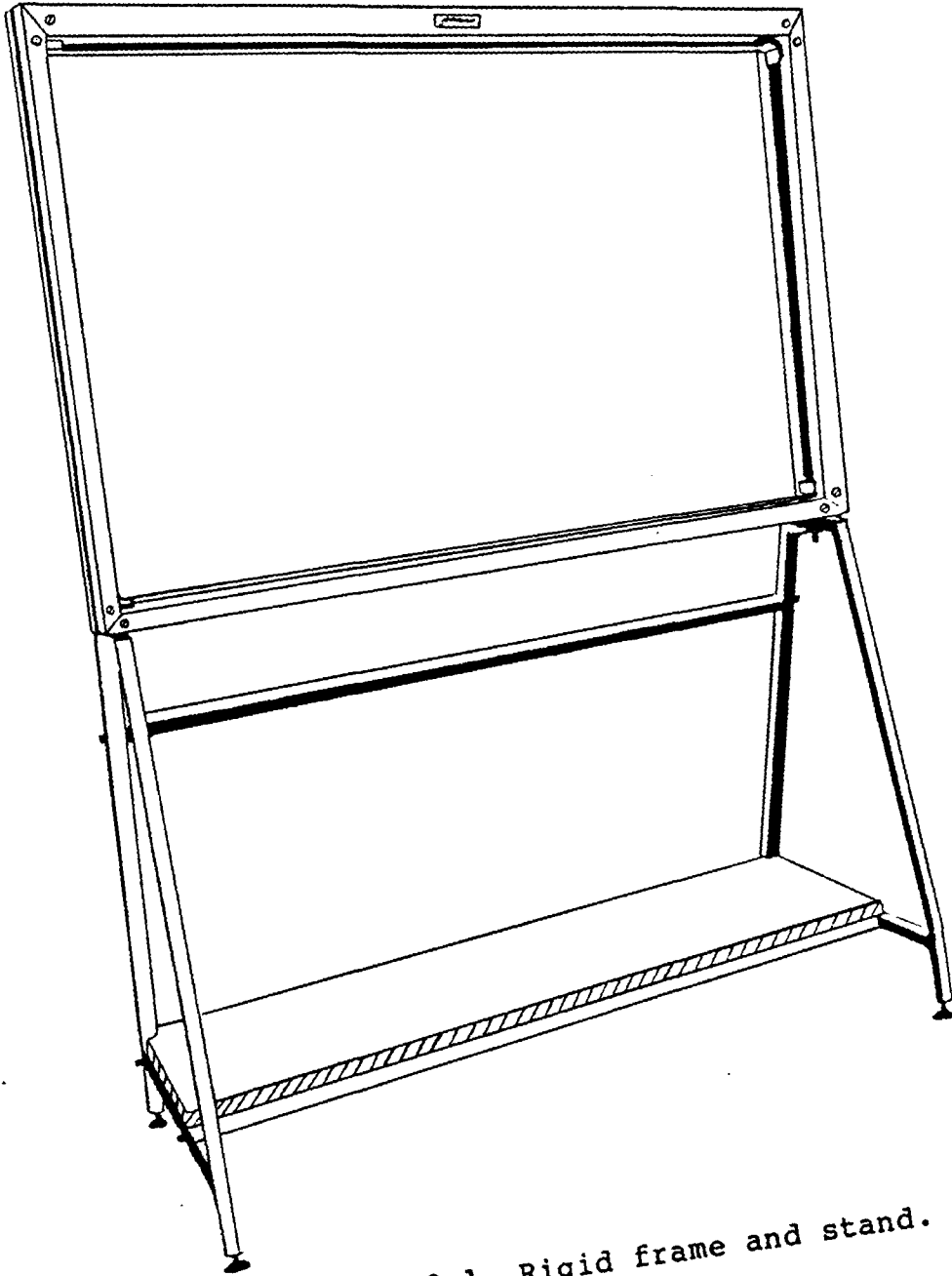
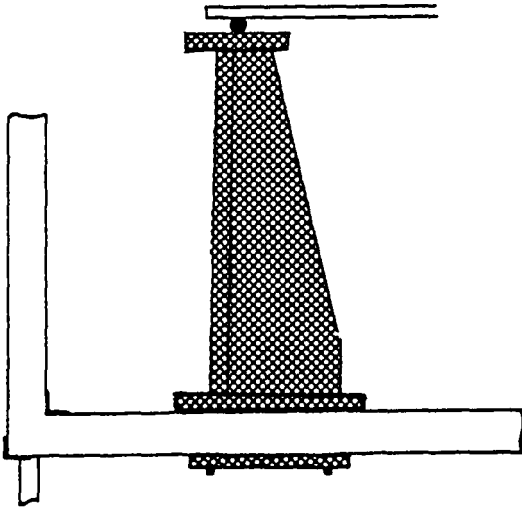
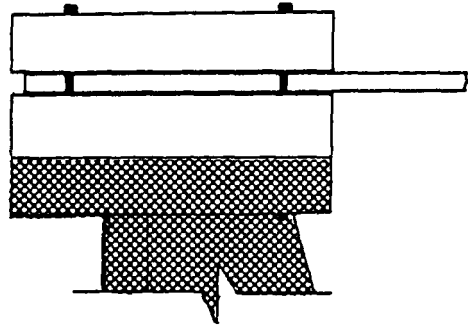


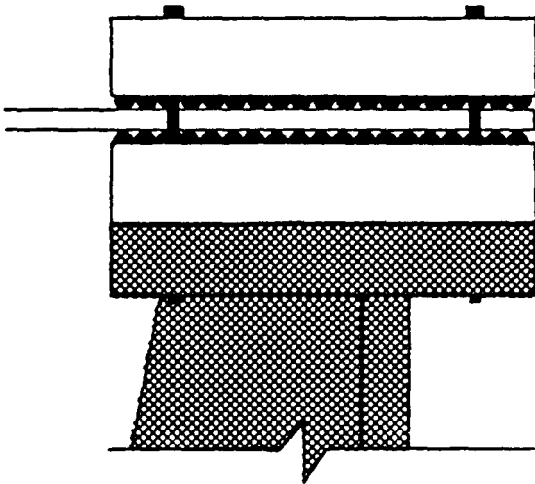
Fig. 2.1 Rigid frame and stand.



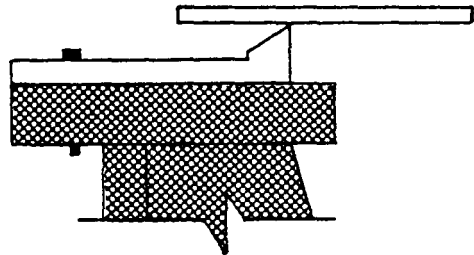
A. End support bracket and roller



B. Clamp plate



C. Roller fixture



D. Knife-edge support

Fig. 2.2 Beam supporting fixtures

1. Clamped plate used as a built-in (fixed) support condition. It prevents any displacement or rotation of the beam at the supporting point (Figure 2.2b).
2. Roller fixture used as a built-in (fixed) support that allows the beam to move in an axial direction (Figure 2.2c).
3. Knife-edge and roller supports prevent the beam from moving in a vertical direction but allow rotation and movement in its axial direction (Figures 2.2d & 2.2a).

#### 2.4 Model Frame and Its Fixtures

A single bay portal frame was used for experimental testing. It is fabricated from a mild steel 25 mm x 8 mm thick rectangular cross section with 450 mm vertical legs at 600 mm on center. The beam cantilevers 90 mm at each side (Figure 2.3). One end of the frame is welded to an aluminum block which can be used along with the knife-edge bracket as a fixed support (Figure 2.4a) or pin support (Figure 2.4b). The other end is connected to a pair of ball bearing wheels which allows this end to move freely in the horizontal direction on the ground track plate (Figure 2.5a). A hold down device (Figure 2.5b) and a fixing block (Figure 2.5c) were added to the system to be used on the end of the frame with the wheel. The hold down device is

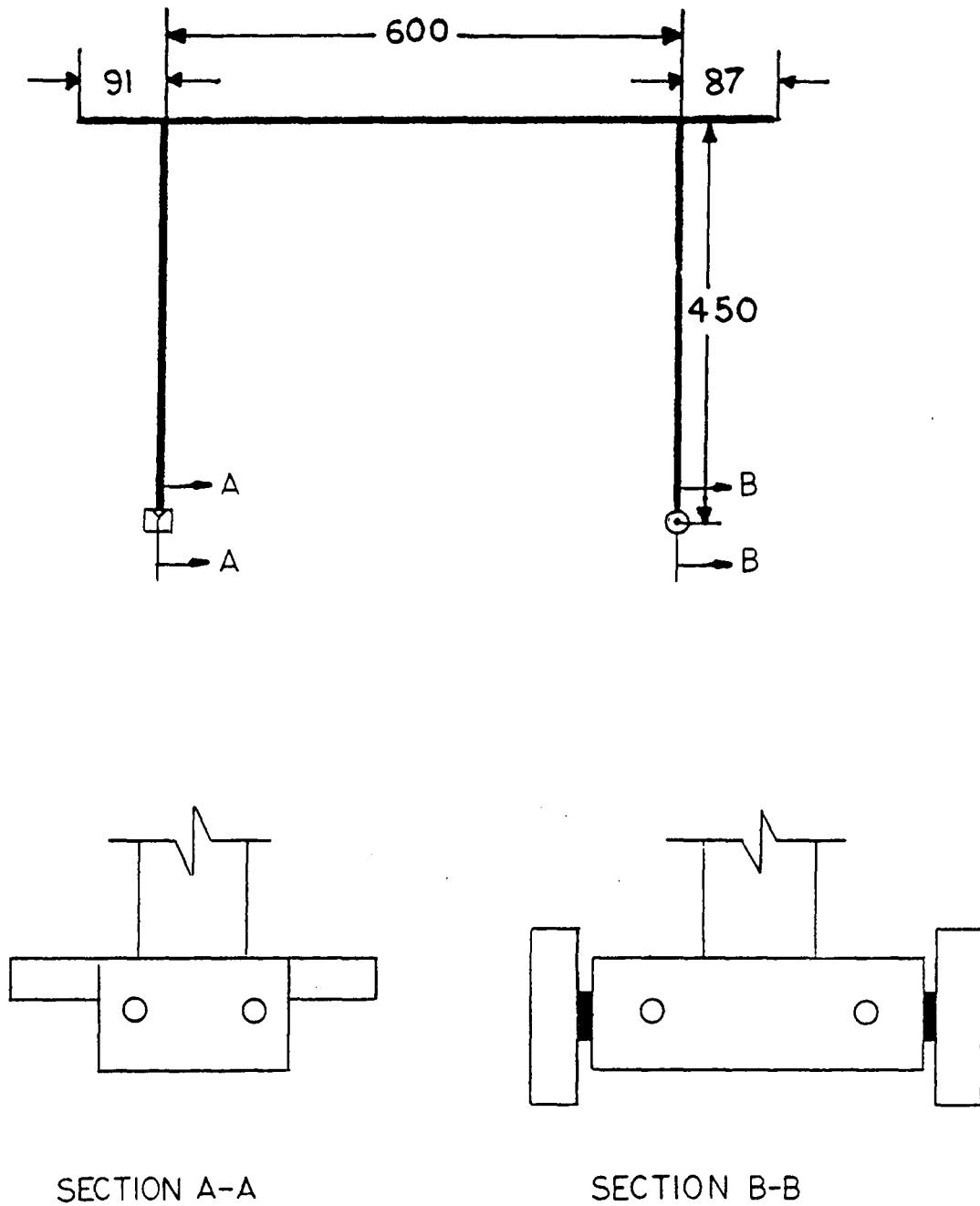
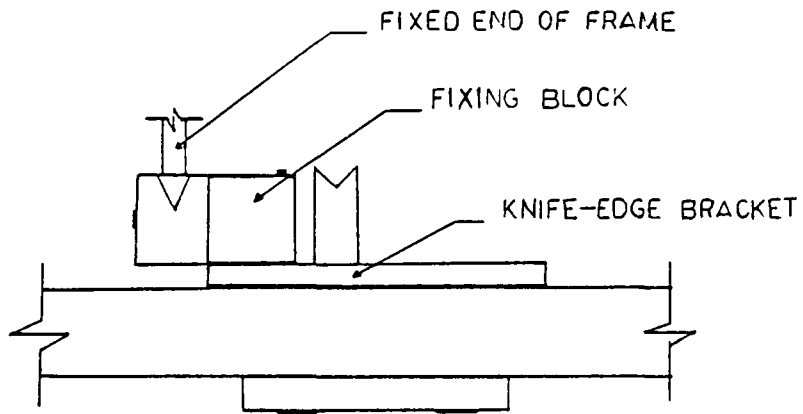
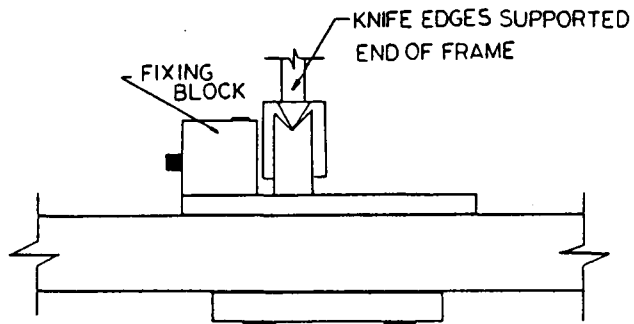


Fig. 2.3 Portal frame model (Unit = mm)



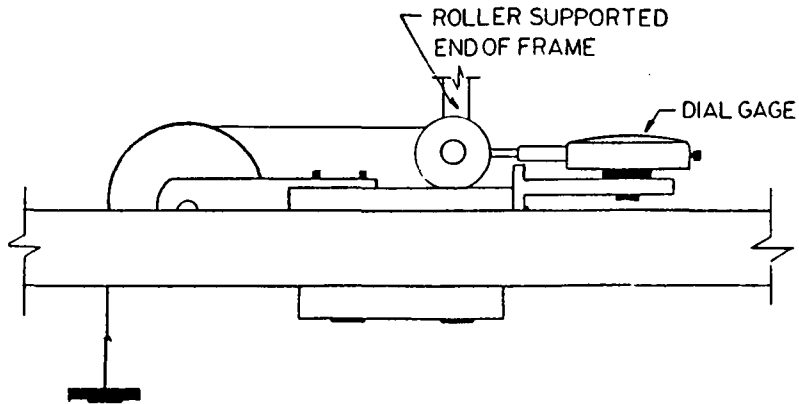


A. Fixed supported end of frame

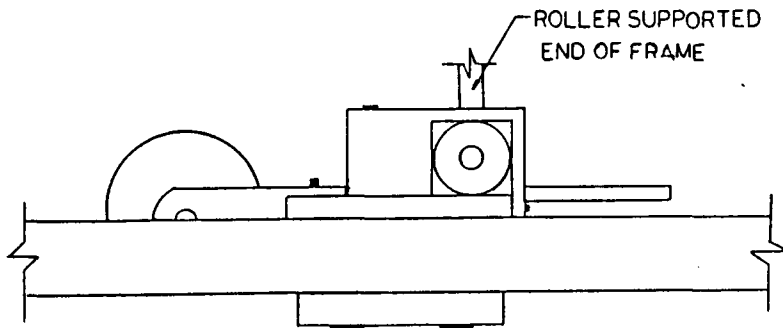


B. Pin supported end of frame

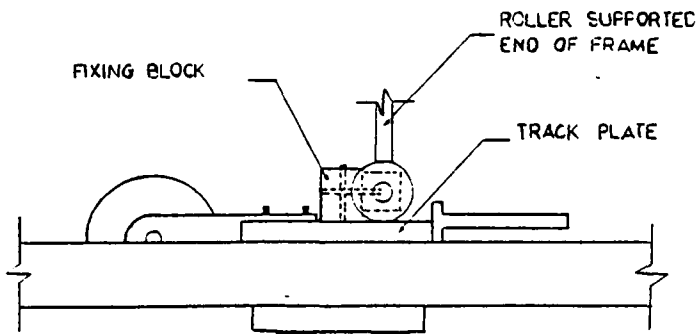
Fig. 2.4 Portal frame supporting fixtures



A. Roller supported end of frame



B. Pin supported end of frame



C. Fix supported end of frame

Fig. 2.5 Portal frame supporting fixtures

to prevent horizontal movement of the wheels to simulate a pin support.

## 2.5 Loading System

### 2.5.1 Concentrated load.

Cast iron slotted weights were used as concentrated forces. They were applied to the structure by two types of devices. One is knife-edge rigid hangers for concentrated gravity force application and the other is knife-edge stirrup and cord for applying point loads virtually in any direction. This is done by passing the cord through a pulley bracket mounted on the stand. Figure 2.6 shows such a device when it is used to apply sideway load to the model frame.

### 2.5.2 Distributed loads.

A number of steel blocks all with approximately the same weight (5.048 gr) and dimensions (73 mm x 73 mm x 12mm) were available in the lab. These blocks approximate a uniformly distributed load when placed close together on the beam model structure (Figure 2.7a).

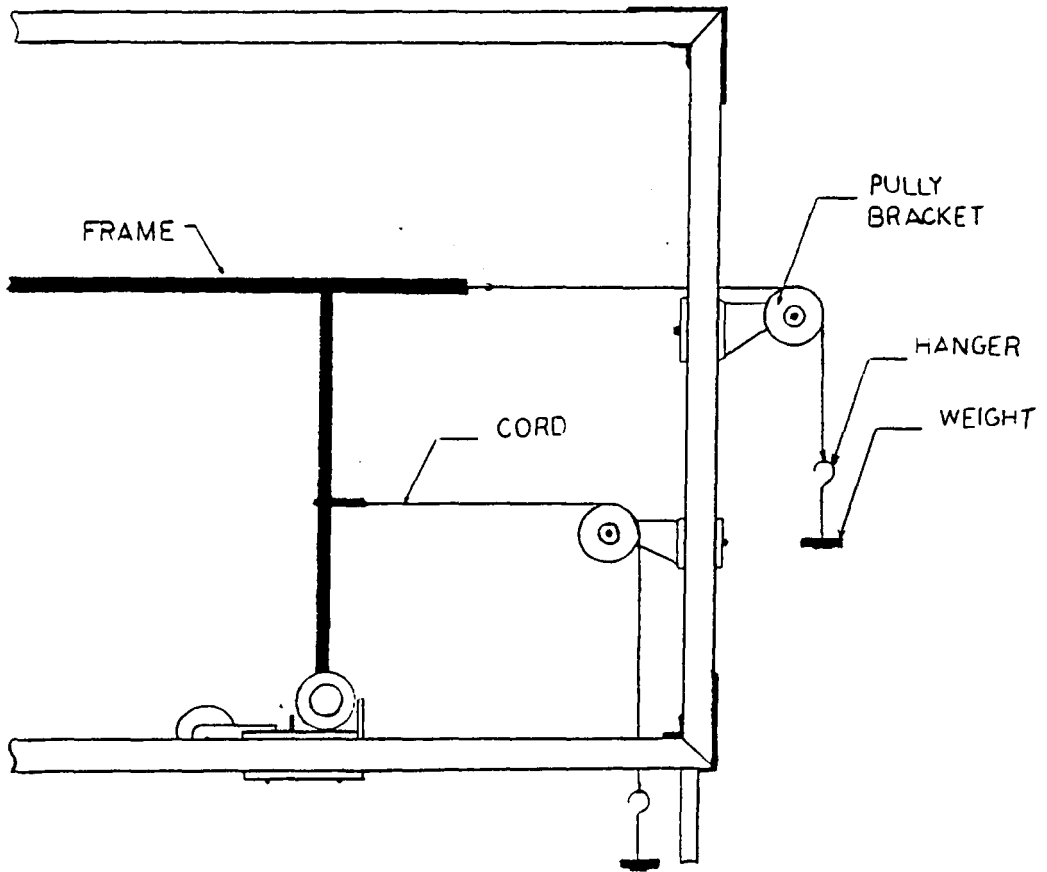
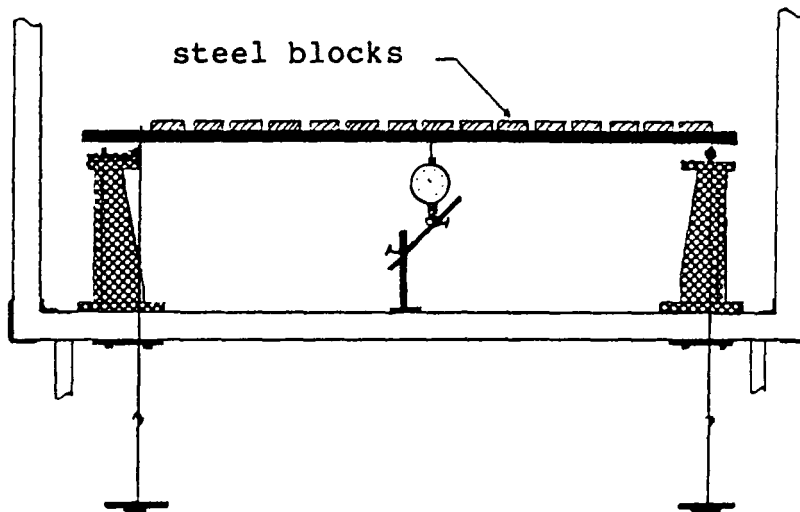
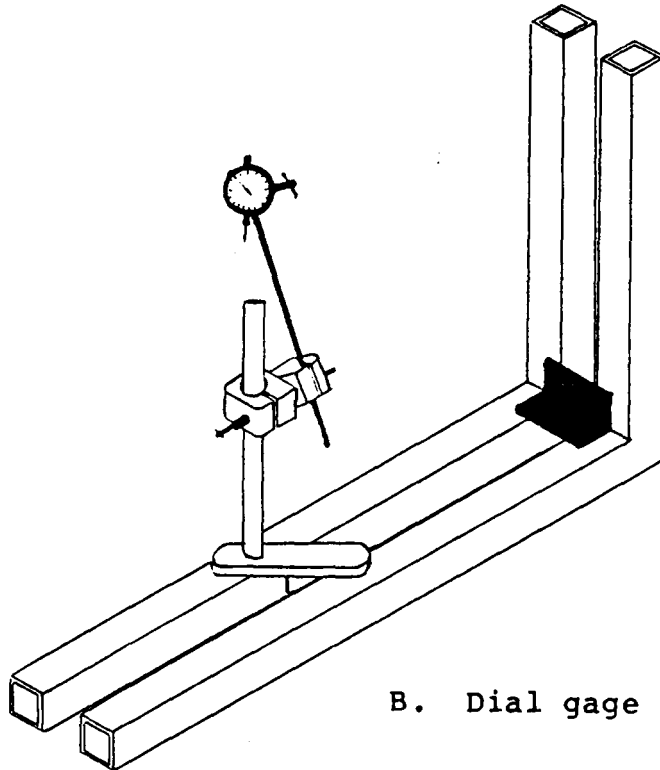


Fig. 2.6 Portal frame subjected to sideways loading



A. Uniformly distributed loading



B. Dial gage assembly

Fig. 2.7 Loading and measuring devices

## 2.6 Measuring Device

Three mechanical dial gages were used to measure the deflection of the structures. All three are graduated in 0.01 mm increments. Two of the gages have 12 mm travel and the third one can measure movements up to 25 mm.

The gages are supported on an arm mounted to a rigid post by means of a clamp. The post is fastened to the universal frame by a base plate (Figure 2.7b). By adjusting the clamp and the base plate, direction and the position of the indicators can be changed easily along entire structure.

## CHAPTER III

### TESTING PROCEDURE

#### 3.1 Introduction

The manufacturer's catalog (8) gives several figures showing set-ups for a variety of experimental tests, but it does not specify any particular testing procedure. To develop a testing technique, several separate tests were conducted on the beam and the frame. During testing, some of the components within the apparatus, particularly supporting fixtures, did not function as intended. Therefore, a few alterations were needed to improve the resulting data. In the following sections, the nature of such malfunctions are emphasized as well as improvements that were made on the apparatus. Due to similarity in testing procedures, just one typical case for each structural model is presented.

#### 3.2 Beam Deflection

Case presented here is a two-span continuous beam fixed at both ends and resting on a roller at the middle (Figure 3.1).

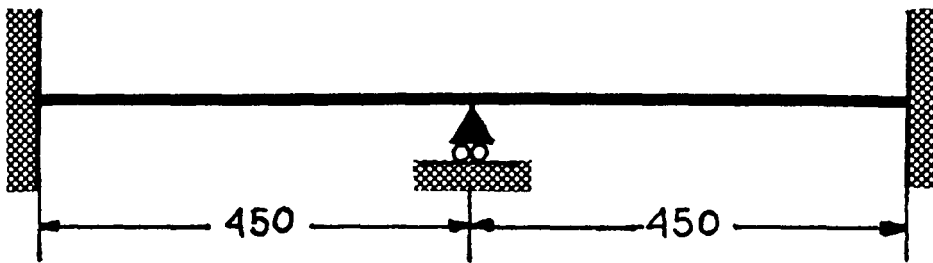


Fig 3.1 Two spans continuous beams (Unit - mm)



### 3.2.1 Set-up Procedure

Three support brackets were bolted to the rigid frame at equal distances. Beam was clamped to the brackets using clamp plate (Figure 2.2b) at one end and roller fixture (Figure 2.2c) at the other end. Due to the bowed shape of the beam, there was no contact between the beam and the cylindrical shaft used as roller support at the middle. When the beam was inverted, the shaft had to be forced in place at the middle. It was also observed during testing that the roller support can not assume a stable position and it is moving when the beam is subjected to load. To prevent such movement, some forces were applied at the support point to secure the position of the shaft. The technique used, is shown in Figure 3.2. Later on, this method had to be used in almost all experiments with the roller or knife-edge supports since some other problems such as uplifting of the beam at the end were observed. An example is shown in Figure 3.3. In the case under consideration, two dial gages were used to measure the deflection at two midspans of the beam.

### 3.2.2 Loading Technique

As suggested by the manufacturer, slotted cast iron weights were directly hung on the structure using knife-edge hangers as vertical concentrated forces. A difficulty encountered here was movement of the hangers

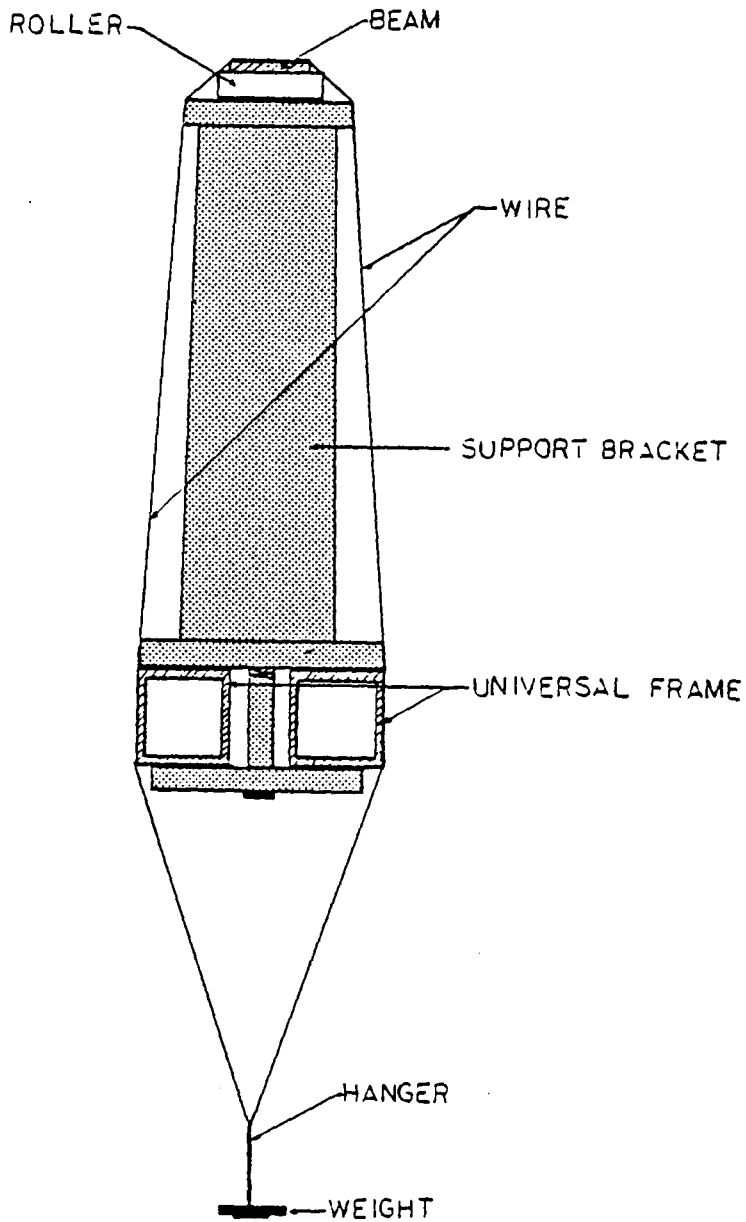
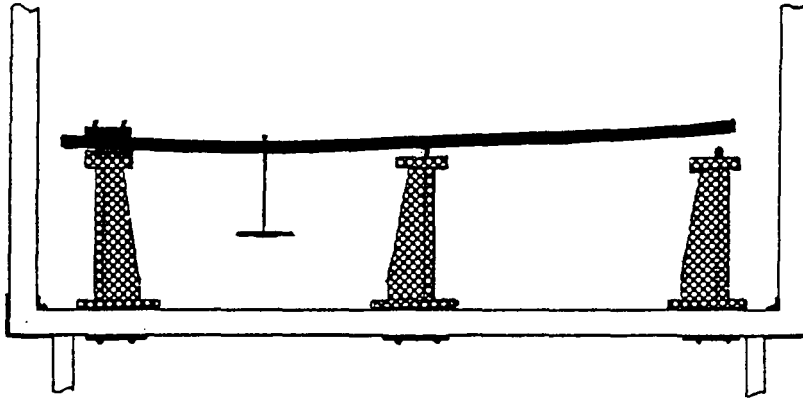
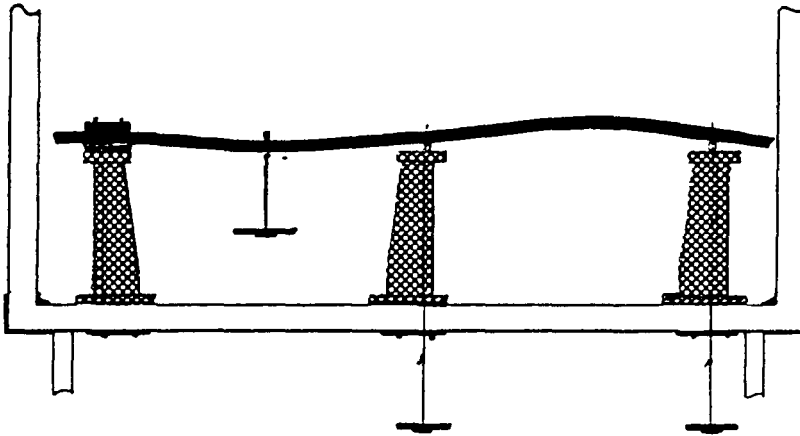


Fig. 3.2 Stabilizing force technique



A. Uplift at one end



B. Method to prevent uplift

Fig. 3.3 Continuous beam test set-up

when the weights were placed or removed. This caused inconsistencies in dial gage readings because of the change in application point of the load. This was quite noticeable when dial gages were placed directly on the top of hangers to measure deflection at the loading point. More stable readings were taken when weights were applied indirectly, using a cord attached to the structure at the point of loading. This cord was passed through a double pulley mounted on the testing frame and loads were applied at the lower end of the cord (Figure 3.4).

This method caused less physical dislocation of the loading point. Since just one of these pulley brackets was available it could not be used for the cases with more than one concentrated force.

As mentioned in section 2.5.2, several steel blocks were available in the lab for use as uniformly distributed loads on the structures. These blocks were placed directly on the beam with equal distances between them. The length of these distances was dictated by the length of the beam and the maximum number of blocks that would cover the structure in one row. Besides the length limitation, these spaces between the blocks are necessary since contact between them would add to the rigidity of the structure and prevent the structure from free deformation. Each block was individually weighed, and their weight distribution is shown in Figure 3.5. Their mean value of 5.048 gr. was used in the theoretical analysis.

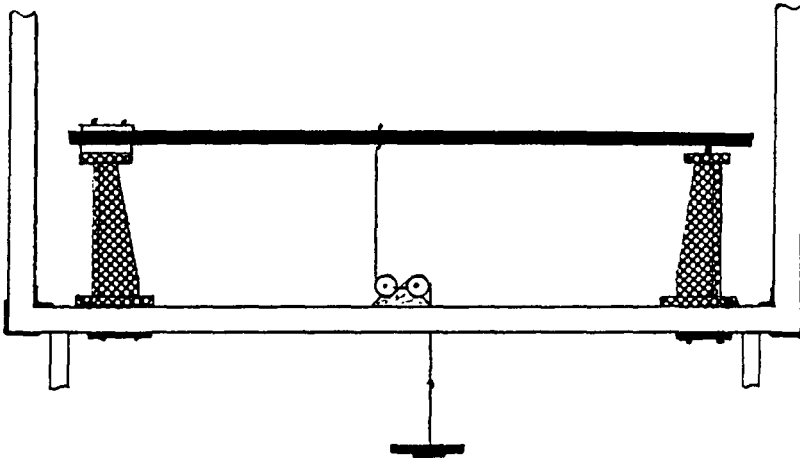


Fig. 3.4 Indirect application of concentrated force

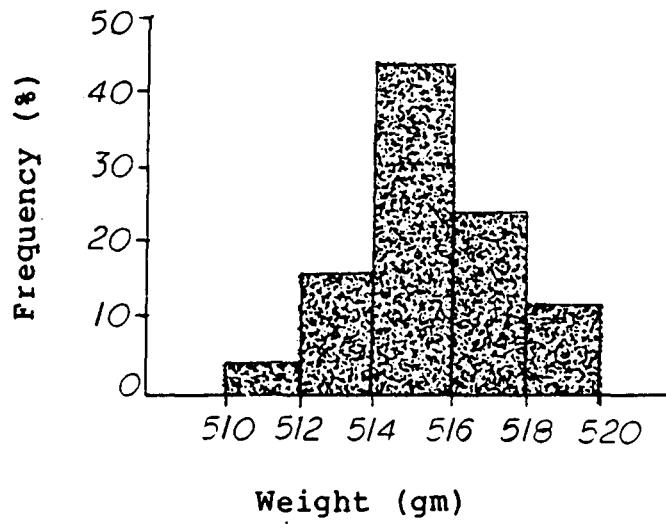


Fig. 3.5 Weight distribution histogram for steel blocks

### 3.2.3 Data Reading

Each experiment was repeated three times and readings were taken before and after the loads were applied. The difference between the two readings was recorded as deflection at those points. The average of three experimental tests was used as the final result.

Data for all of the tests on the beams are tabulated for each of the structural arrangements in Appendix A.

## 3.3 Elastic Deflection of Portal Frame

The case to be studied here is a typical portal frame supported on a knife-edge (pinned) at one end and standing on a roller at the other end (Figure 3.6).

### 3.3.1 Set-up Procedures

The frame was placed on two support fixtures, a knife-edge bracket of Figure 2.4b and a track plate assembly of Figure 2.5a which were mounted on the rigid frame. Due to manufacturing imperfection, the ball bearings on the roller end of the frame did not bear evenly on the track plate. A paper shim placed between the track plate and the rigid frame (Figure 3.7) corrected this problem. Three dial gages were used. One was placed on left end of the beam to measure the amount of sidesway that the frame undergoes. A second one was placed to measure



Fig. 3.6 Pin-Roller supported portal frame



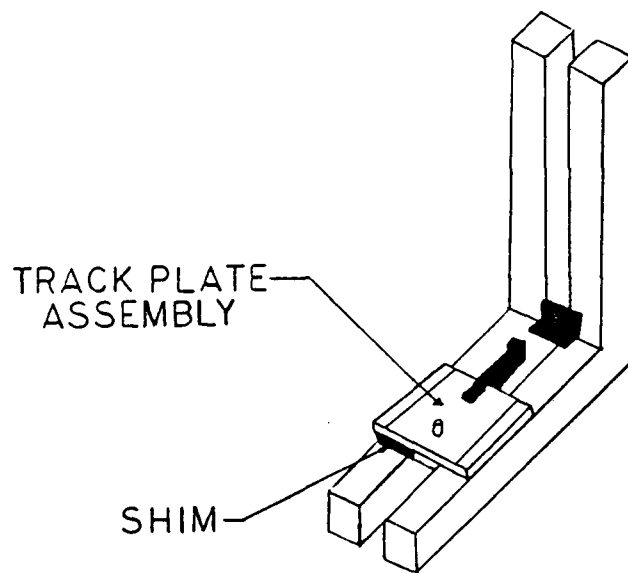


Fig. 3.7 Using shim to obtain even bearing on the support

the deflection of the beam at midspan and the third one was mounted on the track plate to measure the horizontal movement of the frame at the roller support.

### 3.3.2 Loading Technique

As in the case of the beam, cast iron weights and steel blocks were used as concentrated and uniformly distributed loads. Again a problem occurred when concentrated vertical forces were hung directly on the structure. The contact point between the hanger and the structure moved when the weights were placed and the effect was minimized if the loads were applied indirectly by a cord and pulley bracket. The end of the frame supported on knife-edge brackets lifted upward due to sideways loads. A similar technique as for the beam was used here to hold the frame intact at its support without inducing any moment in the structure. The arrangement is shown in Figure 3.8.

### 3.3.3 Data Collection

Each experiment was performed three times and readings were taken before and after application of the forces. The difference of two readings in each experiment was recorded. The average of three test results was taken as the final experimental deflection value. The results of the several tests performed on the frame are presented in Appendix B in tabulated form.

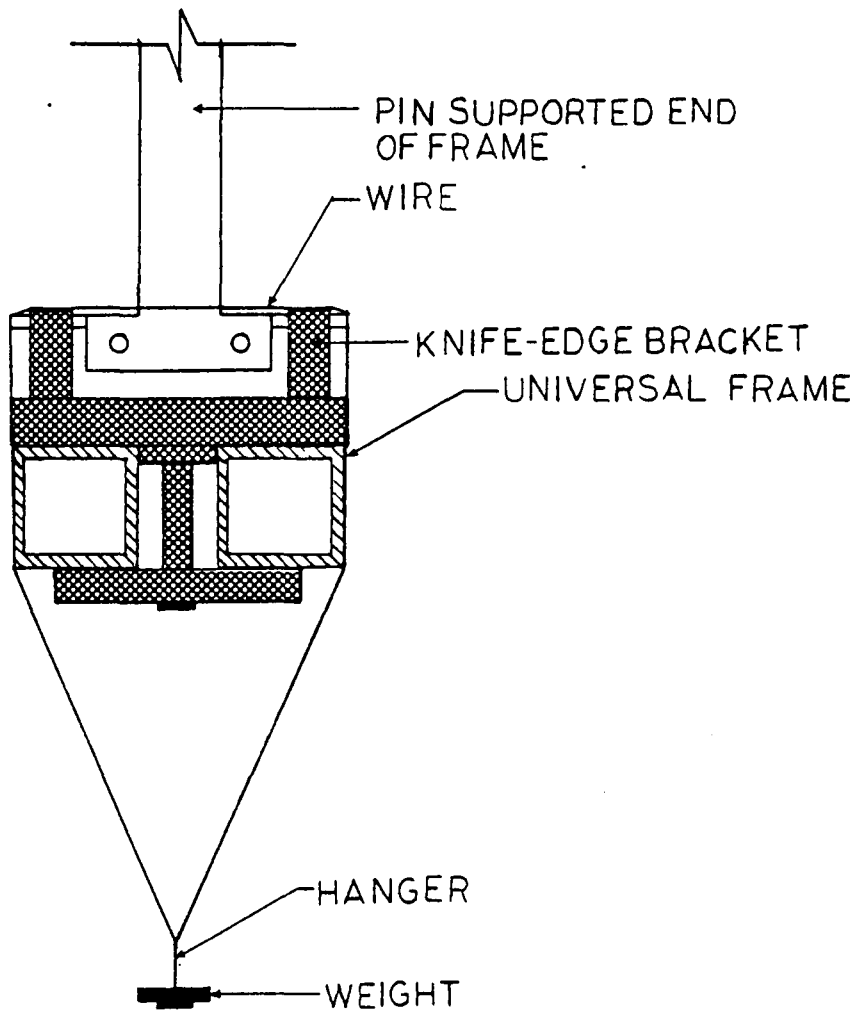


Fig. 3.8 Stabilizing force to prevent uplifting of the frame

Two major problems were encountered during testing of the frame: first, poor degree of repeatability and second, relatively large lateral displacements. The main reason for the first was the friction between the ball bearings and the track plate assembly. The instruction manual (7) prepared by the manufacturer suggests adding some stabilizing forces to the frame (without specifying any particular point on the frame) and tapping the apparatus to minimize the frictional effects. It was observed that adding such forces along with tapping of the apparatus lightly prior to taking any reading each time, improved the results to a certain degree.

The second problem is caused by the portal frame undergoing some side-sway due to any nonsymmetrical situation, whether this is a nonsymmetrical loading on a symmetrically supported frame or a symmetrical loading of a frame supported nonsymmetrically. Because the position of the dial gages are fixed, the dial indicator that is measuring vertical deflection of the beam is no longer at midspan of the beam after deformation. This was a major contributing factor to errors involved at those points.

## CHAPTER IV

### PROGRAM DEVELOPMENT

#### 4.1 Introduction

In this chapter, the development of a computer program for the computation of reactions and deflections for a continuous beam with constant cross section is presented.

#### 4.2 Theoretical Background

The computation is based on Clebsch's Method (1862) (4). This method gives an exact solution of the Euler-Bernoulli beam equation which exhibits clearly and concisely the contribution of different types of loading to the deflection curve. In addition to the name, Clebsch's method, the various related forms of this approach are often called singularity or step function methods.

#### 4.3 Mathematical Presentation

Solution for the slope and deflection of an elastic beam with uniform cross section and subjected to sets of uniformly distributed loads  $w_i$  ( $i = 1, 2, \dots, n$ ) and concentrated loads  $P_j$  ( $j = 1, 2, \dots, n$ ) is readily obtained by direct integration of the Euler-Bernoulli

equation if it is written as a continuous function of  $x$  in the form of:

$$EIy'' = M \quad (4.1)$$

where:

$$M = H \langle x, a \rangle, f(x, a) \quad (4.2)$$

$$H \langle x, a \rangle = \begin{array}{ll} 0 & \text{If } x \leq a \\ 1 & \text{If } x > a \end{array}$$

Also  $M$  is bending moment due to the different types of loading taken at any point of the beam,  $E$  is Young's modulus, and  $I$  is the moment of inertia of the cross section about the neutral axis.

Integrals of Equation (4.1) yield the slope and deflection curves while the differential of Equation (4.1), evaluated at any point on the beam gives the shear at that point.

$$EIy' = \int_0^x Mdx + C1 \quad (4.3)$$

$$EIy = \int_0^x \int_0^x Mdx^2 + C1x + C2 \quad (4.4)$$

$$EIy''' = V = dM/dx \quad (4.5)$$

There are two constants of integration that can be evaluated by employing the beam's boundary conditions.

To illustrate the application of the above equations, the following example is presented. The case considered is a n-span beam with constant cross-section and fixed at both ends. It is subjected to k sets of uniformly distributed loads and m concentrated loads (Figure 4.1).

In the following equations L denotes the span length and subscript (j) refers to span number. The first span starts from left end of the beam. To be able to show the equations in general form and have a better analogy with the program, L with the subscript zero in all the following equations is zero.

Bending moment as a function of x is:

$$\begin{aligned}
 M = & -Ml(x)^0 + \sum_{i=1}^n R_i H \left\langle x - \sum_{j=0}^{i-1} L_j \right\rangle \left( x - \sum_{j=0}^{i-1} L_j \right) \\
 & - \sum_{i=1}^m P_i H \langle x - a_i \rangle (x - a_i) - \\
 & \sum_{i=1}^k \frac{W_i}{2} H \langle x - b_i \rangle (x - b_i)^2 + \sum_{i=1}^k \frac{W_i}{2} \langle x - e_i \rangle (x - e_i)^2 \quad (4.6)
 \end{aligned}$$

By Equation (4.1)

$$\begin{aligned}
 EI \frac{d^2 y}{dx^2} = & -Ml(x)^0 + \sum_{i=1}^n R_i H \left\langle x - \sum_{j=0}^{i-1} L_j \right\rangle \left( x - \sum_{j=0}^{i-1} L_j \right) - \sum_{i=1}^m P_i H \langle x - a_i \rangle \\
 & \left( x - a_i \right) - \sum_{i=1}^k \frac{W_i}{2} H \langle x - b_i \rangle (x - b_i)^2 + \sum_{i=1}^k \frac{W_i}{2} \langle x - e_i \rangle (x - e_i)^2 \quad (4.7)
 \end{aligned}$$

First integration of Equation (4.7) gives the slope equation and by second integration the equation of deflected curve is obtained.

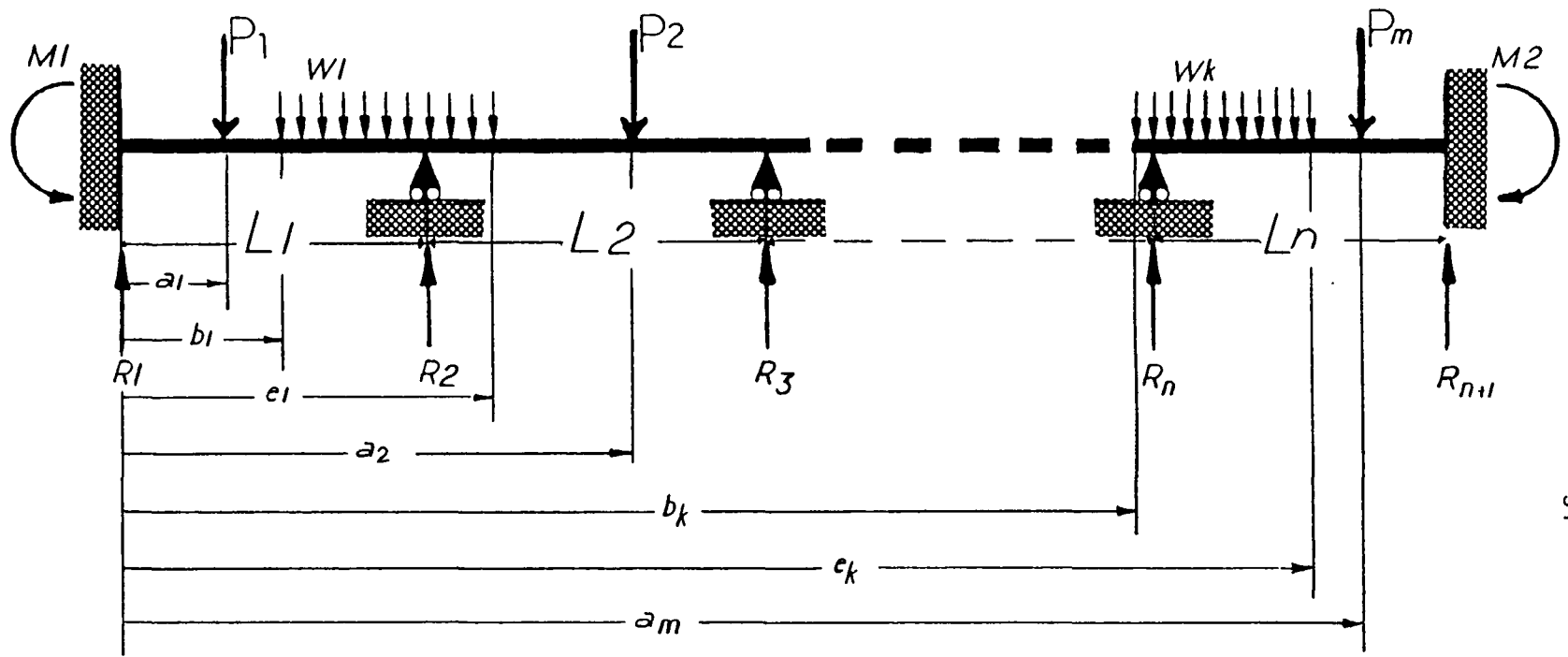


Fig. 4.1  $n$ -spans continuous beam



$$EI \frac{dy}{dx} = -Ml(x) + \sum_{i=1}^n \frac{R_i}{2} H \langle x - \sum_{j=0}^{i-1} L_j \rangle (x - \sum_{j=0}^{i-1} L_j)^2 - \sum_{i=1}^m \frac{P_i}{2} H \langle x - a_i \rangle (x - a_i)^2 - \sum_{i=1}^k \frac{W_i}{6} H \langle x - b_i \rangle (x - b_i)^3 + \sum_{i=1}^k \frac{W_i}{6} H \langle x - e_i \rangle (x - e_i)^3 + C1 \tag{4.8}$$

$$EIy = -\frac{Ml}{2} (x)^2 + \sum_{i=1}^n \frac{R_i}{6} H \langle x - \sum_{j=0}^{i-1} L_j \rangle (x - \sum_{j=0}^{i-1} L_j)^3 - \sum_{i=1}^m \frac{P_i}{6} H \langle x - a_i \rangle (x - a_i)^3 - \sum_{i=1}^k \frac{W_i}{24} H \langle x - b_i \rangle (x - b_i)^4 + \sum_{i=1}^k \frac{W_i}{24} H \langle x - e_i \rangle (x - e_i)^4 + C1x + C2 \tag{4.9}$$

To solve Equation (4.9) there are (n+3) unknowns:

Ml	1
$R_i (i=1, 2, \dots, n)$	n
C1 & C2	<u>2</u>
	(n+3)

Also there are (n+3) boundary conditions for this case:

at x = 0	dy/dx = 0	1
at x = $L_i (i=0, 1, 2, \dots, n)$	y = 0	n+1
at x = S	dy/dx = 0	1
		(n+3)

where  $S = \sum_{i=1}^n L_i$

By employing these boundary conditions in Equations (4.8) and (4.9) sufficient equations are in hand to evaluate the unknowns. A program has been written to generate the system of equations from specific loads and boundary condition. These equations are then solved by using a subroutine from CMS called LEQTLF. Finally the values for the unknowns are substituted in Equation (4.9) to get the value of deflection at the desired points.

#### 4.4 Program Design

The program structure is presented in Figure 4.2. The program listing presented in Appendix C uses a WATFIV compiler. Since the program is meant to execute interactively (reads and writes on the terminal on some parts), a three line subprogram is needed to set the read and write statements with device numbers other than 5 and 6 (which reads from the file and writes on listing file). This execute file is listed at the end of the program.

#### 4.5 Specifications

Beams are defined according to cartesian coordinates and the starting point of the beam is at the origin of this coordinate. All the distances are measured from the origin. Moments are positive when the upper fiber of the beam is in compression and upward reactions are positive.

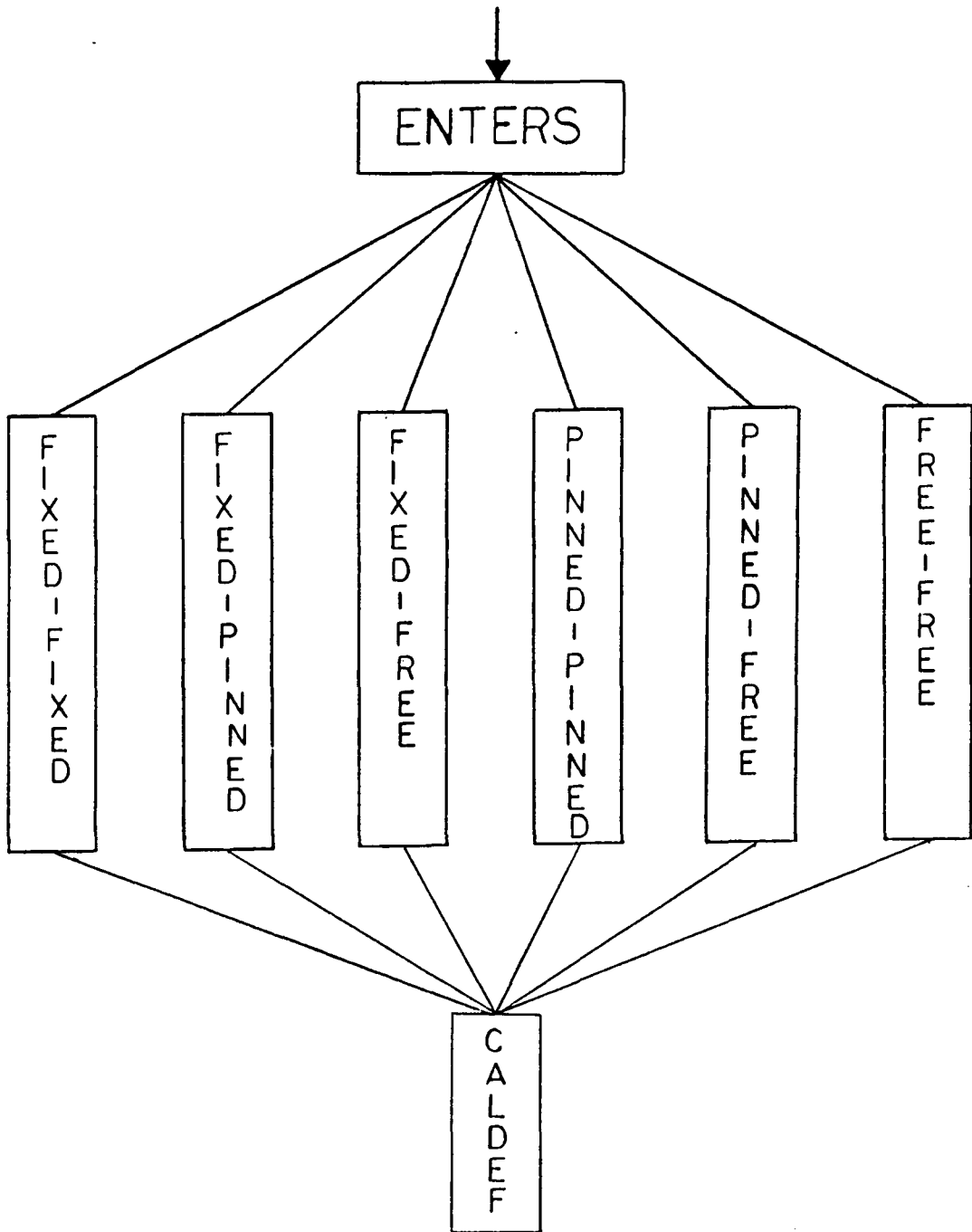


Fig. 4.2 Program structure.

Deflections are positive in the direction of the positive axis.

#### 4.6 Limitations

4.6.1 Beam should have uniform cross section with no hinges or points of discontinuity. Since the solution is based on simple beam theory, the axial deformation is neglected.

4.6.2 Only two types of loads are considered: 1) uniformly distributed loads, and 2) concentrated vertical forces.

4.6.3 The size of arrays in the main program and subroutines are arranged for a maximum of 30 uniform loads and concentrated forces and a maximum of 25 spans. Therefore, for larger sizes the array dimensions have to be increased.

#### 4.7 Units

All the units are in metric system.

Displacements	mm
Length	mm
Force	newton
Uniform loads	newton/mm
Moments	newton*mm

Moment of inertia       $(\text{mm})^4$   
 Young's modulus         $\text{n/mm}^2$

#### 4.8 Parameters

A(I)                    Distance of force (I) from origin  
 BG(I)                   Distance of starting point of W(I) from  
                                  origin support condition  
 DELX                    The interval on the beam where  
                                  deflection is computed  
 E(I)                    Distance of ending point of W(I) from  
                                  origin  
 EM                      Value of Young's modulus  
 ICONT                   Dummy variable to specify the first and  
                                  last support condition  
 IM                      Moment of inertia  
 L(I)                    Length of span (I)  
 N                        Number of spans in the beam  
 NP                      Number of concentrated loads  
 NW                      Number of uniformly distributed loads  
 P(I)                    Value of force (I)  
 W(I)                    Value of uniformly distributed load (I)

## CHAPTER V

### DISCUSSION OF RESULTS

#### 5.1 Introduction

In this chapter the basic goal is the comparison of the theoretical data with the experimental results obtained from the structural models tested. The discussion will center around the validity and accuracy of the findings and will draw attention to the major elements which contribute to some of the differences existing between the results found experimentally and mathematically. In general all the experiments have been performed within the elastic range of the material for two reasons: first, to conserve the model for possible future use, and second, to enable the use of conventional beam theory for mathematical analysis. The following discussion has been divided into two main sections. One is devoted to the beam experiments and the second one to the findings from the portal frame.

## 5.2 Deflection of the Beam

Beams were tested using twelve different supporting arrangements. Theoretically they were analyzed with the use of computer program described in Chapter Four. The results obtained from model structures and the mathematical analysis are compared and given in Appendix A. Generally, it was found that the theoretical values of the deflections were lower than deflections measured on the actual structures and the differences are in the range of 8 to 24 percent.

Theoretically there is an argument that the use of classical beam theory would not consider the contribution of shear to the deflection curve and therefore the theoretical models are stiffer compared to their experimental counterparts. Timoshenko in his book "Theory of Elasticity" (9) examines the effect of both bending and shear to the deflected shape of a simply supported beam subjected to a concentrated force at its midspan. He arrives at the following equation:

$$\Delta = \frac{PL^3}{48EI} + \frac{PL}{4C} \left( \frac{3}{4G} - \frac{3}{10E} - \frac{3\nu}{4E} \right) - .21 \frac{P}{E} \quad (5.1)$$

where,

$$G = \text{shear modulus} = \frac{E}{2(I + \nu)}$$

$\nu$  = Poisson's ratio

C = distance between the neutral axes and the extreme fiber

P = value of concentrated force

E = modulus of elasticity

I = moment of inertia

L = length of the span

Using .3 for  $\nu$  in Equation (5.1) it was found that shear contribution is about 0.2% of total deflection for the simple beam used in the tests.

In another case the effect of the shear was considered on a cantilever beam subjected to a concentrated force at its free end. The following equation is shown to be approximately valid (3):

$$\Delta\nu = 1.6 \left( \frac{L^2}{h} \right) \Delta m \quad (5.2)$$

where

$\Delta\nu$  = deflection due to shear

$\Delta m$  = deflection due to bending

L = length of the beam

h = height of cross section

Error encountered in deflection value if shear contribution is neglected has been examined for several beams with different (L/h) ratios. The results are presented in Table 5.1. The conclusion is that for the beams with (L/h) ratio greater than 20 the deflection caused by the shear is not



TABLE 5.1:

L/h	$\frac{\Delta \nu}{\Delta m} \times 100$ (Percent)
1	N.A.
3	7
5	2.5
7	1.3
10	0.6
20	0.2

significant. For the cases tested the minimum L/h ratio is 59. Therefore the use of classical beam theory for the purpose of this study is valid.

Another factor that has significance in theoretical computations is the modulus of elasticity of the material. The manufacturer (6) suggests to use  $30 \times 10^6$  psi for the value of E. This value in general has variations and one way to calculate it is to substitute the experimental value of deflection into the bending equation for a simply supported beam. The value comes out to be about  $28.8 \times 10^6$  psi. This value was used in the entire theoretical analysis of the beam.

In order to examine the source of errors the structures were loaded in such a fashion that deflections were computed through the use of superposition. It was found that the summation of the results obtained from the beam subjected to individual loads are approximately the same as those of all loads applied simultaneously (differences are about 1%). This indicates that the errors are related to physical characteristics of the actual structure or to the actual measurements.

#### 5.2.1 Physical Characteristics

Since the percent errors increased significantly when the beam was fixed, there is speculation that fixing brackets fail to prevent the beam from

rotation. The effect can be noticed clearly when the beam is supported on a fixed clamp at one end and a roller at the other. The error increases by 6% as the load is applied closer to the fixed support. One of the two fixing brackets supplied by the manufacturer is equipped with internal rollers that is intended to allow axial movement to the beam. This one is even more flexible compared to the previous one and percent error increases to 30% at the points near this support. The above cases show systematic errors that can be accounted for if proper instruments are employed to measure amount of rotation taking place at these supports.

### 5.2.2 Errors due to Physical Measurements

The components measured physically are the beam dimensions, the weights, and the deflections. The cross sectional dimensions of the beam were measured at ten points along the beam with a caliper. The accuracy of the measuring device was 0.5 mm. The average values were used for calculating cross sectional area and moment of inertia. The cast iron slotted weights were weighed with a scale with an accuracy within 0.1 gram. The actual reading of deflections was taken using a mechanical dial gage with an accuracy within 0.01 mm. The uncertainty in physical measurements using a method

(1) which is explained in Appendix E came out to be about 3% of the theoretical values calculated for the deflections.

### 5.3 Deflection of the Frame

The portal frame described in Chapter Two was tested with six different support conditions. Frames were theoretically analyzed using STRUDL. The listing of program and its output is given in Appendix F. The experimental data are compared with theoretical results in Appendix B.

In general three dial gages were used to measure the deflections at three points on the frame. Referring to the tables in Appendix B, the letters with the subscript v denotes the vertical deflection and with the subscript h denotes the horizontal movement of the point. The experimental data show that horizontal deflection results are consistently more accurate compared to vertical deflections. This was because the dial gage is not pointed at the same point before and after deformation. Errors involved in horizontal deflections were in the range of zero percent for the fix-fix case to 13 percent for the pin-roller supported frame. As in the case of beams, the loads have been applied consistently at the positions so that the results can be superimposed. The principle of

superposition holds for horizontal deformations within 3 percent. Generally, as the supports get stiffer the error percentages decrease. The highest error of 13 percent is obtained from a pin-roller support condition.

Instruction manuals (6 & 7) warn against the friction that might take place between the ball-bearings and the track plate. They suggest using stabilizing forces on the frame or lightly tapping the stand in order to minimize the effect of friction. This was examined in two cases. One was with pin-roller support conditions and a second was with fixed-roller support conditions. In each case several readings were taken with and without stabilizing forces. The dial gages were zeroed and a total of 26 readings were taken just after the stand was lightly tapped.

The pin-roller supported frame was observed to behave in the following manner. After tapping, the roller support did not return to zero. Without any load on the system, the highest deviation from zero was 0.08 mm on one occasion but the most frequent deviation was 0.06 mm from zero (35 percent of the time). When two horizontal stabilizing forces were applied at the right corner and roller abutment, the deviation decreased to 0.04 mm with a frequency of 70 percent.

In the case of fixed-roller supports, without any load on the system, the most frequent deviation of the roller from zero was 0.02 mm (frequency of 46 percent). The roller did not show any deviation when the two horizontal stabilizing forces were added to the frame.

Lack of sensitivity in dial gages had a major role in contributing to the differences between the experimental and theoretical analysis. Although theoretical results were given by the computer to six significant figures, they were rounded off to 2 significant figures because the accuracy of the gages was only 0.01 mm. The experimental results show that small deflections up to order of 0.09 mm are susceptible to high percentage of error. This is to be expected because of the limited accuracy of the dial gages. Other contributing elements to the differences were manufacturing imperfection of the model such as crookedness of welded roller block relative to the frame, difference in length of the overhanging part of the beam, and lack of complete rigidity in brackets.

## CHAPTER VI

### CONCLUSIONS AND RECOMMENDATIONS

This study presents the experimental techniques involved with the testing of two model structures and compares the results with theoretical predictions. In general the equipment is easy to use, adaptable to a variety of structural models, and is reasonably accurate. It meets the requirements of a qualitative structural teaching model, but falls short of providing accurate quantitative values for all boundary conditions.

#### 6.1 Conclusions

Results obtained throughout the experimental work and mathematical analysis leads to the following conclusions for each of the two models.

##### 6.1.1 Beam Bending Experiments

1. For beams with simple supports the experimental deflections are about 7 percent higher than the theoretical values.
2. For beams which contain fixed supports, the experimental deflection values are considerably

higher than the theoretical values. The inability of the testing apparatus to duplicate the idealized fixed condition is the main reason for the large difference.

3. The principle of superposition is experimentally demonstrated within an accuracy of about 3 percent.
4. The contribution of shear to the deflection is shown to be negligible and the classical beam theory can be used for the analytical model.
5. Application of vertical stabilizing forces to the beam when it is supported on knife-edge or roller prevents the uplifting and undesired movement of the beam at the supports.

#### 6.1.2 Portal Frame Experiments

1. The percent errors for horizontal deflections are smaller for the fixed-fixed support conditions than for non-fixed supports for pinned-roller supports.
2. For symmetrical loading and boundary conditions (no sidesway), vertical deflections compare reasonably well with the theoretical values.
3. For unsymmetrical loading or boundary conditions, sidesway occurs and vertical deflections read from a fixed position are highly inaccurate.



4. The principle of superposition generally holds for horizontal deflections within 3 percent accuracy.
5. Application of horizontal stabilizing forces to the portal frame minimizes the friction effect by reducing the no-load movement after tapping from 0.06 mm to 0.02 mm. In the case of fixed-roller supported frame, application of the stabilizing force reduces the no-load movement after tapping zero.
6. In general the experimental results show that small deflections up to order of 0.09 mm are susceptible to high percentage of error.

## 6.2 Recommendations

To improve and expand the testing capability of the experimental apparatus discussed in this study, a number of recommendations are made.

1. Models made of other material, such as aluminum or epoxy glass, should be substituted for the steel models so that larger deflections are produced when subjected to small loads.
2. To overcome the difficulties encountered with the vertical deflections of the portal frame when sideways occurs, the position of the dial gages should move along with the structure.

### 6.3 Future Studies

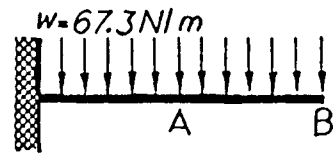
The scope of the work can be extended along the following directions:

1. By using a second parallel testing frame to provide additional support conditions, three dimensional space frame can be tested.
2. With the addition of an instantaneous deflections reading device, the use of the testing apparatus can be extended to problems in structural dynamics.

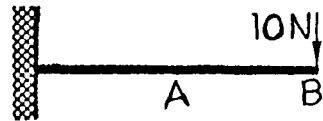
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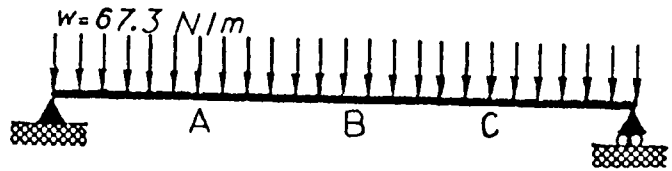
APPENDIX A  
BEAM TESTING RESULTS



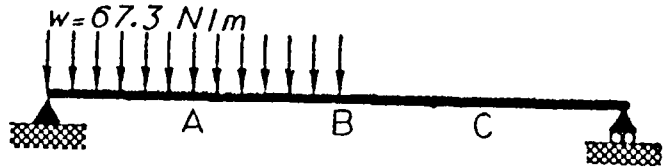
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-2.55	-2.21	15.4
B	-7.05	-6.23	13.2



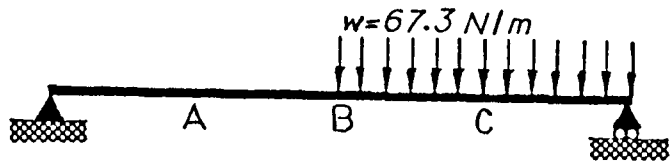
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-1.96	-1.71	14.6
B	-6.16	-5.48	12.4



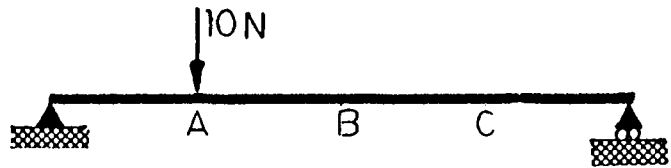
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-8.00	-7.40	8.1
B	-11.17	-10.38	7.6
C	-7.92	-7.40	7.0



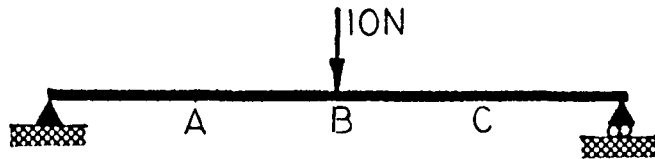
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-4.37	-4.02	8.7
B	-5.59	-5.19	7.7
C	-3.62	-3.37	7.4



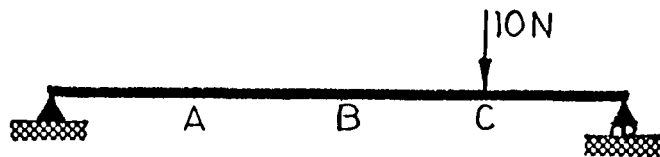
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-3.64	-3.37	8.0
B	-5.58	-5.19	7.5
C	-4.32	-4.02	7.5



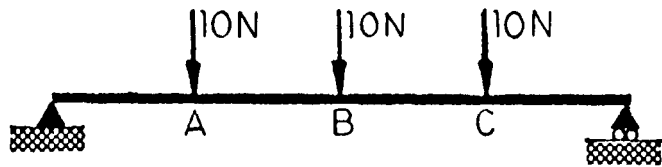
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-1.67	-1.54	8.4
B	-2.01	-1.88	6.9
C	-1.29	-1.20	7.5



Location	Experimental (mm)	Theoretical (mm)	% Error
A	-2.04	-1.88	8.5
B	-2.93	-2.74	6.9
C	-2.03	-1.88	8.0

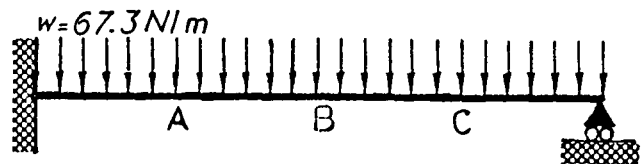


Location	Experimental (mm)	Theoretical (mm)	% Error
A	-1.28	-1.20	6.7
B	-2.01	-1.88	6.9
C	-1.66	-1.54	7.8

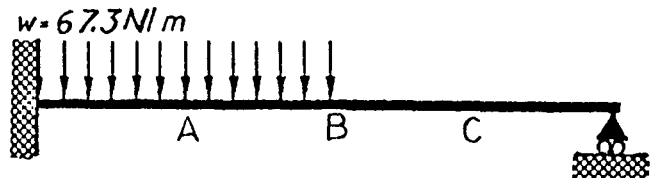


Location	Experimental (mm)	Theoretical (mm)	% Error
A	-5.00	-4.62	8.2
B	-6.96	-6.51	6.9
C	-4.98	-4.62	7.8

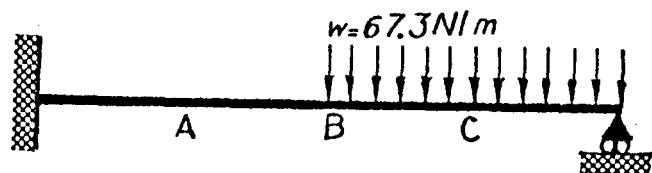




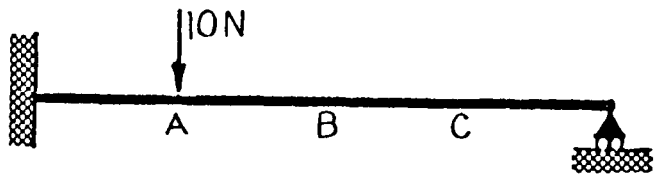
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-2.31	-1.95	18.5
B	-4.68	-4.15	12.8
C	-3.91	-3.50	11.7



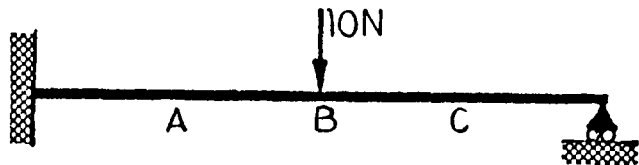
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-1.14	-0.96	18.7
B	-1.95	-1.69	15.4
C	-1.36	-1.18	15.2



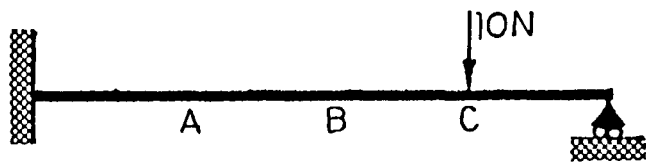
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-1.14	-0.99	15.1
B	-2.74	-2.46	11.4
C	-2.55	-2.32	9.9



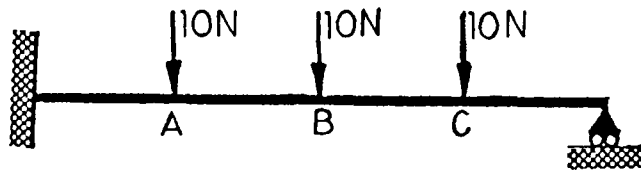
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.43	-0.36	19.4
B	-0.63	-0.53	18.9
C	-0.42	-0.36	16.7



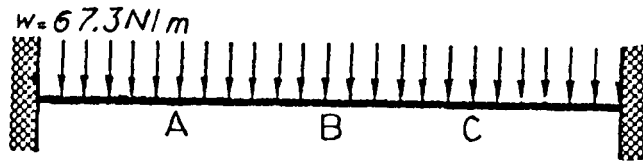
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.63	-0.53	18.9
B	-1.35	-1.20	12.5
C	-1.03	-0.92	11.9



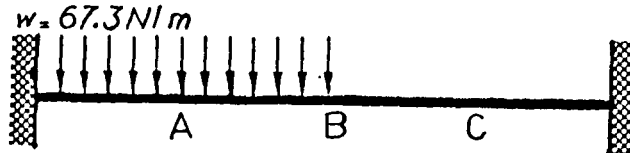
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.41	-0.36	13.9
B	-1.03	-0.92	11.9
C	-1.04	-0.94	10.6



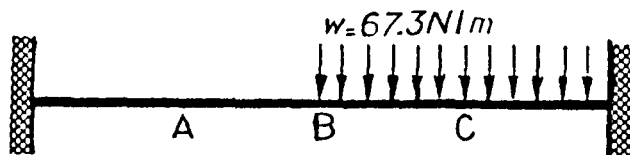
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-1.49	-1.25	19.2
B	-3.01	-2.66	13.1
C	-2.49	-2.22	12.2



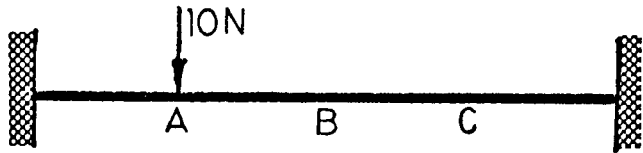
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-1.46	-1.16	25.9
B	-2.52	-2.08	21.1
C	-1.52	-1.16	31.0



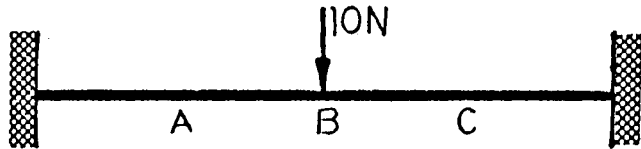
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.87	-0.71	22.5
B	-1.23	-1.03	19.4
C	-0.57	-0.45	26.7



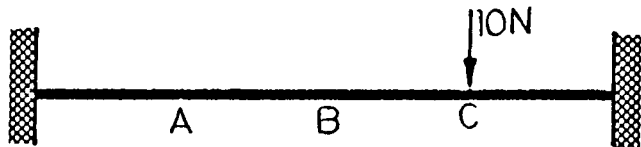
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.57	-0.45	26.7
B	-1.26	-1.03	22.3
C	-0.93	-0.71	31.0



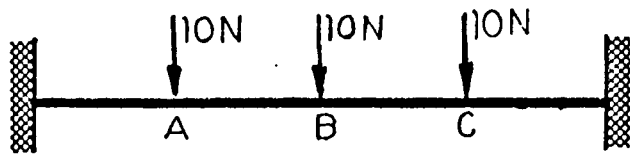
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.35	-0.29	20.7
B	-0.41	-0.34	20.6
C	-0.18	-0.14	28.6



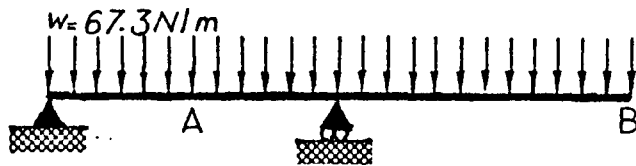
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.41	-0.34	20.6
B	-0.80	-0.69	15.9
C	-0.43	-0.34	26.5



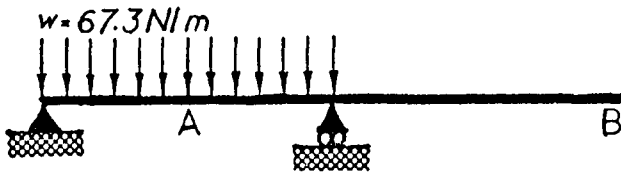
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.18	-0.14	28.6
B	-0.42	-0.34	23.5
C	-0.37	-0.29	27.6



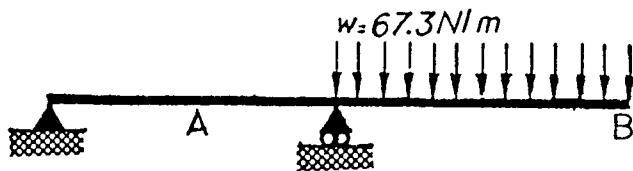
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.95	-0.77	23.4
B	-1.65	-1.37	20.4
C	-0.99	-0.77	28.6



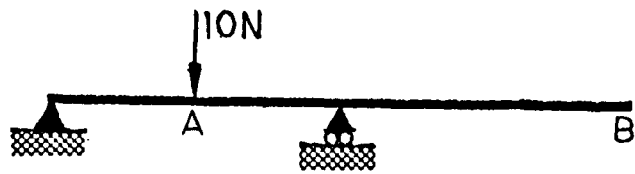
Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.97	0.91	6.6
B	-13.51	-12.46	8.4



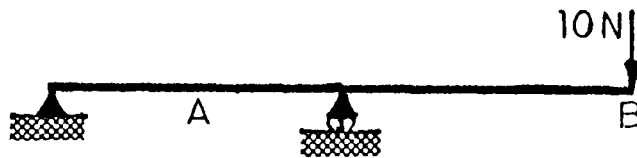
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.71	-0.65	9.2
B	2.10	2.08	1.0



Location	Experimental (mm)	Theoretical (mm)	% Error
A	1.70	1.56	9.0
B	-15.76	-14.53	8.5



Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.38	-0.34	11.8
B	1.0	1.03	2.9

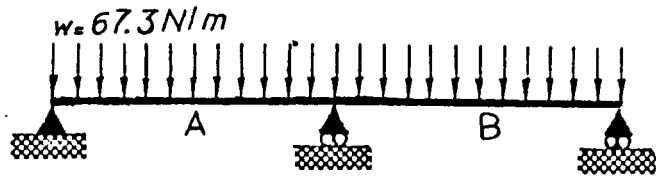


Location	Experimental (mm)	Theoretical (mm)	% Error
A	1.14	1.03	10.7
B	-11.86	-10.97	8.1

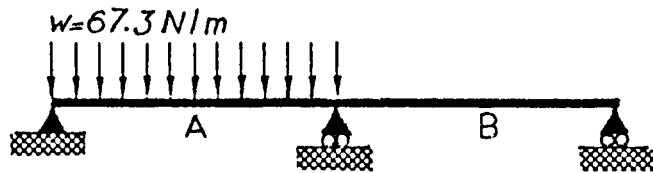


Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.76	0.68	11.8
B	-10.82	-9.94	8.8

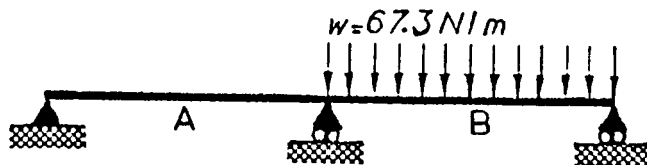




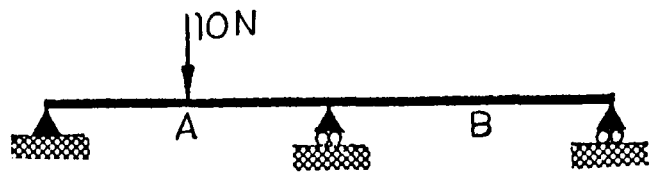
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.29	-0.26	11.5
B	-0.29	-0.26	11.5



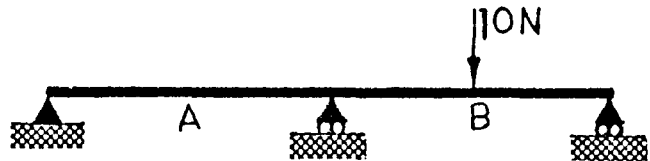
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.49	-0.45	8.9
B	0.21	0.19	10.5



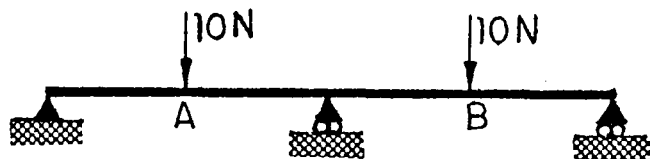
Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.22	0.19	15.8
B	-0.50	-0.45	11.1



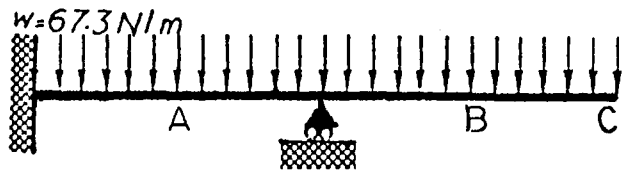
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.27	-0.25	8.0
B	0.10	0.10	0.0



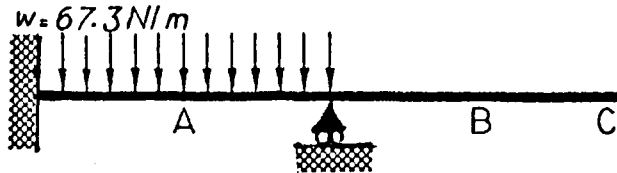
Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.11	0.10	10.0
B	-0.26	-0.25	4.0



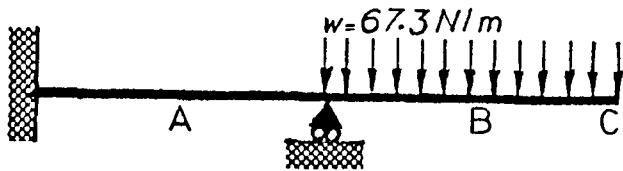
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.16	-0.15	6.7
B	-0.17	-0.15	13.3



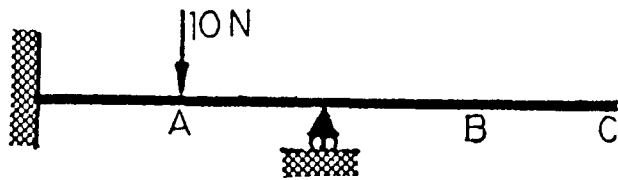
Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.56	0.52	7.7
B	-5.24	-4.80	9.2
C	-12.41	-11.42	8.7



Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.31	-0.26	19.2
B	0.53	0.52	1.9
C	1.04	1.04	0.0



Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.87	0.78	11.5
B	-5.81	-5.32	9.2
C	-13.57	-12.46	8.9



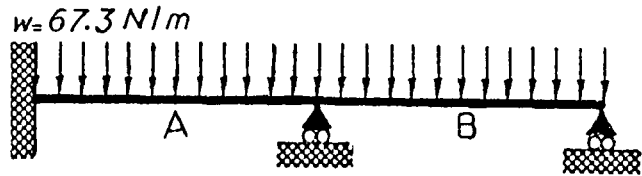
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.17	-0.15	13.3
B	0.24	0.26	7.7
C	0.46	0.51	9.8



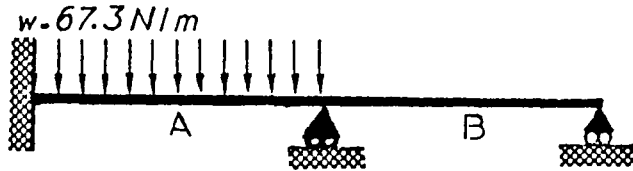
Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.56	0.51	9.8
B	-4.09	-3.77	8.5
C	-10.36	-9.60	7.9



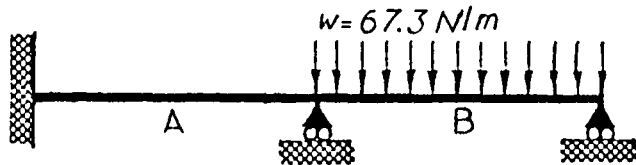
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.39	-0.36	8.3
B	-3.83	-3.51	9.1
C	-9.86	-9.08	8.6



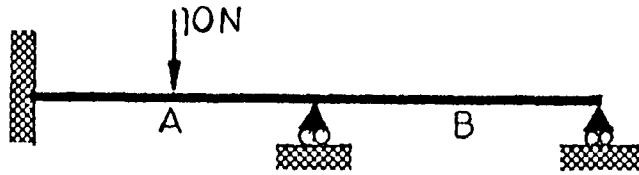
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.13	-0.09	44.4
B	-0.36	-0.31	16.1



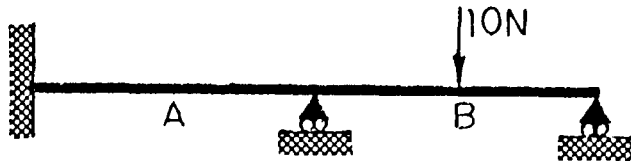
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.25	-0.2	25
B	0.11	0.11	0.0



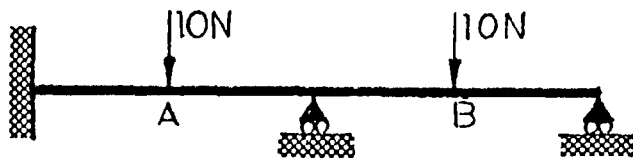
Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.11	0.11	0.0
B	-0.49	-0.43	13.9



Location	Experimental (mm)	Theoretical (mm)	% Error *
A	-0.14	-0.12	16.7
B	0.06	0.05	<del>20.0</del>

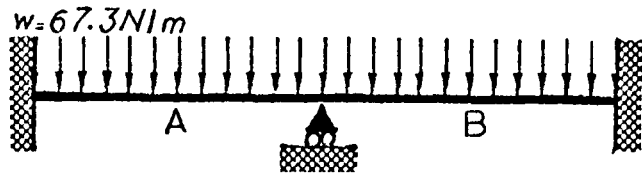


Location	Experimental (mm)	Theoretical (mm)	% Error *
A	0.06	0.05	<del>20.0</del>
B	-0.27	-0.23	17.4

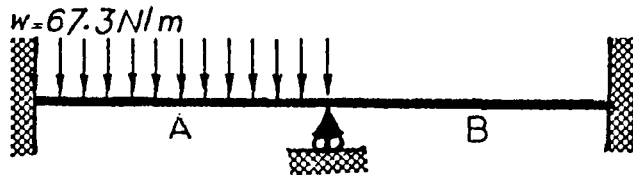


Location	Experimental (mm)	Theoretical (mm)	% Error *
A	-0.08	-0.07	<del>14.3</del>
B	-0.20	-0.18	11.1

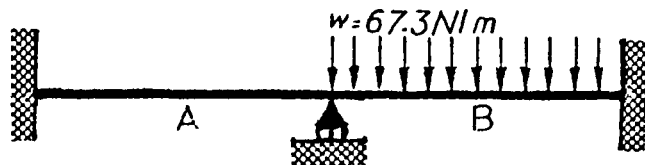
\* For measurements less than 0.09 mm, % error not given.



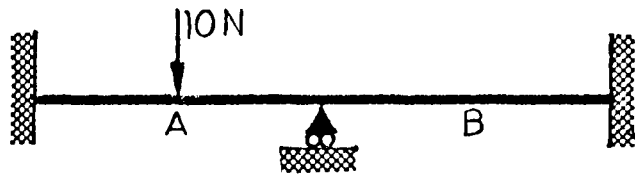
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.16	-0.13	23.1
B	-0.17	-0.13	30.8



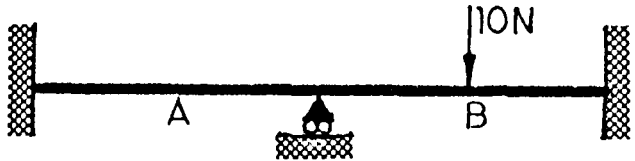
Location	Experimental (mm)	Theoretical (mm)	% Error *
A	-0.24	-0.19	26.3
B	0.08	0.06	<del>33.3</del>



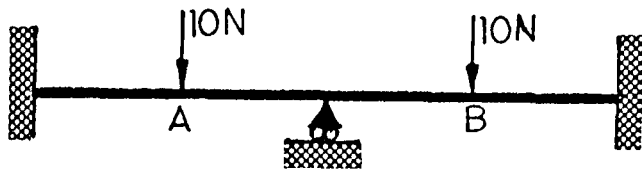
Location	Experimental (mm)	Theoretical (mm)	% Error *
A	0.08	0.06	<del>33.3</del>
B	-0.25	-0.19	31.6



Location	Experimental (mm)	Theoretical (mm)	% Error *
A	-0.10	-0.12	16.7
B	0.04	0.03	<del>33.3</del>

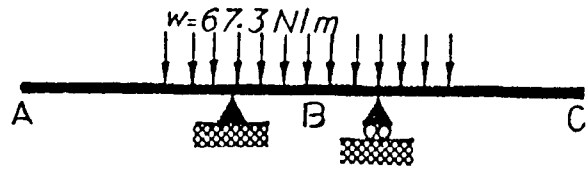


Location	Experimental (mm)	Theoretical (mm)	% Error *
A	0.04	0.03	<del>33.3</del>
B	-0.15	-0.12	25.0

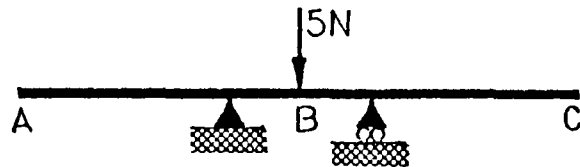


Location	Experimental (mm)	Theoretical (mm)	% Error *
A	-0.10	-0.08	<del>25</del>
B	-0.10	-0.08	<del>25</del>

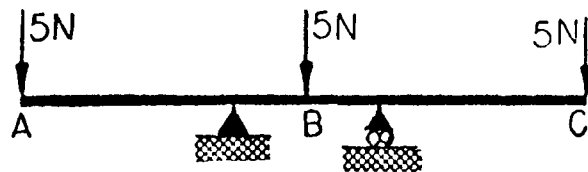




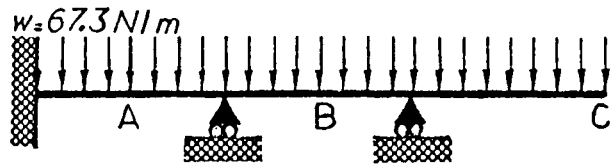
Location	Experimental (mm)	Theoretical (mm)	% Error *
A	-1.46	-1.34	8.9
B	0.08	0.09	<del>11.1</del>
C	-1.43	-1.34	6.7



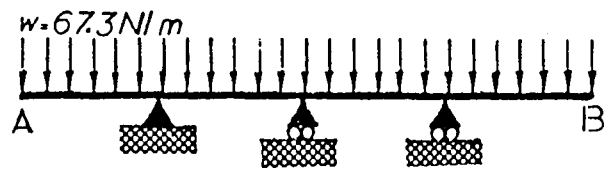
Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.55	0.53	3.8
B	-0.19	-0.18	5.5
C	0.50	0.53	5.7



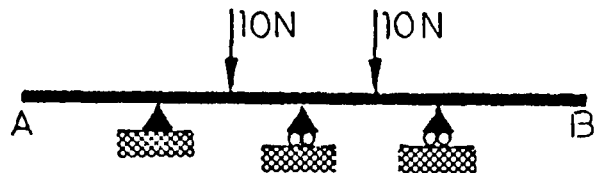
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-7.01	-6.57	6.7
B	-0.96	-0.89	7.9
C	-7.00	-6.57	6.5



Location	Experimental (mm)	Theoretical (mm)	% Error
A	-0.25	-0.22	13.6
B	0.60	0.55	9.1
C	-9.40	-8.4	11.9



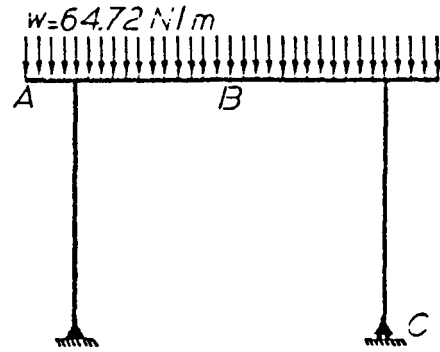
Location	Experimental (mm)	Theoretical (mm)	% Error
A	-2.93	-2.49	17.7
B	-2.78	-2.49	11.6



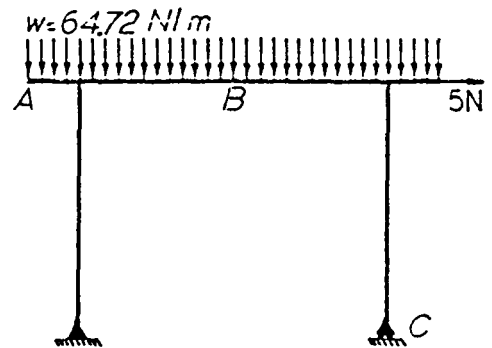
Location	Experimental (mm)	Theoretical (mm)	% Error
A	0.26	0.22	18.2
B	0.26	0.22	18.2

**APPENDIX B**  
**PORTAL FRAME TESTING RESULTS**

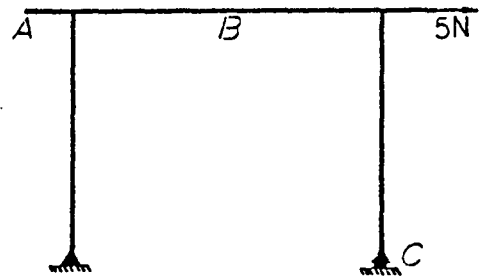
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	1.08	1.03	5
B <sub>V</sub>	-0.5	-0.44	14
C <sub>H</sub>	2.11	2.07	2



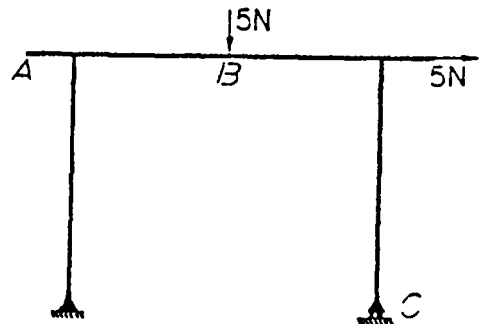
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	2.67	2.64	1
B <sub>V</sub>	-0.70	-0.68	3
C <sub>H</sub>	4.22	4.14	2



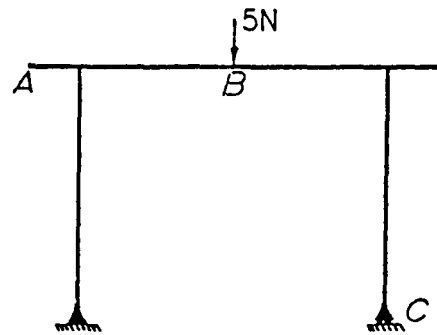
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	1.59	1.61	1.24
B <sub>V</sub>	-0.21	-0.23	8.7
C <sub>H</sub>	2.06	2.07	0.5



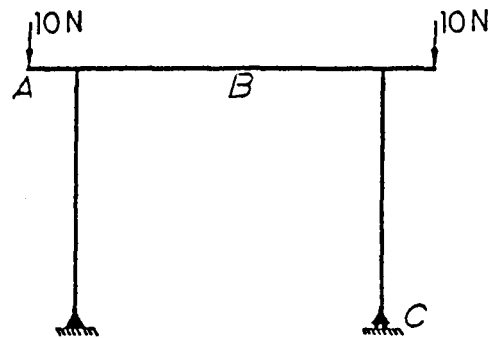
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	1.79	1.84	2.7
B <sub>V</sub>	-0.34	-0.33	3
C <sub>H</sub>	2.48	2.53	2



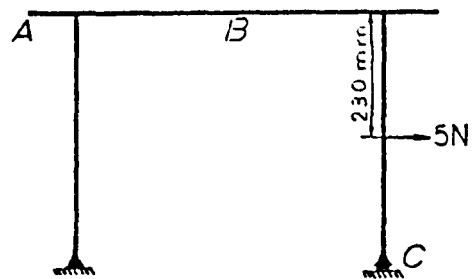
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.2	0.23	13
B <sub>V</sub>	-0.11	-0.1	10
C <sub>H</sub>	0.44	0.46	4



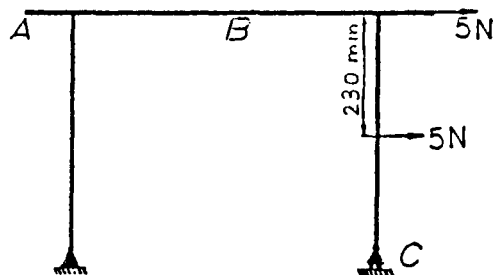
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.55	-0.55	0
B <sub>V</sub>	0.163	0.183	11
C <sub>H</sub>	-1.05	-1.1	4.5



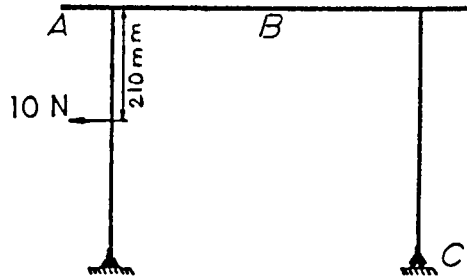
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	1.71	1.84	7
B <sub>V</sub>	-0.33	-0.35	6
C <sub>H</sub>	2.79	2.98	6



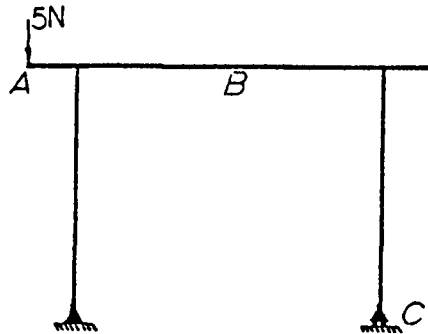
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	3.3	3.4	3
B <sub>V</sub>	-0.56	-0.58	3
C <sub>H</sub>	4.84	5.05	4



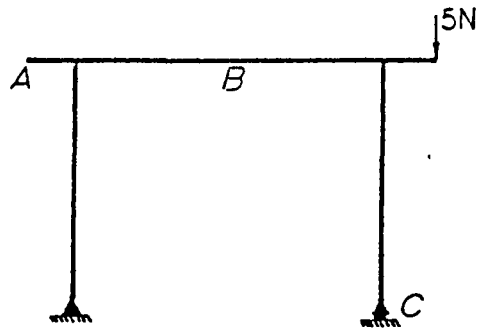
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-1.99	-2.01	0.1
B <sub>V</sub>	0.23	0.25	8
C <sub>H</sub>	-2.47	-2.51	1.6



Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.18	-0.19	5
B <sub>V</sub>	0.045	0.046	<del>1</del>
C <sub>H</sub>	-0.27	-0.28	3.6

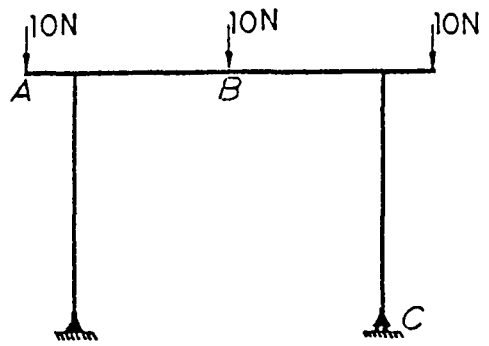


Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.08	-0.09	<del>11</del>
B <sub>V</sub>	0.04	0.04	0
C <sub>H</sub>	-0.25	-0.27	7

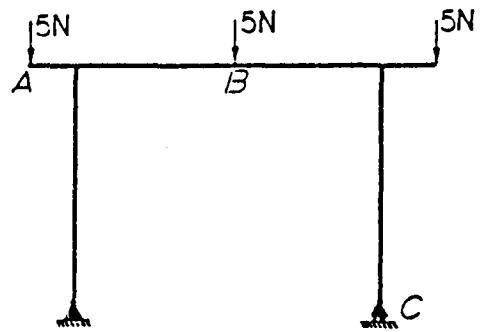


\* For measurements less than 0.09 mm, % error not given.

Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.09	-0.09	0
B <sub>V</sub>	-0.02	-0.02	0
C <sub>H</sub>	-0.17	-0.18	5.5

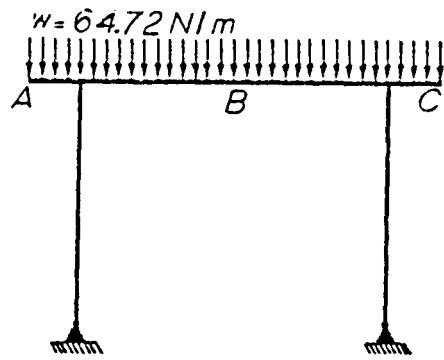


Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.045	-0.046	<del>1</del>
B <sub>V</sub>	-0.01	-0.01	0
C <sub>H</sub>	-0.08	-0.09	<del>11</del>

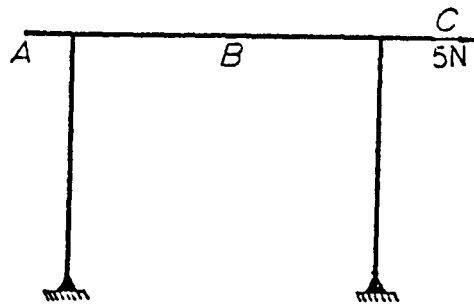




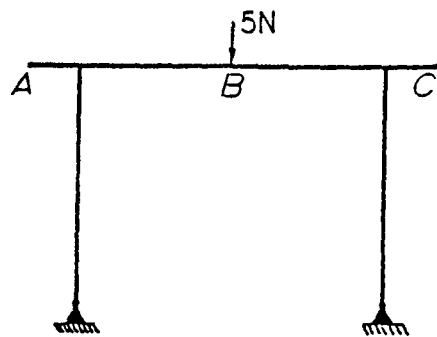
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0	0	0
B <sub>V</sub>	-0.22	-0.21	5
C <sub>V</sub>	0.05	0.06	<del>11</del>



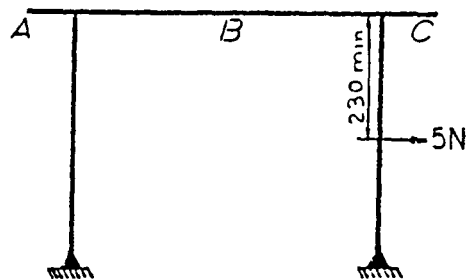
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.56	0.57	2
B <sub>V</sub>	0	0	0
C <sub>V</sub>	-0.04	-0.04	0



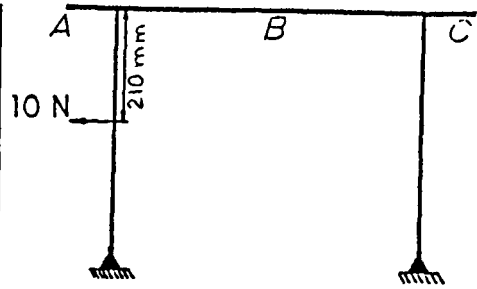
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0	0	0
B <sub>V</sub>	-0.05	-0.05	0
C <sub>V</sub>	0.01	0.01	0



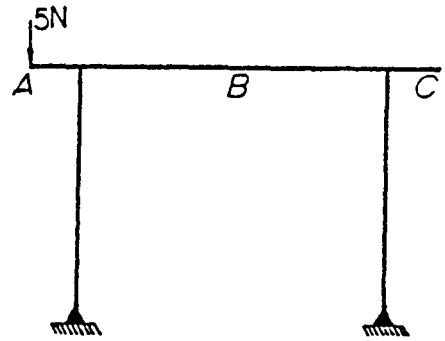
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.32	0.35	8.6
B <sub>V</sub>	-0.03	-0.014	<del>11</del>
C <sub>V</sub>	-0.01	-0.01	0



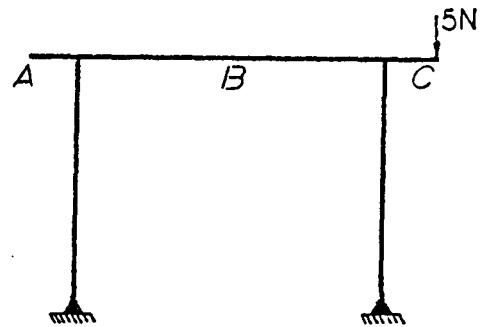
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.74	-0.75	1
B <sub>V</sub>	-0.09	-0.03	<del>200</del>
C <sub>V</sub>	0.02	0.07	<del>71</del>



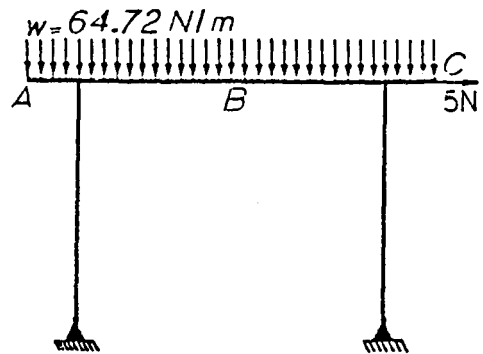
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.04	-0.05	<del>20</del>
B <sub>V</sub>	0.015	0.015	0
C <sub>V</sub>	0	0	0



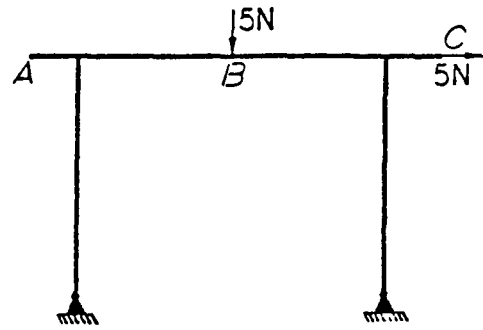
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.04	0.04	0
B <sub>V</sub>	0.01	0.01	0
C <sub>V</sub>	-0.03	-0.02	<del>50</del>



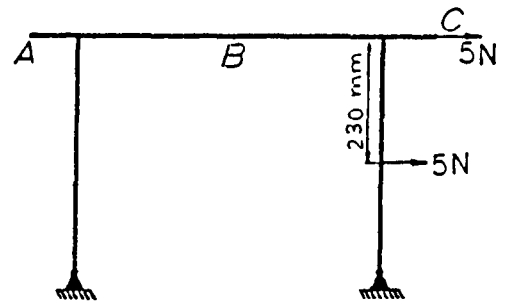
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.54	0.57	5
B <sub>V</sub>	-0.26	-0.21	24
C <sub>V</sub>	0.02	0.02	0



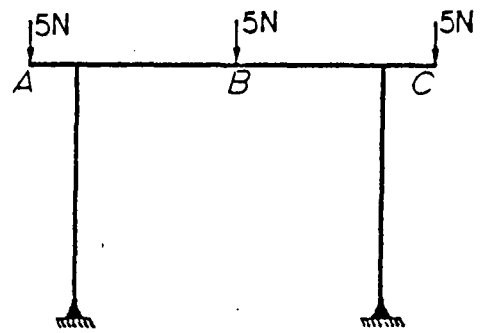
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.56	0.57	2
B <sub>V</sub>	-0.05	-0.05	0
C <sub>V</sub>	-0.02	-0.03	<del>33</del>



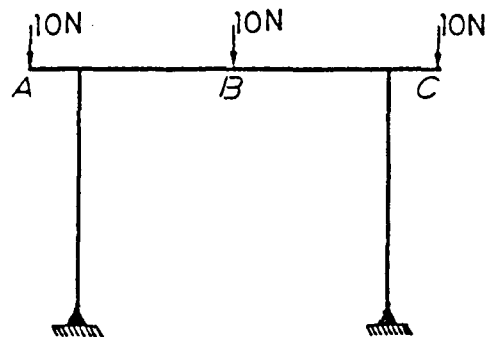
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.89	0.92	3
B <sub>V</sub>	-0.06	-0.01	<del>500</del>
C <sub>V</sub>	-0.12	-0.06	<del>100</del>



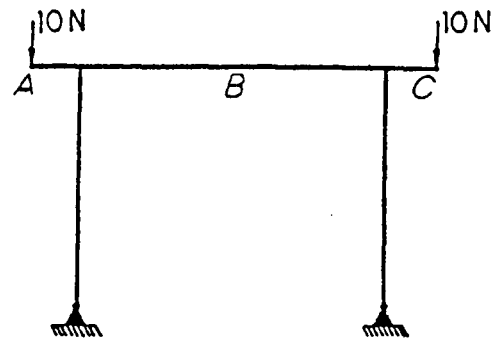
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0	0	0
B <sub>V</sub>	-0.02	-0.02	0
C <sub>V</sub>	0	0	0



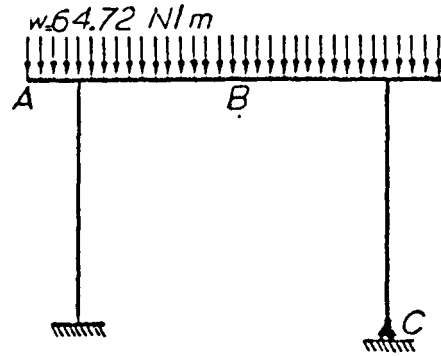
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0	0	0
B <sub>V</sub>	-0.04	-0.04	0
C <sub>V</sub>	-0.02	-0.01	<del>100</del>



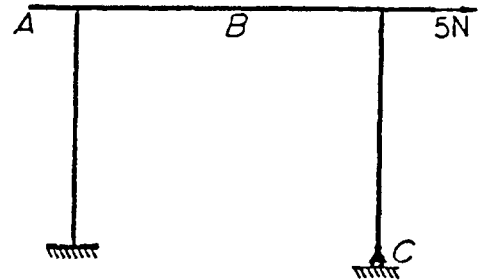
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0	0	0
B <sub>V</sub>	0.05	0.06	<del>17</del>
C <sub>V</sub>	-0.03	-0.04	<del>25</del>



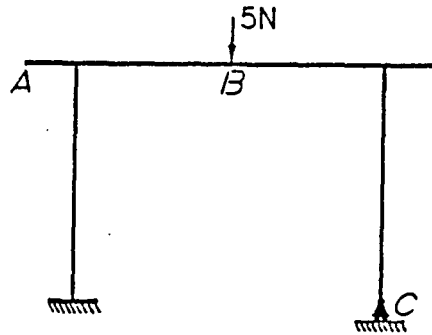
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.37	0.36	3
B <sub>V</sub>	-0.39	-0.37	5.4
C <sub>H</sub>	1.32	1.23	7



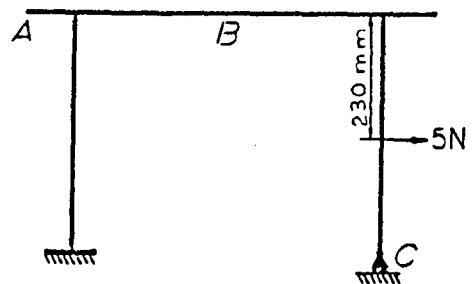
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.32	0.33	3
B <sub>V</sub>	-0.09	-0.08	<del>12.5</del>
C <sub>H</sub>	0.49	0.49	0



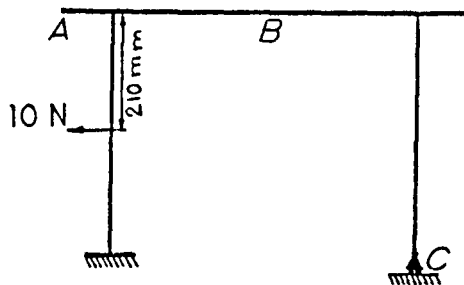
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.07	0.08	<del>12.5</del>
B <sub>V</sub>	-0.09	-0.08	<del>1</del>
C <sub>H</sub>	0.27	0.27	0



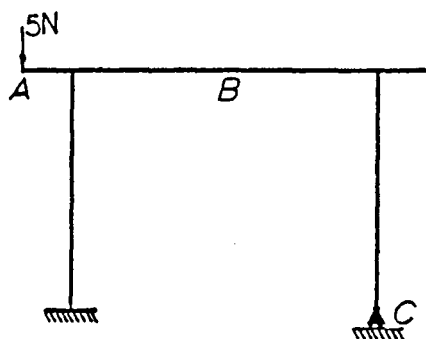
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.41	0.41	0
B <sub>V</sub>	-0.19	-0.18	5.5
C <sub>H</sub>	1.2	1.2	0



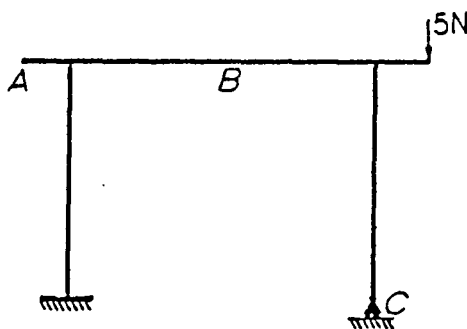
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.30	-0.29	3.4
B <sub>V</sub>	0.05	0.05	0
C <sub>H</sub>	-0.34	-0.38	10.5



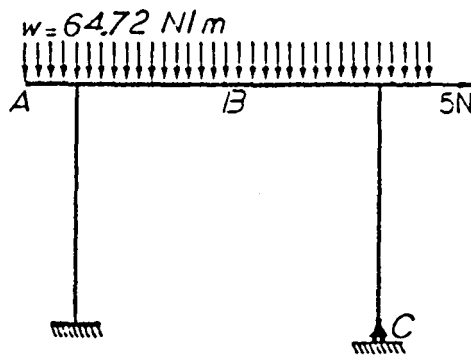
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.06	-0.06	0
B <sub>V</sub>	0.03	0.03	0
C <sub>H</sub>	-0.12	-0.13	8



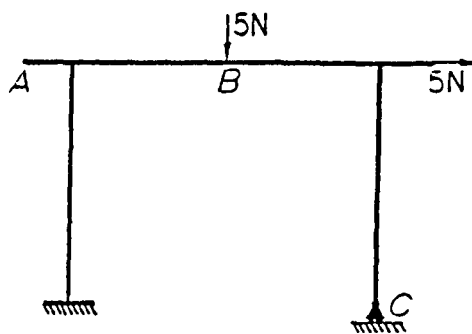
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.03	-0.03	0
B <sub>V</sub>	0.04	0.04	0
C <sub>H</sub>	-0.19	-0.20	5



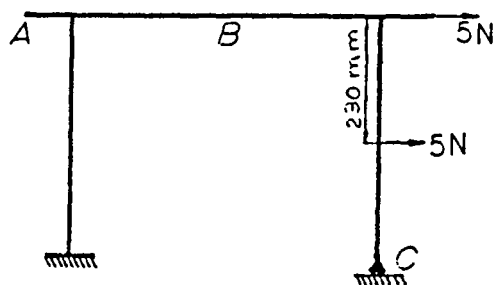
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.71	0.69	3
B <sub>V</sub>	-0.47	-0.44	7
C <sub>H</sub>	1.80	1.73	4



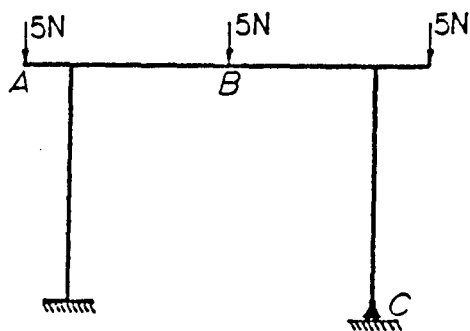
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.40	0.41	2.4
B <sub>V</sub>	-0.20	-0.16	25
C <sub>H</sub>	0.76	0.76	0



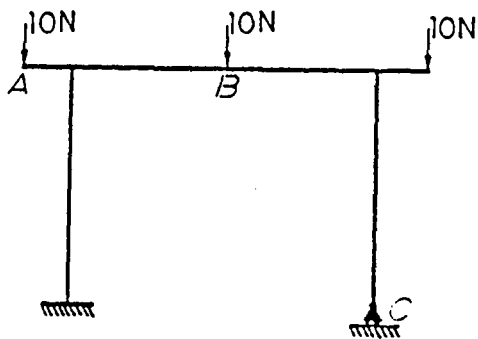
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.74	0.74	0
B <sub>V</sub>	-0.32	-0.26	23
C <sub>H</sub>	1.67	1.70	2



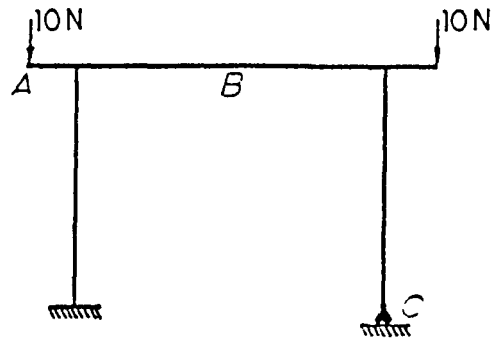
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.01	-0.01	0
B <sub>V</sub>	-0.03	-0.01	<del>200</del>
C <sub>H</sub>	-0.05	-0.05	0



Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.03	-0.03	0
B <sub>V</sub>	-0.04	-0.03	<del>100</del>
C <sub>H</sub>	-0.10	-0.10	0

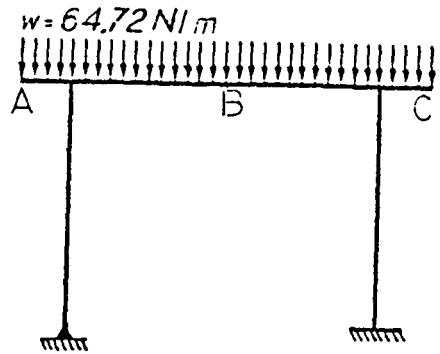


Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.19	-0.19	0
B <sub>V</sub>	0.16	0.14	14
C <sub>H</sub>	-0.65	-0.65	0

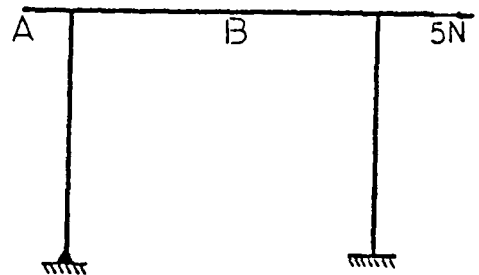




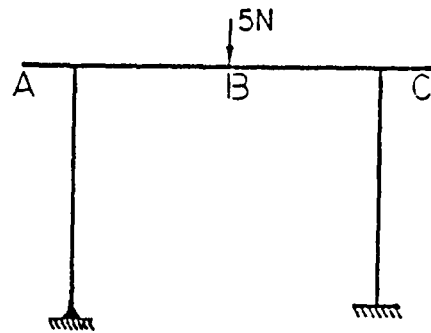
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.09	-0.08	<del>12.5</del>
B <sub>V</sub>	-0.2	-0.2	0
C <sub>V</sub>	0.06	0.07	<del>14</del>



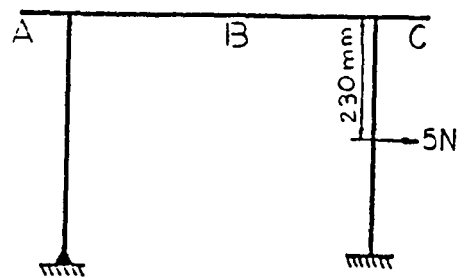
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.22	0.22	0
B <sub>V</sub>	0.02	0.02	0
C <sub>V</sub>	-0.04	-0.03	<del>33</del>



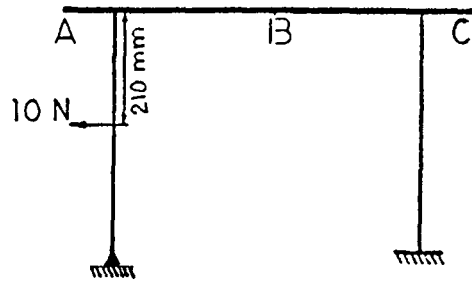
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.01	-0.02	<del>50</del>
B <sub>V</sub>	-0.05	-0.05	0
C <sub>V</sub>	0.01	0.01	0



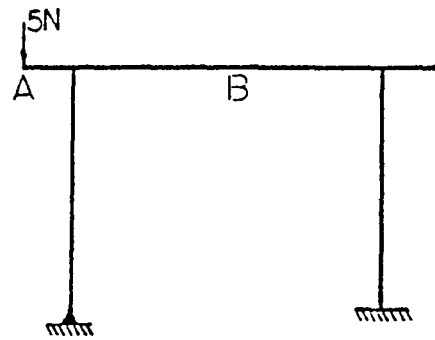
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.08	0.09	<del>11</del>
B <sub>V</sub>	0	0	0
C <sub>V</sub>	0	0.01	<del>100</del>



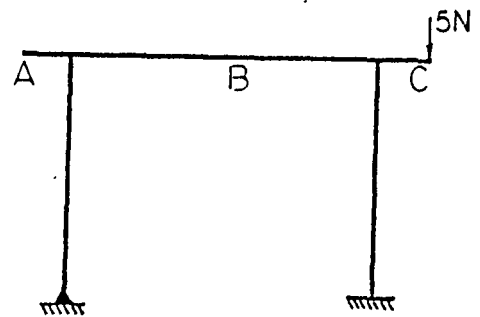
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.29	-0.30	3
B <sub>V</sub>	-0.04	-0.05	<del>20</del>
C <sub>V</sub>	0.19	0.05	<del>236</del>



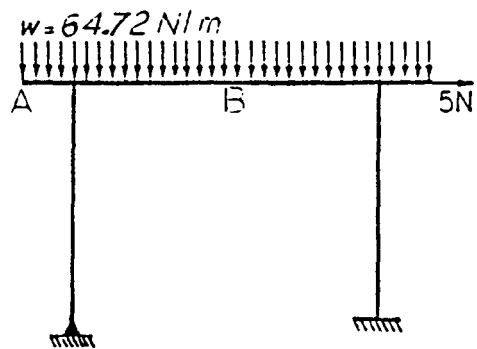
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.01	-0.01	0
A <sub>V</sub>	-0.02	-0.02	0
B <sub>V</sub>	0.01	0.01	0



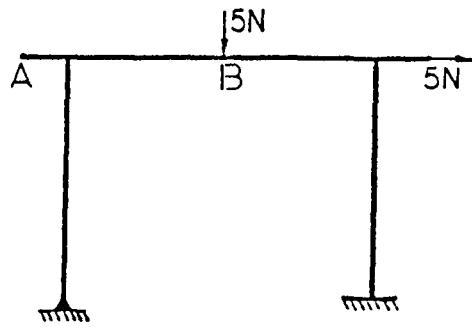
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.03	0.03	0
B <sub>V</sub>	0.01	0.01	0
C <sub>V</sub>	-0.02	-0.02	0



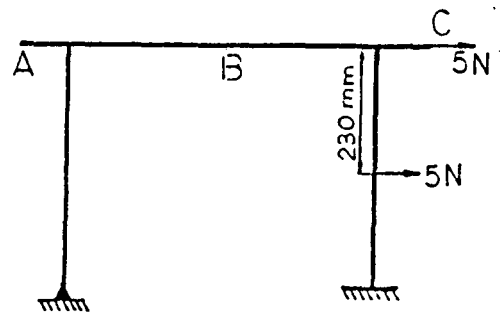
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.12	0.14	14
A <sub>V</sub>	0.04	0.07	<del>43</del>
B <sub>V</sub>	-0.2	-0.2	0



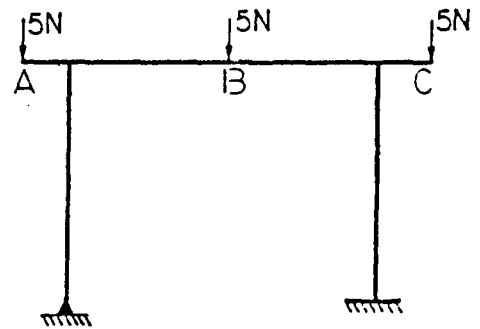
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.2	0.2	0
A <sub>V</sub>	0.02	0.03	<del>10</del>
B <sub>V</sub>	-0.03	-0.03	0



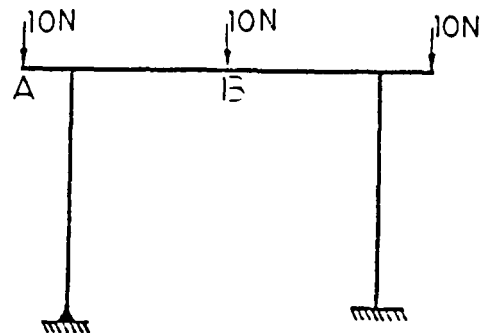
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.31	0.31	0
B <sub>V</sub>	0.02	0.02	0
C <sub>V</sub>	-0.04	-0.04	0



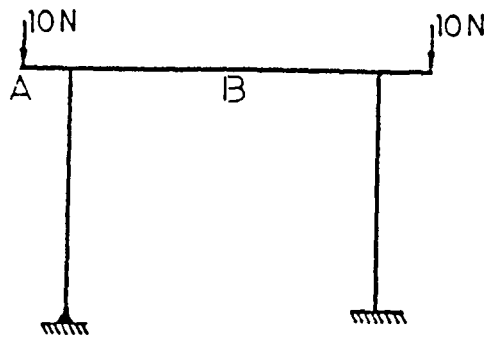
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0	0	0
B <sub>V</sub>	-0.02	-0.02	0
C <sub>V</sub>	-0.01	-0.01	0



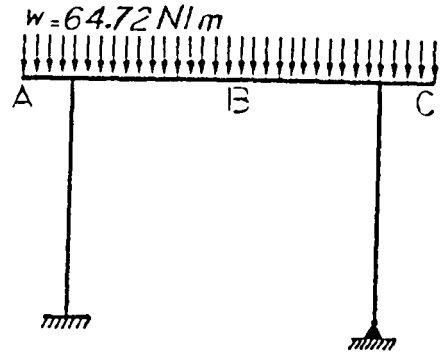
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.01	0.005	<del>100</del>
A <sub>V</sub>	-0.02	-0.02	0
B <sub>V</sub>	-0.05	-0.04	<del>25</del>



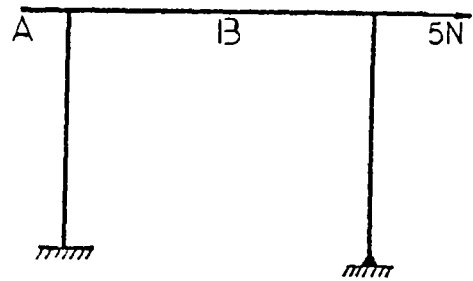
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.04	0.04	0
A <sub>V</sub>	-0.05	-0.04	<del>25</del>
B <sub>V</sub>	0.05	0.06	<del>17</del>



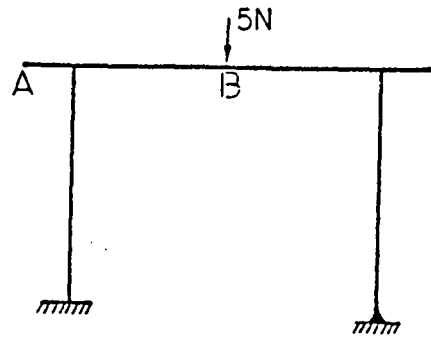
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.08	0.08	0
B <sub>V</sub>	-0.2	-0.2	0
C <sub>V</sub>	0.04	0.06	<del>33</del>



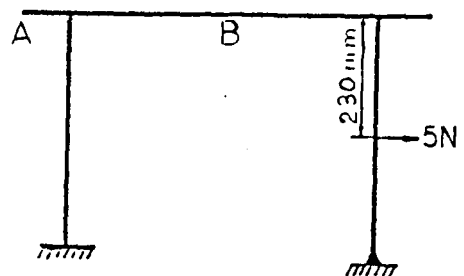
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.22	0.22	0
A <sub>V</sub>	0.02	0.03	<del>33</del>
B <sub>V</sub>	-0.03	-0.02	<del>50</del>



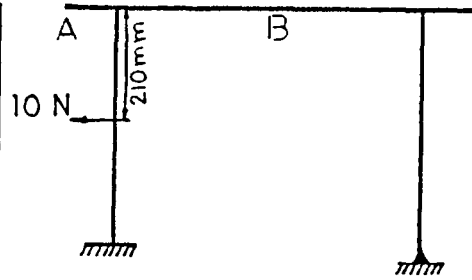
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.02	0.02	0
A <sub>V</sub>	0.01	0.02	<del>50</del>
B <sub>V</sub>	-0.05	-0.05	0



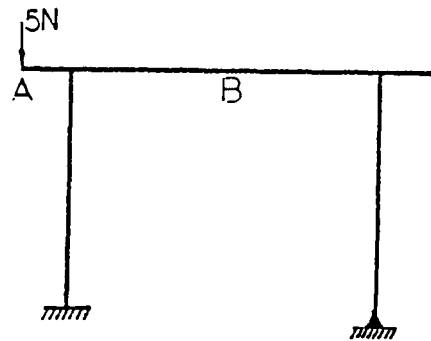
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.14	0.14	0
A <sub>V</sub>	0.02	0.02	0
B <sub>V</sub>	-0.025	-0.025	0



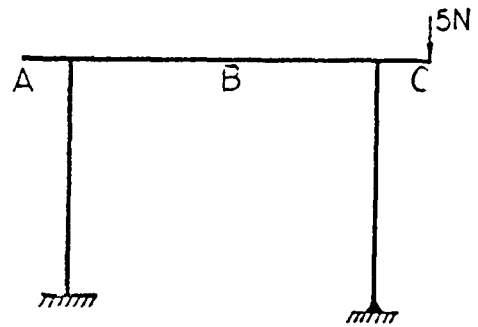
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.2	-0.2	0
A <sub>V</sub>	-0.01	-0.01	0
B <sub>V</sub>	0	0	0



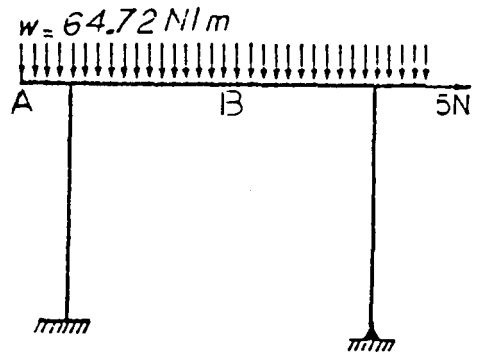
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.03	-0.03	0
A <sub>V</sub>	-0.02	-0.02	0
B <sub>V</sub>	0.01	0.02	<del>50</del>



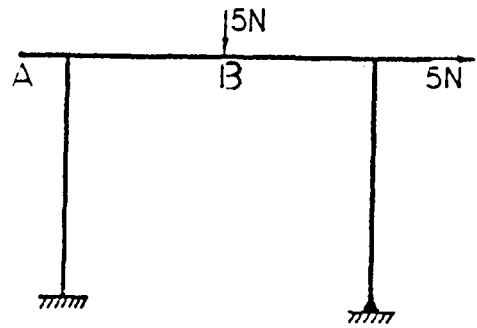
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>V</sub>	0.01	0.01	0
B <sub>V</sub>	0.01	0.01	0
C <sub>H</sub>	-0.02	-0.02	0



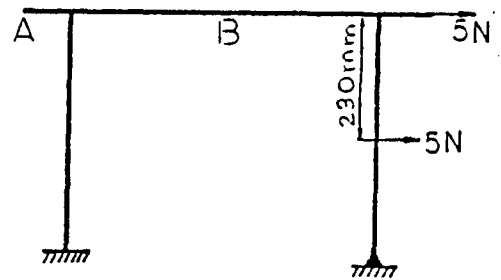
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.29	0.30	3
A <sub>V</sub>	0.08	0.1	<del>20</del>
B <sub>V</sub>	-0.24	-0.23	4



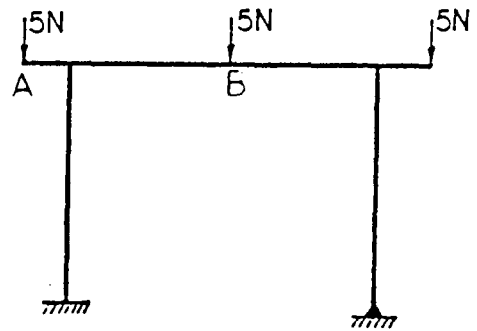
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.23	0.24	4
A <sub>V</sub>	0.03	0.05	<del>36</del>
B <sub>V</sub>	-0.09	-0.07	<del>28</del>



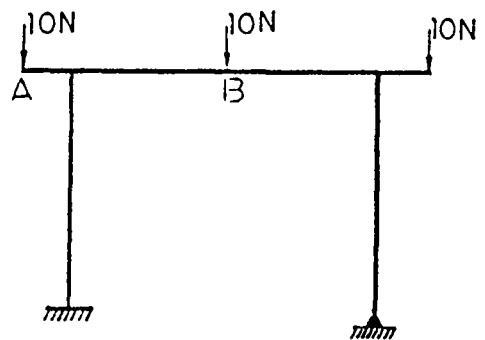
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.35	0.36	3
A <sub>V</sub>	0.03	0.06	<del>36</del>
B <sub>V</sub>	-0.07	-0.04	<del>75</del>



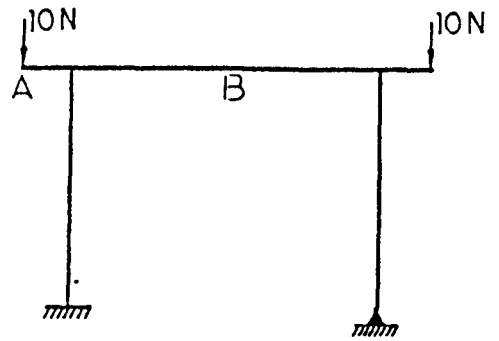
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0	0	0
A <sub>V</sub>	-0.01	-0.01	0
B <sub>V</sub>	-0.02	-0.02	0



Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0	0	0
A <sub>V</sub>	-0.02	-0.02	0
B <sub>V</sub>	-0.05	-0.04	<del>25</del>

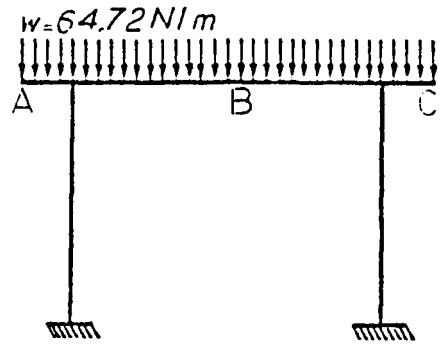


Location	Experimental (mm)	Theoretical (mm)	% Error *
$A_H$	-0.04	-0.04	0
$A_V$	-0.05	-0.05	0
$B_V$	0.05	0.06	<del>17</del>

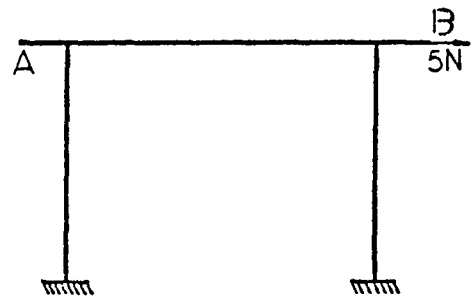




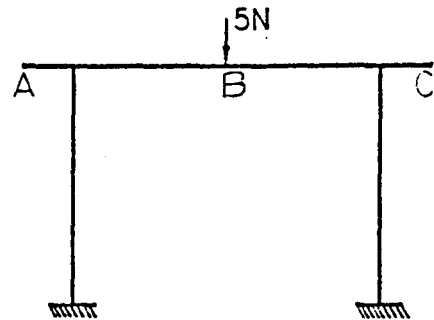
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0	0	0
B <sub>V</sub>	-0.2	-0.2	0
C <sub>V</sub>	0.05	0.05	0



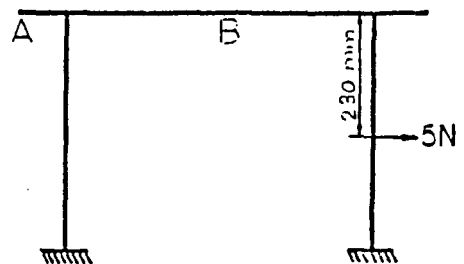
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.13	0.13	0
A <sub>V</sub>	0.02	0.02	0
B <sub>V</sub>	0	0	0



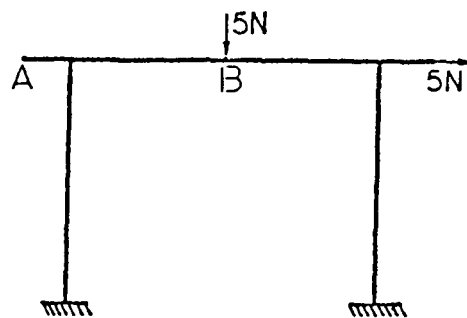
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>V</sub>	0.01	0.01	0
B <sub>V</sub>	-0.05	-0.05	0
C <sub>V</sub>	0.01	0.01	0



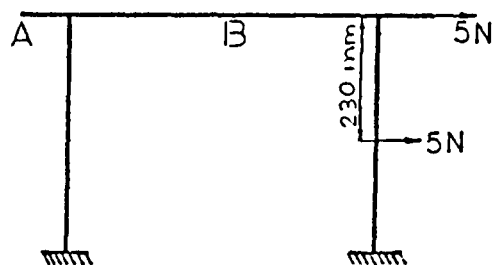
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.05	0.05	0
A <sub>V</sub>	0	0.01	100
B <sub>V</sub>	0	-0.01	100



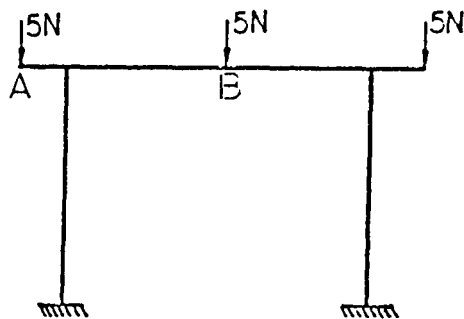
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.13	0.13	0
A <sub>V</sub>	0.03	0.03	0
B <sub>V</sub>	-0.05	-0.05	0



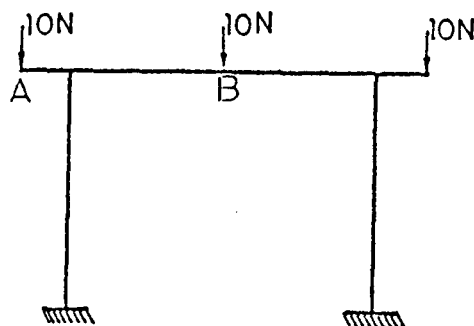
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0.18	0.19	5
A <sub>V</sub>	0.02	0.03	<del>10</del>
B <sub>V</sub>	0	-0.01	<del>100</del>



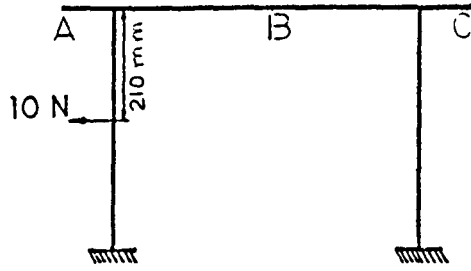
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0	0	0
A <sub>V</sub>	-0.01	-0.01	0
B <sub>V</sub>	-0.02	-0.02	0



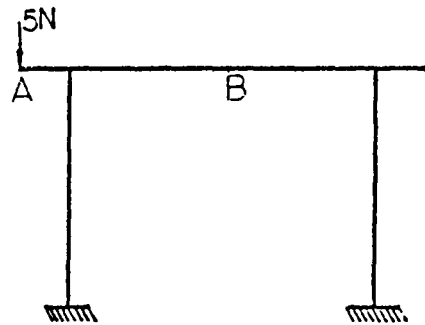
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0	0	0
A <sub>V</sub>	-0.02	-0.02	0
B <sub>V</sub>	-0.04	-0.04	0



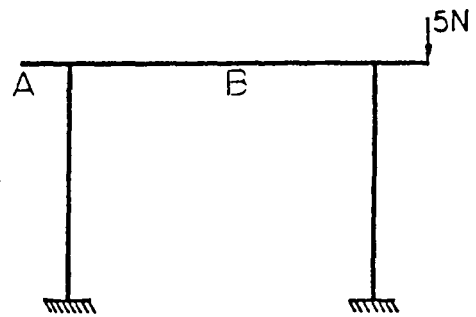
Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	-0.12	-0.12	0
B <sub>V</sub>	-0.02	-0.02	0
C <sub>V</sub>	0.01	0.02	<del>50</del>



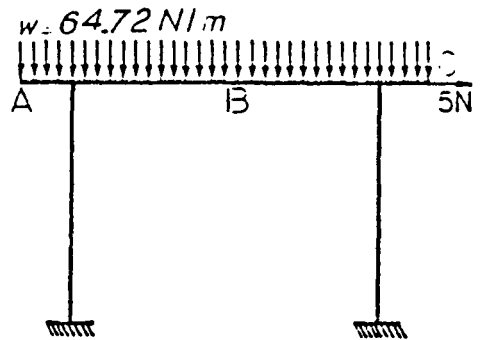
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	-0.02	-0.02	0
A <sub>V</sub>	-0.02	-0.02	0
B <sub>V</sub>	0.01	0.01	0



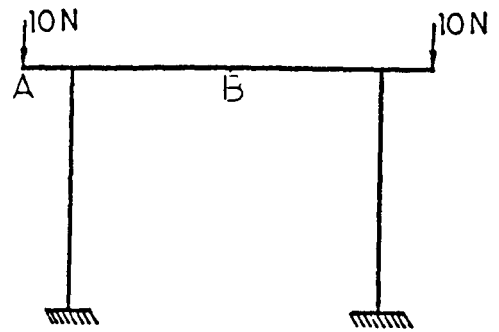
Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.02	0.02	0
A <sub>V</sub>	0	0	0
B <sub>V</sub>	0.01	0.01	0



Location	Experimental (mm)	Theoretical (mm)	% Error
A <sub>H</sub>	0.13	0.13	0
B <sub>V</sub>	-0.2	-0.2	0
C <sub>V</sub>	0.03	0.03	0



Location	Experimental (mm)	Theoretical (mm)	% Error *
A <sub>H</sub>	0	0	0
A <sub>V</sub>	-0.04	-0.04	0
B <sub>V</sub>	0.04	0.05	<del>20</del>



APPENDIX C  
BEAM ANALYSIS PROGRAM

```

$JOB
C *****
C *                               MAIN PROGRAM                               *
C *****
  DOUBLE PRECISION AU(25,25),BU(25),WKAREA(25)
  DIMENSION P(30),A(30),W(30),BG(30),E(30),R(26)
  DATA M/1/,IA/25/,IDGT/3/
  REAL L(30)
  WRITE(9,10)
10  FORMAT('THE DATA REQUIRED HERE IS TO DESIGNATE THE BOUNDARY' /,
$ 'CONDITION OF THE BEAM AT THE FIRST AND LAST SUPPORT.THERE ARE' /,
$ 'SIX CONDITIONS THAT THIS PROGRAM IS DESIGNED TO ANALYZE.' /,
$ '1-(FIX-FIX)',9X,'2-(FIX-PIN)', /, '3-(FIX-FREE)',8X,'4-(PIN-PIN)'
$ /, '5-(PINNED-FREE)',5X,'6-(FREE-FREE)', /,
$ 'THE CONDITIONS MENTIONED IN THE PARENTHESIS ARE REFERED TO THE' /,
$ 'FIRST AND LAST SUPPORT IN THE BEAM.ENTER THE NUMBER TO THE' /,
$ 'LEFT OF PARENTHESIS FOR THE CONDITION APPLICABLE TO THE BEAM' /,
$ 'YOU ARE ANALYZING....???' )
  READ(8,*)ICONT
  WRITE(9,15)
15  FORMAT('ENTER THE NUMBER OF SPANS IN THE BEAM....???' )
  READ(8,*)N
  WRITE(9,20)
20  FORMAT('ENTER LENGTH OF EACH SPAN ONE PER LINE...???' )
  TSP=0
  DO 22 I=1,N
  READ(8,*)L(I)
  WRITE(6,23)I,L(I)
23  FORMAT(/,5X,'L(',I2,') =' ,F6.2)
  TSP=L(I)+TSP
22  CONTINUE
  WRITE(9,25)
25  FORMAT('ENTER THE TOTAL NUMBER OF CONCENTRATED VERTICAL FORCE' /,
$ 'TO BE APPLIED ON THE BEAM....???' /)
  READ(8,*)NP
  IF(NP.EQ.0)GO TO 42
  WRITE(9,30)
30  FORMAT('ENTER THE VALUE OF EACH CONCENTRATED FORCE AND ITS' /,
$ 'DISTANCE FROM FIRST SUPPORT...???' )
  DO 35 I=1,NP
  READ(8,*)P(I),A(I)
  IF(A(I).GT.TSP)GO TO 37
  WRITE(6,34)I,P(I),A(I)
34  FORMAT(/,10X,'P(',I2,')=' ,F6.2,'(N)',5X,'AT X =' ,
$ F6.2,'(MM)' ,//)
  GO TO 35
37  WRITE(9,40)I
40  FORMAT(/,5X,'P(',I2,')ACTING BEYOND RANGE OF THE BEAM' )
  STOP
35  CONTINUE
  GO TO 42
42  WRITE(9,45)
45  FORMAT(,5X,'ENTER TOTAL NUMBER OF VERTICAL UNIFORM LOAD TO BE'
$ /, 'APPLIED ON THE BEAM...???' )
  READ(8,*)NW
  IF(NW.EQ.0)GO TO 46
  WRITE(9,50)
50  FORMAT('ENTER THE DISTANCE OF STARTING POINT,VALUE AND DISTANCE
$ OF END POINT OF THE UNIFORM LOAD.ONE LINE PER EACH SET OF LOAD.
$ /, ' NOTE ALL THE DISTANCES MEASURED FROM FIRST SUPPORT...???' )
  DO 57 I=1,NW
  READ(8,*)BG(I),W(I),E(I)
  IF(BG(I).GT.TSP.OR.E(I).GT.TSP)GO TO 59
  WRITE(6,58)I,W(I),BG(I),E(I)
58  FORMAT(/,10X,'W(',I2,')=' ,F10.8,5X,'(N/MM)',5X,'STARTS AT
$ X=' ,F6.2,/,42X,'AND ENDS AT' ,7X,'X=' ,F6.2,'(MM)' )
  GO TO 57

```

```

59 WRITE(9,60)I
60 FORMAT('W(' ,I2,')ACTING BEYOND RANGE OF THE BEAM')
STOP
57 CONTINUE
46 GO TO(51,52,53,54,55,56),ICONT
51 CALL FF(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
GO TO 65
52 CALL FP(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
GO TO 65
53 CALL FFR(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
GO TO 65
54 CALL PP(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
GO TO 65
55 CALL PFR(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
GO TO 65
56 CALL FRFR(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
65 STOP
END

```

```

C *****
C *                               FIXED-FIXED                               *
C *****

```

```

SUBROUTINE FF(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
DOUBLE PRECISION AU(25,25),BU(25),WKAREA(25)
DIMENSION P(30),A(30),W(30),BG(30),E(30),R(26)
REAL L(30)
WRITE(6,10)N
10 FORMAT(/,35X,15X,'BEAM FIXED AT BOTH ENDS (' ,I2,' SPANS)',/)
K=N+2
NS=N
CALL DEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT)
S=L(1)
DO 20 I=1,N
MD=K-1
AU(I,MD)=S**2/(-2)
J=I+1
IF(J.GT.N)GO TO 25
S=L(J)+S
20 CONTINUE
25 DO 30 I=1,N
AU(I,K)=0
30 CONTINUE
CALL SLOPE(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
AU(MD,MD)=-TSP
AU(MD,K)=0
CALL MOM(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
AU(K,MD)=-1
AU(K,K)=1
CALL LEQT1F(AU,M,K,IA,BU,IDGT,WKAREA,IER)
AM1=BU(MD)
AM2=BU(K)
CALL EQU(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,RL)
R(MD)=RL
Z1=0
Z2=0
CALL CALDEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,TSP,
$AM1,AM2,Z1,Z2)
RETURN
END

```

```

C *****
C *                               FIXED-PINNED                               *
C *****

```

```

SUBROUTINE FP(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
DOUBLE PRECISION AU(25,25),BU(25),WKAREA(25)
DIMENSION P(30),A(30),W(30),BG(30),E(30),R(26)
REAL L(30)
WRITE(6,10)N
10 FORMAT(/,30X,'BEAM FIXED AT THE FIRST AND PINNED AT THE LAST

```

```

$SUPPORT (' ,I2,' SPANS)' ,//)
  K=N+1
  NS=N
  CALL DEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT)
  S=L(1)
  DO 100 I=1,N
    AU(I,K)=S**2/(-2)
    IC=I+1
    IF(IC.GT.N)GO TO 20
    S=L(IC)+S
100 CONTINUE
20 CALL MOM(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
  AU(K,K)=-1
  CALL LEQT1F(AU,M,K,IA,BU,IDGT,WKAREA,IER)
  CALL EQU(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,RL)
  AM1=BU(K)
  R(K)=RL
  Z1=0
  Z2=0
  CALL CALDEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,TSP,
$AM1,AM2,Z1,Z2)
  RETURN
  END
C *****
C *                               FIXED-FREE                               *
C *****
  SUBROUTINE FFR(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
  DOUBLE PRECISION AU(25,25),BU(25),WKAREA(25)
  DIMENSION P(30),A(30),W(30),BG(30),E(30),R(26)
  REAL L(30)
  WRITE(6,10)N
10 FORMAT(//,30X,' BEAM FIXED AT THE FIRST AND FREE AT THE LAST SUPPOR
$T (' ,I2,' SPANS)' ,//)
  K=N+1
  NS=N-1
  IF(N.EQ.1)GO TO 15
  CALL DEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT)
  SP=L(1)
  DO 12 I=1,NS
    AU(I,K)=-SP**2/2
    J=I+1
    SP=SP+L(J)
12 CONTINUE
15 CALL SHEAR(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
  AU(N,K)=0.
  CALL MOM(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
  AU(K,K)=-1
  CALL LEQT1F(AU,M,K,IA,BU,IDGT,WKAREA,IER)
  NS=N
  CALL EQU(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,RL)
  AM1=BU(K)
  Z1=0
  Z2=0
  CALL CALDEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,TSP,
$AM1,AM2,Z1,Z2)
  RETURN
  END
C *****
C *                               PINNED-PINNED                               *
C *****
  SUBROUTINE PP(N,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP,IA,M,IDGT)
  DOUBLE PRECISION AU(25,25),BU(25),WKAREA(25)
  DIMENSION P(30),A(30),W(30),BG(30),E(30),R(26)
  REAL L(30)
  WRITE(6,10)N
10 FORMAT(//,35X,' BEAM PINNED AT BOTH ENDS (' ,I2,' SPANS)' ,//)
  K=N+1

```



```

NS=N
CALL DEF(N,K,NS, NP, NW, P, A, W, BG, E, L, AU, BU, ICONT)
S=L(1)
  DO 12 I=1, N
    AU(I, K)=S
    IO=I+1
    IF( IO. GT. N)GO TO 20
    S=S+L( IO)
12  CONTINUE
20  CALL MOM(N, K, NS, NP, NW, P, A, W, BG, E, L, AU, BU, ICONT, TSP)
    AU(K, K)=0
    CALL LEQT1F(AU, M, K, IA, BU, IDGT, WKAREA, IER)
    CALL EQU(N, K, NS, NP, NW, P, A, W, BG, E, L, AU, BU, R, ICONT, RL)
    IN=N+1
    R( IN)=RL
    Z1=BU(K)
    Z2=0
    AM1=0
    CALL CALDEF(N, K, NS, NP, NW, P, A, W, BG, E, L, AU, BU, R, ICONT, TSP,
$AM1, AM2, Z1, Z2)
    RETURN
    END
C *****
C * PINNED-FREE *
C *****
SUBROUTINE PFR(N, NP, NW, P, A, W, BG, E, L, AU, BU, ICONT, TSP, IA, M, IDGT)
DIMENSION P(30), A(30), W(30), BG(30), E(30), R(26)
DOUBLE PRECISION AU(25, 25), BU(25), WKAREA(25)
REAL L(30)
WRITE(6, 10)N
10  FORMAT(//, 30X, 'BEAM PINNED AT THE FIRST AND FREE AT THE LAST SUPPO
$S (', 12, ' SPANS)', //)
    IF(N. GT. 1)THEN DO
      K=N+1
      NS=N-1
      CALL DEF(N, K, NS, NP, NW, P, A, W, BG, E, L, AU, BU, ICONT)
      S=L(1)
      DO 12 I=1, NS
        AU(I, K)=S
        IC=I+1
        S=S+L( IC)
12  CONTINUE
      CALL SHEAR(N, K, NS, NP, NW, P, A, W, BG, E, L, AU, BU, ICONT, TSP)
      AU(N, K)=0
      CALL MOM(N, K, NS, NP, NW, P, A, W, BG, E, L, AU, BU, ICONT, TSP)
      AU(K, K)=0
      CALL LEQT1F(AU, M, K, IA, BU, IDGT, WKAREA, IER)
      NS=N
      CALL EQU(N, K, NS, NP, NW, P, A, W, BG, E, L, AU, BU, R, ICONT, RL)
      Z1=BU(K)
      AM1=0
      Z2=0
      CALL CALDEF(N, K, NS, NP, NW, P, A, W, BG, E, L, AU, BU, R, ICONT, TSP,
$AM1, AM2, Z1, Z2)
    ELSE DO
25  WRITE(9, 25)
      FORMAT(//, 25X, 'SYSTEM IS UNSTABLE', //)
      STOP
    END IF
    RETURN
    END
C *****
C * FREE-FREE *
C *****
SUBROUTINE FRFR(N, NP, NW, P, A, W, BG, E, L, AU, BU, ICONT, TSP, IA, M, IDGT)
DOUBLE PRECISION AU(25, 25), BU(25), WKAREA(25)
DIMENSION P(30), A(30), W(30), BG(30), E(30), R(26)

```

```

REAL L(30)
WRITE(6,10)N
10 FORMAT(//,35X,'BEAM IS FREE AT BOTH ENDS (' ,I2,' SPANS)' ,//)
K=N+1
IF(N.GT.2)THEN DO
NS=N-1
CALL DEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT)
S=L(1)
DO 12 I=1,NS
AU(I,N)=S
MD=I+1
S=S+L(MD)
12 CONTINUE
DO 20 I=1,NS
AU(I,K)=1
20 CONTINUE
CALL SHEAR(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
DO 30 I=N,K
AU(N,I)=0
30 CONTINUE
CALL MOM(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
DO 40 I=N,K
AU(K,I)=0.
40 CONTINUE
CALL LEQT1F(AU,M,K,IA,BU,IDGT,WKAREA,IER)
CALL EQU(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,RL)
Z1=BU(N)
Z2=BU(K)
AM1=0
CALL CALDEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,TSP,
$ AM1,AM2,Z1,Z2)
ELSE DO
35 WRITE(9,35)
FORMAT(//,'SYSTEM IS UNSTABLE' ,//)
STOP
END IF
RETURN
END
C *****
C * DEFLECTION BOUNDARY CONDITION *
C *****
SUBROUTINE DEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT)
DOUBLE PRECISION AU(25,25),BU(25)
DIMENSION P(30),A(30),W(30),BG(30),E(30)
REAL L(30)
X=L(1)
DO 10 I=1,NS
IF(ICONT.EQ.6)GO TO 15
SP=0
DO 20 J=1,N
AU(I,J)=(X-SP)**3/6
IF((X-SP).LT.0.)AU(I,J)=0.
SP=SP+L(J)
20 CONTINUE
GO TO 17
15 SP=L(1)
DO 16 J=1,NS
AU(I,J)=(X-SP)**3/6
JJ=J+1
IF(JJ.GT.N)GO TO 17
SP=SP+L(J+1)
IF((X-SP).LT.0)AU(I,J)=0
16 CONTINUE
17 CA1=0
IF(NP.EQ.0)GO TO 26
DO 25 NL=1,NP
CA=(X-A(NL))**3/6*P(NL)

```

```

                IF( (X-A(NL)).LT.0)CA=0
                CA1=CA+CA1
25             CONTINUE
26             US1=0
                IF(NW.EQ.0)GO TO 29
                DO 30 I=1,NW
                    UB=(X-BG(LL))**4/24*W(LL)
                    UE=-(X-E(LL))**4/24*W(LL)
                    IF((X-BG(LL)).LT.0)UB=0
                    IF((X-E(LL)).LT.0)UE=0
                    US1=UB+UE+US1
30             CONTINUE
29             BU(I)=CA1+US1
                IC=I+1
                IF(IC.GT.NS)GO TO 35
                X=X+L(IC)
10             CONTINUE
35             RETURN
            END
C *****
C *                               SLOPE BOUNDARY CONDITION                               *
C *****
            SUBROUTINE SLOPE(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
            DOUBLE PRECISION AU(25,25),BU(25)
            DIMENSION P(30),A(30),W(30),BG(30),E(30)
            REAL L(30)
            SPL=0
                DO 10 I=1,NS
                    J=K-1
                    AU(J,I)=(TSP-SPL)**2/2
                    SPL=SPL+L(I)
10             CONTINUE
            CB1=0
            IF(NP.EQ.0)GO TO 21
                DO 20 I=1,NP
                    CB=(TSP-A(I))**2/2*P(I)
                    CB1=CB1+CB
20             CONTINUE
21             UB1=0
                UE1=0
                IF(NW.EQ.0)GO TO 31
                DO 30 I=1,NW
                    UB=(TSP-BG(I))**3/6*W(I)
                    UE=-(TSP-E(I))**3/6*W(I)
                    UB1=UB+UB1
                    UE1=UE+UE1
30             CONTINUE
31             BU(K-1)=CB1+UB1+UE1
                RETURN
            END
C *****
C *                               MOMENT BOUNDARY CONDITION                               *
C *****
            SUBROUTINE MOM(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
            DOUBLE PRECISION AU(25,25),BU(25)
            DIMENSION P(30),A(30),W(30),BG(30),E(30)
            REAL L(30)
            IF(ICONT.EQ.6)GO TO 15
            SPL=0
                DO 10 I=1,N
                    AU(K,I)=(TSP-SPL)
                    SPL=SPL+L(I)
10             CONTINUE
            GO TO 16
15             SPL=L(1)
                DO 12 I=1,NS
                    AU(K,I)=TSP-SPL

```

```

      J=I+1
      SPL=SPL+L(J)
12     CONTINUE
16     CB1=0
      IF(NP.EQ.0)GO TO 21
      DO 20 I=1,NP
        CB=(TSP-A(I))*P(I)
        CB1=CB1+CB
20     CONTINUE
21     UB1=0
      UE1=0
      IF(NW.EQ.0)GO TO 31
      DO 30 I=1,NW
        UB=(TSP-BG(I))*2/2*W(I)
        UE=- (TSP-E(I))*2/2*W(I)
        UB1=UB+UB1
        UE1=UE+UE1
30     CONTINUE
31     BU(K)=CB1+UB1+UE1
      RETURN
      END
C *****
C *                               SHEAR BOUNDARY CONDITION                               *
C *****
      SUBROUTINE SHEAR(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,ICONT,TSP)
      DOUBLE PRECISION AU(25,25),BU(25)
      DIMENSION P(30),A(30),W(30),BG(30),E(30)
      REAL L(30)
      IF(ICONT.EQ.6)GO TO 15
      SPL=0
      DO 10 I=1,N
        J=K-1
        IF((TSP-SPL).EQ.0.)GO TO 8
        AU(J,I)=(TSP-SPL)**0
        GO TO 9
      8     AU(J,I)=1.
      9     SPL=SPL+L(I)
10     CONTINUE
15     GO TO 16
      SPL=L(1)
      DO 12 I=1,NS
        IF((TSP-SPL).EQ.0.)GO TO 211
        AU(N,I)=(TSP-SPL)**0
        J=I+1
        GO TO 23
      211    AU(N,I)=1
      23    SPL=SPL+L(J)
12     CONTINUE
16     CB1=0
      IF(NP.EQ.0)GO TO 21
      DO 20 I=1,NP
        IF((TSP-A(I)).EQ.0.)GO TO 28
        CB=(TSP-A(I))*0*P(I)
        GO TO 29
      28     CB=P(I)
      29     CB1=CB1+CB
20     CONTINUE
21     UB1=0
      UE1=0
      IF(NW.EQ.0)GO TO 31
      DO 30 I=1,NW
        UB=(TSP-BG(I))*W(I)
        UE=- (TSP-E(I))*W(I)
        UB1=UB+UB1
        UE1=UE+UE1
30     CONTINUE
31     BU(K-1)=CB1+UB1+UE1

```

```

      RETURN
      END
C *****
C *                               EQUILIBRIUM                               *
C *****
      SUBROUTINE EQU(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,RL)
      DOUBLE PRECISION AU(25,25),BU(25)
      DIMENSION P(30),A(30),W(30),BG(30),E(30),R(26)
      REAL L(30)
      DO 10 I=1,NS
          R(I)=BU(I)
10      CONTINUE
      RE=0
      DO 20 I=1,NS
          CO=R(I)
          RE=CO+RE
20      CONTINUE
      PE=0
      IF(NP.EQ.0)GO TO 31
      DO 30 I=1,NP
          EO=P(I)
          PE=EO+PE
30      CONTINUE
31      WE=0
      IF(NW.EQ.0)GO TO 41
      DO 40 I=1,NW
          FO=W(I)*(E(I)-BG(I))
          WE=FO+WE
40      CONTINUE
41      AP=PE+WE
      RL=AP-RE
      RETURN
      END
C *****
C *                               CALDEF                               *
C *****
      SUBROUTINE CALDEF(N,K,NS,NP,NW,P,A,W,BG,E,L,AU,BU,R,ICONT,TSP,
      $AM1,AM2,Z1,Z2)
      DIMENSION P(30),A(30),W(30),BG(30),E(30),AU(25,25),BU(25),
      $R(26)
      REAL L(30),EM,IM
      WRITE(9,15)
15      FORMAT(15X,'ENTER FRACTION OF THE BEAM THAT DISPLACEMENT' ,/,
      $' NEEDED... ???' )
      READ(8,*)DELX
      WRITE(9,10)
10      FORMAT('ENTER VALUE OF MODULUS OF ELASTICITY AND MOMENT OF INERTIA
      $OF THE BEAM... ???' )
      READ(8,*)EM,IM
      WRITE(6,20)EM,IM
20      FORMAT(/,10X,' YOUNG' S MODULUS=' ,F12.3,2X,' (N/MM**2)' ,3X,' MOMENT OF
      $INERTIA=' ,F12.2,2X,' (MM**4)' )
      IF(ICONT.EQ.4)GO TO 77
      IF(ICONT.EQ.5)GO TO 104
      IF(ICONT.EQ.6)GO TO 105
      WRITE(6,101)AM1
101      FORMAT(/,10X,' M1 =' ,F10.2,2X,' N. MM' )
      IF(ICONT.EQ.2)GO TO 77
      IF(ICONT.EQ.3)GO TO 104
      WRITE(6,103)AM2
103      FORMAT(/,10X,' M2 =' ,F10.2,2X,' N. MM' )
      77      J=N+1
      GO TO 78
104      J=N
      GO TO 78
105      J=N-1
      78      DO 120 I=1,J

```

```

WRITE(6,130)I,R(I)
130 FORMAT(/,10X,R(' ',I2,' ') =',F10.2,2X,'(N)')
120 CONTINUE
WRITE(6,30)
30 FORMAT(//,5X,'DISTANCE',20X,'DEFLECTION',/,7X,'(MM)',24X,'(MM)',//
$)
X=0
40 SUM=0
IF(ICONT.EQ.6)GO TO 45
RE=0
DO 50 I=1,N
TERM=(X-RE)**3/6*R(I)
IF((X-RE).LT.0)TERM=0
RE=L(I)+RE
SUM=TERM+SUM
50 CONTINUE
GO TO 60
45 RE=L(1)
DO 70 I=1,NS
TERM=(X-RE)**3/6*R(I)
IF((X-RE).LT.0)TERM=0
J=I+1
RE=RE+L(J)
SUM=TERM+SUM
70 CONTINUE
60 SUM1=0
IF(NP.EQ.0)GO TO 85
DO 80 I=1,NP
TERM1=-(X-A(I))**3/6*P(I)
IF((X-A(I)).LT.0)TERM1=0
SUM1=TERM1+SUM1
80 CONTINUE
85 SUM2=0
IF(NW.EQ.0)GO TO 95
DO 90 I=1,NW
TERM2=-(X-BG(I))**4/24*W(I)
TERM3=(X-E(I))**4/24*W(I)
IF((X-BG(I)).LT.0)TERM2=0
IF((X-E(I)).LT.0)TERM3=0
SUM2=TERM2+TERM3+SUM2
90 CONTINUE
95 SUM3=-AM1*X**2/2
SUM4=Z1*X+Z2
Y=(SUM+SUM1+SUM2+SUM3+SUM4)/(EM*IM)
WRITE(6,100)X,Y
100 FORMAT(3X,'X=',2X,F8.3,15X,'Y=',F12.7)
X=X+DELX
IF(X.GT.TSP)GO TO 110
GO TO 40
110 RETURN
END
$ENTRY

```

```

*****
* EXECUTE FILE *
*****

```

```

GLOBAL MACLIB IMSLDP WATLIB
FI 8 TERMINAL
FI 9 TERMINAL
WATFIV GENERAL

```

APPENDIX D  
HAND CALCULATION OF SAMPLE PROBLEMS

As a sample hand calculation a typical portal frame subjected to a concentrated force of 5N (Figure D-1) is analyzed. Castigliano's Theorem based on the principle of minimum potential energy is used to calculate the horizontal deflection (side-sway) of the frame.

Strain energy of a structural element under pure bending is:

$$U = \int_0^L \frac{M^2}{2EI} dx \quad (D-1)$$

where,

M = bending moment

E = modulus of elasticity

I = moment of inertia

L = total length of the element

using the free-body diagram presented in Figure (D-1), potential energy for each member is as follows:

$$u_1 = \frac{1}{2EI} \int_0^L (M_1 + vx_1)^2 dx_1 \quad (D-2)$$

$$u_2 = \frac{1}{2EI} \int_0^a (M_1 + vL - Fx_2)^2 dx_2 \quad (D-3)$$

$$u_3 = \frac{1}{2EI} \int_0^L [M_2 + (Q-v)x_3]^2 dx_3 \quad (D-4)$$

Using the equilibrium equation, F is found as a function of  $M_1$ ,  $M_2$  and Q, namely,

$$F = \frac{M_1 + M_2 + QL}{a} \quad (D-5)$$



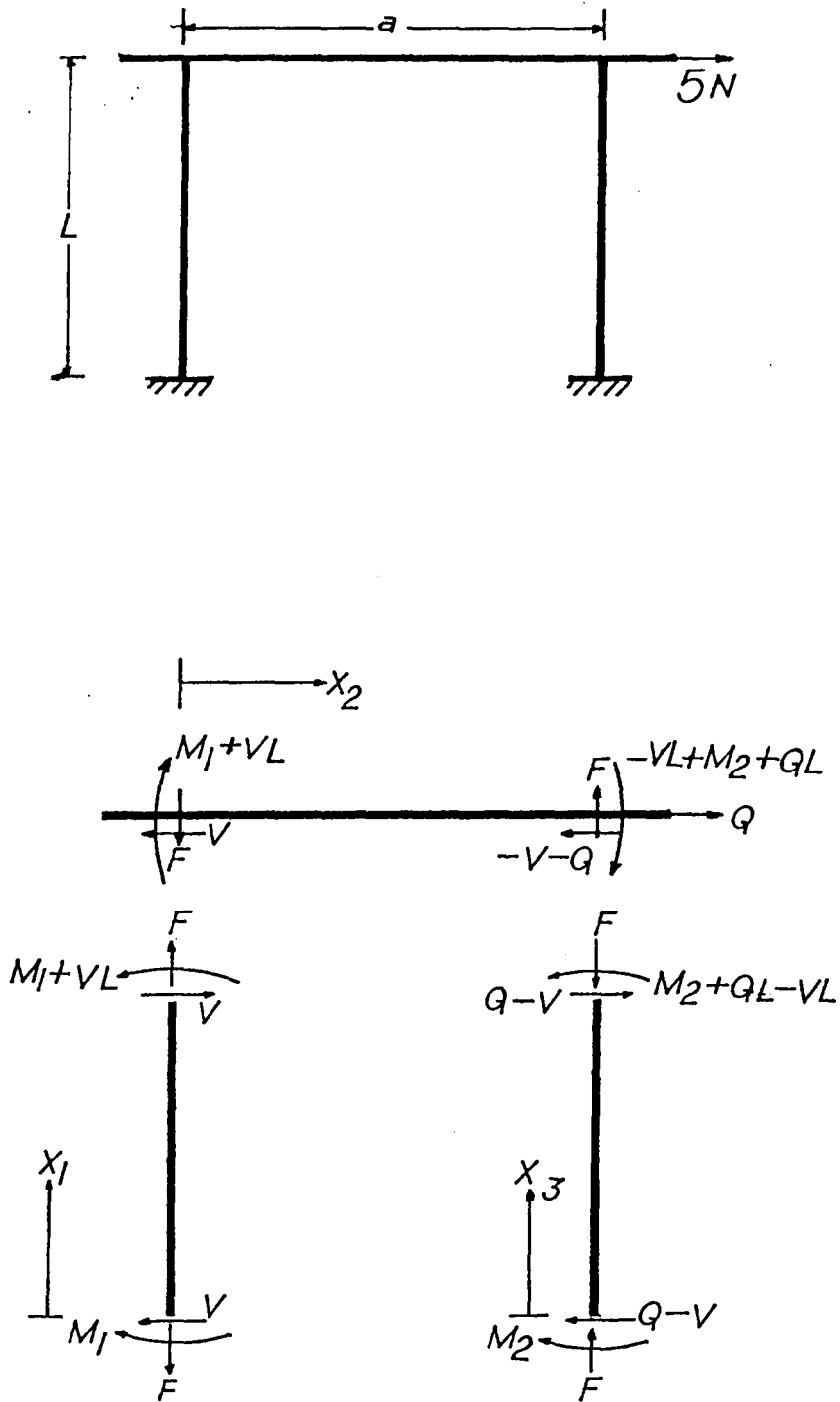


Fig. D-1. A Typical Frame and Its Free-body Diagram.

Substituting (D-5) in equation (D-3) and by adding the three equations (D-2), (D-3) and (D-4) the total strain energy of the frame is found:

$$U = \frac{1}{2EI} \int_0^L (M_1 + vx_1)^2 dx_1 + \int_0^a [M_1 + vL - (M_1 + M_2 + QL) \frac{x_2}{a}]^2 dx_2 + \int_0^L [M_2 + (Q-v)x]^2 dx_3 \quad (D-6)$$

According to Castigliano's Theorem, the partial derivative of total strain energy (D-6) with respect to horizontal force Q yields the horizontal deflection (side-sway) of the frame.

$$\frac{\partial U}{\partial Q} = \Delta_H \quad (D-7)$$

The above procedure is used for a typical case when the frame is supported on pins at both ends. Since rotation is allowed at both ends of the frame,  $M_1$  and  $M_2$  in equation (D-6) are zero. Therefore,

$$U = \frac{1}{2EI} \left[ \int_0^L (vx_1)^2 dx_1 + \int_0^a (vL - QL \frac{x_2}{a})^2 dx_2 + \int_0^L (Q-v)^2 x_3^2 dx_3 \right] \quad (D-8)$$

To calculate horizontal reaction  $v$  at the supports:

$$\frac{\partial U}{\partial v} = \Delta_v = 0 \quad (D-9)$$

which results:

$$v = \frac{1}{2} Q \quad (D-10)$$

Horizontal deflection (side-sway) of the frame is found by

taking the partial derivative of U in (D-8) with respect to Q after substituting for v in terms of Q from (D-10).

This yields,

$$\frac{\partial U}{\partial Q} = \Delta_H = .5734 \text{ mm} \quad (\text{D-11})$$

which is compared to the STRUDL result  $\Delta_H = 0.5744 \text{ mm}$ .

APPENDIX E  
UNCERTAINTY ANALYSIS

## APPENDIX E

## UNCERTAINTY ANALYSIS

A precise method of estimating uncertainty in experimental results is based on a careful specification of the uncertainties in the various primary experimental measurements. Suppose we wish to estimate the uncertainty in the calculated result on the basis of the uncertainties in the primary measurements. The result  $R$  is a given function of the independent variables  $x_1, x_2, \dots, x_n$

$$R = R (x_1, x_2, \dots, x_n)$$

let  $W_R$  be the uncertainty in the result and  $W_1, W_2, \dots, W_n$  be the uncertainties in the independent variables. If the uncertainties in the independent variables are all given with the same odds, then the uncertainty in the result having these odds is given as:

$$W_R = \left[ \left( \frac{\partial R}{\partial x_1} W_1 \right)^2 + \left( \frac{\partial R}{\partial x_2} W_2 \right)^2 + \dots + \left( \frac{\partial R}{\partial x_n} W_n \right)^2 \right]^{1/2}$$

EXAMPLE: The area of a rectangle is given as  $A = bh$ . The base  $b$ , and height,  $h$ , are measured with a ruler marked in millimeters. The uncertainty in its measuring ability is

+0.5 mm. B is measured as 10 mm and h as 22mm. Find the uncertainty in the area measurement using the measurements for b and h.

$$W_A = \left[ \left( \frac{\partial A}{\partial b} W_b \right)^2 + \left( \frac{\partial A}{\partial h} W_h \right)^2 \right]^{1/2}$$

$$\frac{\partial A}{\partial b} = h, \quad \frac{\partial A}{\partial h} = b, \quad W_b = W_h = 0.5 \text{ mm}$$

$$W_A = [(hW_b)^2 + (bW_h)^2]^{1/2}$$

$$W_A = ([22)(0.5)]^2 + [(10)(0.5)]^2)^{1/2}$$

$$W_A = 12.1 \text{ mm}^2$$

$$A = bh = (10)(22) = 220$$

$$A = 220 \pm 12 \text{ mm}^2$$

APPENDIX F  
LISTING OF STRUDL INPUT AND OUTPUT  
FOR PORTAL FRAME TESTS







LOADING COMB 30 '3 10N CONCENTRATED LOADS ON JOINTS 1&3&5'  
 COMBINE 30 6 2. 5 2. 7 2.  
 LOADING COMB 35 '2 10N CONCENTRATED LOADS ON JOINTS 1&5'  
 COMBINE 35 6 2. 7 2.  
 UNIT IN  
 PLOT DEVI PRIN COL 10 ROWS 10  
 PLOT FORMAT ORIEN NONSTANDARD  
 PLOT PLAN

PLANE IDENTIFIED BY - PLANE Z EQUALS 0.0

IN PLANE JOINTS	COORDINATES		
JOINT	X	Y	Z
1	0.0	17.7165	0.0
2	5.827	17.7165	0.0
3	11.654	17.7165	0.0
4	17.481	17.7165	0.0
5	23.308	17.7165	0.0
6	29.135	0.0	0.0
7	27.2440	0.0	0.0

IN PLANE MEMBERS  
 MEMBER INCIDENCES

MEMBER	START	END
1	1	2
2	2	3
3	3	4
4	4	5
5	1	5
6	6	7



1	ACTIVE
2	ACTIVE
3	ACTIVE
4	ACTIVE
5	ACTIVE
6	ACTIVE
7	ACTIVE

RELEASES-----/						ELASTIC SUPPORT RELEASES-----				
JOINT	FORCE	MOMENT	THETA 1	THETA 2	THETA 3	KFX	KFY	KFZ	KMX	KMY
6		Z	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7	X	Z	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\*  
 \* END OF DATA FROM INTERNAL STORAGE \*  
 \*\*\*\*\*

LIST DISP ALL

\*\*\*\*\*  
 \*RESULTS OF LATEST ANALYSES\*  
 \*\*\*\*\*

PROBLEM - FRAME      TITLE - ANALYSIS

ACTIVE UNITS    CM    KG    RAD    DEGF    SEC

ACTIVE STRUCTURE TYPE    PLANE    FRAME

ACTIVE COORDINATE AXES    X    Y

LOADING - 1                    UNIFORM LOAD ON BEAM

RESULTANT JOINT

DISPLACEMENTS -    SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0022975
7	GLOBAL	0.2071300	0.0				0.0023054

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 MIDSPAN	GLOBAL	0.1033872	0.0206280				-0.0022606
	GLOBAL	0.1033872	-0.0000276				-0.0022975
	GLOBAL	0.1033872	-0.0445787				-0.0000020
	GLOBAL	0.1033872	-0.0000273				0.0023054
	GLOBAL	0.1033872	0.0200401				0.0022721

LOADING - 2

SIDESWAY CONC.LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0043403
7	GLOBAL	0.2068685	0.0				0.0010220

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 MIDSPAN	GLOBAL	0.1608725	0.0186076				-0.0020443
	GLOBAL	0.1608725	0.0000041				-0.0020443
	GLOBAL	0.1608725	-0.0230357				0.0002554
	GLOBAL	0.1608725	-0.0000041				0.0010220
	GLOBAL	0.1608809	0.0089892				0.0010220

LOADING - 3

CONC. LOAD ON MIDSPAN OF BEAM

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0005119
7	GLOBAL	0.0460714	0.0				0.0005119

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0230357	0.0046556				-0.0005119
	GLOBAL	0.0230357	-0.0000027				-0.0005119
	GLOBAL	0.0230357	-0.0102579				-0.0000000
	GLOBAL	0.0230357	-0.0000027				0.0005119
	GLOBAL	0.0230357	0.0045020				0.0005119

LOADING - 4 SIDESWAY LOAD ALONG THE COLUMN 6

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0048535
7	GLOBAL	0.2978636	0.0				0.0026278

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.1839691	0.0232762				-0.0025576
	GLOBAL	0.1839691	0.0000020				-0.0025576
	GLOBAL	0.1839727	-0.0346047				0.0001271
	GLOBAL	0.1839764	-0.0000020				0.0020487
	GLOBAL	0.1839764	0.0180264				0.0020487

LOADING - 5 SIDESWAY LOAD ALONG THE COLUMN 5

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0058465
7	GLOBAL	-0.2506961	0.0				-0.0011083

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.

UNIFORM	GLOBAL	-0.2008240	-0.0201789			0.0022170
	GLOBAL	-0.2008240	-0.0000044			0.0022170
	GLOBAL	-0.2008240	0.0249809			-0.0002770
	GLOBAL	-0.2008240	0.0000044			-0.0011083
	GLOBAL	-0.2008240	-0.0097483			-0.0011083

LOADING - 6 CON VERTICAL LOAD ON JOINT 1

RESULTANT JOINT DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0004135
7	GLOBAL	-0.0279034	0.0			-0.0002066

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIFORM	GLOBAL	-0.0186076	-0.0043387			0.0005074
	GLOBAL	-0.0186076	-0.0000063			0.0004135
	GLOBAL	-0.0186076	0.0046556			-0.0000516
	GLOBAL	-0.0186076	0.0000008			-0.0002066
	GLOBAL	-0.0186076	-0.0018170			-0.0002066

LOADING - 7 CONC VERTICAL LOAD ON JOINT 5

RESULTANT JOINT DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0001998
7	GLOBAL	-0.0269835	0.0			-0.0003999

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIFORM	GLOBAL	-0.0089892	-0.0018170			0.0001998
	GLOBAL	-0.0089892	0.0000008			0.0001998
	GLOBAL	-0.0089892	0.0045020			0.0000499
	GLOBAL	-0.0089892	-0.0000062			-0.0003999
	GLOBAL	-0.0089892	-0.0040402			-0.0004877

LOADING - 10 UNIF.LOAD ON BEAM+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0066377
7	GLOBAL	0.4139984	0.0				0.0033274

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 COLUMN	GLOBAL	0.2642597	0.0392356				-0.0043050
	GLOBAL	0.2642597	-0.0000235				-0.0043418
	GLOBAL	0.2642633	-0.0676144				0.0002534
	GLOBAL	0.2642670	-0.0000314				0.0032274
	GLOBAL	0.2642680	0.0290293				0.0032940
	GLOBAL	0.2642680	0.0290293				0.0032940

LOADING - 15 VERTICAL LOAD ON BEAM MIDSPAN+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0048522
7	GLOBAL	0.2529398	0.0				0.0015339

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 COLUMN	GLOBAL	0.1839082	0.0232632				-0.0025562
	GLOBAL	0.1839082	0.0000014				-0.0025562
	GLOBAL	0.1839119	-0.0332935				0.0002556
	GLOBAL	0.1839152	-0.0000068				0.0015339
	GLOBAL	0.1839165	0.0134913				0.0015339
	GLOBAL	0.1839165	0.0134913				0.0015339

LOADING - 20 SIDESWAY LOAD ON J 5+SIDESWAY LOAD ON COLUMN #6

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.



DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0091223	-0.0030003			0.0003905
2	GLOBAL	-0.0091223	-0.0000164			0.0002027
3	GLOBAL	-0.0091223	-0.0022004			-0.0000034
4	GLOBAL	-0.0091223	-0.0000163			-0.0001891
5	GLOBAL	-0.0091223	-0.0027104			-0.0003647

LOADING - 35                    2 10N CONCENTRATED LOADS ON JOINTS 1&5

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0012265
7	GLOBAL	-0.1097739	0.0			-0.0012129

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0551937	-0.0123115			0.0014143
2	GLOBAL	-0.0551937	-0.0000109			0.0012265
3	GLOBAL	-0.0551937	0.0183153			-0.0000034
4	GLOBAL	-0.0551937	-0.0000108			-0.0012129
5	GLOBAL	-0.0551937	-0.0117145			-0.0013885

\$ FIXED-ROLLER SUPPORT CONDITION  
 DELETIONS  
 JOINT RELEASES  
 6 MOM Z  
 STIFFNES ANALYSIS  
 PRINT JOINT STATUS

\*\*\*\*\*  
 \* PROBLEM DATA FROM INTERNAL STORAGE \*  
 \*\*\*\*\*

JOB ID -	FRAME	JOB TITLE -	ANALYSIS	TEMPERATURE	TIME
ACTIVE UNITS -	LENGTH	WEIGHT	ANGLE	DEGF	SEC
	CM	KG	RAD	STATUS----	
JOINT					
JOINT					
1				ACTIVE	
2				ACTIVE	
3					

		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0091938
7	GLOBAL	0.5047321	0.0				0.0036497

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	0.3448417	0.0418838			-0.0046019
	GLOBAL	0.3448417	0.0000061			-0.0046019
	GLOBAL	0.3448489	-0.0576404			0.0003825
	GLOBAL	0.3448561	-0.0000061			0.0030707
	GLOBAL	0.3448572	0.0270156			0.0030707

LOADING - 25

3 CONCENTRATED LOADS ON J 1&3&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0001014
7	GLOBAL	-0.0088156	0.0			-0.0000945

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	-0.0045612	-0.0015002			0.0001952
	GLOBAL	-0.0045612	-0.0000082			0.0001014
	GLOBAL	-0.0045612	-0.0011002			-0.0000017
	GLOBAL	-0.0045612	-0.0000081			-0.0000945
	GLOBAL	-0.0045612	-0.0013552			-0.0001823

LOADING - 30

3 10N CONCENTRATED LOADS ON JOINTS 1&3&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0002027
7	GLOBAL	-0.0176312	0.0			-0.0001891

RESULTANT JOINT

		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0091938
7	GLOBAL	0.5047321	0.0				0.0036497

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.3448417	0.0418838				-0.0046019
	GLOBAL	0.448417	0.0000061				-0.0046019
	GLOBAL	0.448489	-0.0576404				0.0003825
	GLOBAL	0.448561	-0.0000061				0.0030707
	GLOBAL	0.5448572	0.0270156				0.0030707

LOADING - 25

3 CONCENTRATED LOADS ON J 1&3&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0001014
7	GLOBAL	-0.0088156	0.0				-0.0000945

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0045612	-0.0015002				0.0001952
	GLOBAL	-0.0045612	-0.0000082				0.0001014
	GLOBAL	-0.0045612	-0.0011002				-0.0000017
	GLOBAL	-0.0045612	-0.0000082				-0.0000945
	GLOBAL	-0.0045612	-0.0013552				-0.0001823

LOADING - 30

3 10N CONCENTRATED LOADS ON JOINTS 1&3&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0002027
7	GLOBAL	-0.0176312	0.0				-0.0001891

RESULTANT JOINT

4 ACTIVE  
 5 ACTIVE  
 6 ACTIVE  
 7 ACTIVE

RELEASES-----/						ELASTIC SUPPORT RELEASES-----/				
JOINT	FORCE	MOMENT	THETA 1	THETA 2	THETA 3	KFX	KFY	KFZ	KMX	KMY
7	X	Z	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\*  
 \* END OF DATA FROM INTERNAL STORAGE \*  
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LIST DISP ALL

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 \*RESULTS OF LATEST ANALYSES\*  
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PROBLEM - FRAME TITLE - ANALYSIS

ACTIVE UNITS CM KG RAD DEGF SEC

ACTIVE STRUCTURE TYPE PLANE FRAME

ACTIVE COORDINATE AXES X Y

LOADING - 1 UNIFORM LOAD ON BEAM

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT-----/			ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.1235899	0.0				0.0019516

RESULTANT JOINT

FREE JOINTS

DISPLACEMENTS -

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	0.0357688	0.0141859				-0.0015529
	GLOBAL	0.0357688	-0.0000290				-0.0015897
	GLOBAL	0.0357688	-0.0366036				-0.0000904
	GLOBAL	0.0357688	-0.0000259				0.0019516
	GLOBAL	0.0357688	0.0169279				0.0019182

LOADING - 2

SIDESWAY CONC.LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6 7	GLOBAL	0.0	0.0				0.0
	GLOBAL	0.0490509	0.0				0.0003536

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	0.0331330	0.0064377				-0.0007073
	GLOBAL	0.0331330	0.0000014				-0.0007073
	GLOBAL	0.0331367	-0.0079696				0.0000884
	GLOBAL	0.0331403	-0.0000014				0.0003536
	GLOBAL	0.0331414	0.0031100				0.0003536

LOADING - 3

CONC. LOAD ON MIDSPAN OF BEAM

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6 7	GLOBAL	0.0	0.0				0.0
	GLOBAL	0.0274578	0.0				0.0004331

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2	GLOBAL	0.0079696	0.0032202				-0.0003542
	GLOBAL	0.0079696	-0.0000030				-0.0003542

MEM	GLOBAL	0.0079696	-0.0084809				-0.0000197
	GLOBAL	0.0079696	-0.0000024				0.0004331
	GLOBAL	0.0079696	0.0038086				0.0004331

LOADING - 4                    SIDESWAY LOAD ALONG THE COLUMN 6

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	0.1213834	0.0			0.0018803

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
MEM	GLOBAL	0.0411237	0.0096671			-0.0010624
	GLOBAL	0.0411237	-0.0000010			-0.0010624
	GLOBAL	0.0411237	-0.0177570			-0.0000697
	GLOBAL	0.0411237	0.0000010			0.0013012
	GLOBAL	0.0411237	0.0000010			0.0013012
	GLOBAL	0.0411310	0.0114519			0.0013012

LOADING - 5                    SIDESWAY LOAD ALONG THE COLUMN 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	-0.0381083	0.0			-0.0002079

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
MEM	GLOBAL	-0.0287527	-0.0037854			0.0004159
	GLOBAL	-0.0287527	-0.0000008			0.0004159
	GLOBAL	-0.0287527	0.0046862			-0.0000520
	GLOBAL	-0.0287527	0.0000008			-0.0002079
	GLOBAL	-0.0287527	0.0000008			-0.0002079
	GLOBAL	-0.0287527	-0.0018287			-0.0002079

LOADING - 6                    CON VERTICAL LOAD ON JOINT 1

RESULTANT JOINT  
DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	-0.0128679	0.0			-0.0001429

RESULTANT JOINT  
DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	-0.0064377	-0.0031793			0.0003800
	GLOBAL	-0.0064377	-0.0000060			0.0002861
	GLOBAL	-0.0064377	0.0032202			-0.0000356
	GLOBAL	-0.0064377	0.0000006			-0.0001429
	GLOBAL	-0.0064377	-0.0012569			-0.0001429
	GLOBAL	-0.0064377	-0.0012569			-0.0001429

LOADING - 7                    CONC VERTICAL LOAD ON JOINT 5

RESULTANT JOINT  
DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	-0.0197199	0.0			-0.0003691

RESULTANT JOINT  
DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	-0.0031100	-0.0012569			0.0001382
	GLOBAL	-0.0031100	0.0000009			0.0001382
	GLOBAL	-0.0031100	0.0038086			0.0000575
	GLOBAL	-0.0031100	-0.0000044			-0.0003691
	GLOBAL	-0.0031100	-0.0037696			-0.0004569
	GLOBAL	-0.0031100	-0.0037696			-0.0004569

LOADING - 10                    UNIF.LOAD ON BEAM+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT  
DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	-0.0197199	0.0			-0.0003691

6	GLOBAL	0.0	0.0	0.0
7	GLOBAL	0.1726407	0.0	0.0023052

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 5	GLOBAL	0.0689018	0.0206235				-0.0022601
	GLOBAL	0.0689018	-0.0000275				-0.0022970
	GLOBAL	0.0689054	-0.0445732				-0.0000020
	GLOBAL	0.0689090	-0.0000273				0.0023052
	GLOBAL	0.0689101	0.0200379				0.0022718

LOADING - 15

VERTICAL LOAD ON BEAM MIDSPAN+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0765087	0.0				0.0007866

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 5	GLOBAL	0.0411027	0.0096579				-0.0010615
	GLOBAL	0.0411027	-0.0000016				-0.0010615
	GLOBAL	0.0411063	-0.0164506				0.0000687
	GLOBAL	0.0411099	-0.0000038				0.0007866
	GLOBAL	0.0411110	0.0069186				0.0007866

LOADING - 20

SIDESWAY LOAD ON J 5+SIDESWAY LOAD ON COLUMN #6

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.1704342	0.0				0.0022339

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS



JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNFINISHED	GLOBAL	0.0742567	0.0161047				-0.0017697
	GLOBAL	0.0742567	0.0000005				-0.0017697
	GLOBAL	0.0742640	-0.0257266				0.0000287
	GLOBAL	0.0742713	-0.0000005				0.0016548
	GLOBAL	0.0742723	0.0145619				0.0016548

LOADING - 25 3 CONCENTRATED LOADS ON J 1&3&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	-0.0051300	0.0				-0.0000789

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNFINISHED	GLOBAL	-0.0015780	-0.0012159				0.0001640
	GLOBAL	-0.0015780	-0.0000081				0.0000701
	GLOBAL	-0.0015780	-0.0014521				0.0000022
	GLOBAL	-0.0015780	-0.0000082				-0.0000789
	GLOBAL	-0.0015780	-0.0012179				-0.0001667

LOADING - 30 3 10N CONCENTRATED LOADS ON JOINTS 1&3&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	-0.0102601	0.0				-0.0001579

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNFINISHED	GLOBAL	-0.0031561	-0.0024319				0.0003280
	GLOBAL	-0.0031561	-0.0000163				0.0001403
	GLOBAL	-0.0031561	-0.0029041				0.0000044

4	GLOBAL	-0.0031561	-0.0000164				-0.0001579
5	GLOBAL	-0.0031561	-0.0024358				-0.0003335

LOADING - 35                    2 10N CONCENTRATED LOADS ON JOINTS 1&5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT-----/			-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	-0.0651757	0.0				-0.0010240

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT-----/			-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0190953	-0.0088724				0.0010365
2	GLOBAL	-0.0190953	-0.0000102				0.0008487
3	GLOBAL	-0.0190953	0.0140577				0.0000438
4	GLOBAL	-0.0190953	-0.0000116				-0.0010240
5	GLOBAL	-0.0190953	-0.0100530				-0.0011996

\$ PIN-PIN SUPPORT CONDITION  
 CHANGES  
 JOINT RELEASES  
 6 MOM Z  
 7 MOM Z  
 STIFFNES ANALYSIS  
 PRINT JOINT STATUS

\*\*\*\*\*  
 \* PROBLEM DATA FROM INTERNAL STORAGE \*  
 \*\*\*\*\*

JOB ID - FRAME	LENGTH	WEIGHT	ANGLE	TEMPERATURE	TIME
	CM	KG	RAD	DEGF	SEC
JOINT				STATUS----	
JOINT					
1				ACTIVE	
2				ACTIVE	
3				ACTIVE	
4				ACTIVE	
5				ACTIVE	
6				ACTIVE	

7

ACTIVE

RELEASES-----/					ELASTIC SUPPORT RELEASES-----					
JOINT	FORCE	MOMENT	THETA 1	THETA 2	THETA 3	KFX	KFY	KFZ	KMX	KMY
6		Z	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7		Z	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\*  
 \* END OF DATA FROM INTERNAL STORAGE \*  
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LIST DISP ALL

\*\*\*\*\*  
 \*RESULTS OF LATEST ANALYSES\*  
 \*\*\*\*\*

PROBLEM - FRAME TITLE - ANALYSIS

ACTIVE UNITS CM KG RAD DEGF SEC

ACTIVE STRUCTURE TYPE PLANE FRAME

ACTIVE COORDINATE AXES X Y

LOADING - 1 UNIFORM LOAD ON BEAM

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT-----/			ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0003871
7	GLOBAL	0.0	0.0				-0.0003792

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT-----/			ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.

LOADING - 2	GLOBAL	-0.0001760	0.0066584				
	GLOBAL	-0.0001760	-0.0000276				-0.0007255
	GLOBAL	-0.0001778	-0.0215135				-0.0007624
	GLOBAL	-0.0001796	-0.0000273				-0.0000020
	GLOBAL	-0.0001796	0.0065310				0.0007703

LOADING - 2 SIDESWAY CONC.LOAD ON JOINT 5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				
7	GLOBAL	0.0	0.0				-0.0016591
							-0.0016592

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0574402	0.0046556				
	GLOBAL	0.0574402	0.0000041				-0.0005112
	GLOBAL	0.0574420	0.0000004				-0.0005112
	GLOBAL	0.0574438	-0.0000041				0.0002554
	GLOBAL	0.0574449	-0.0045028				-0.0005112
							-0.0005112

LOADING - 3 CONC. LOAD ON MIDSPAN OF BEAM

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				
7	GLOBAL	0.0	0.0				0.0000852
							-0.0000852

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0000004	0.0015484				
	GLOBAL	0.0000004	-0.0000027				-0.0001705
	GLOBAL	-0.0000000	-0.0051275				-0.0001705
	GLOBAL	-0.0000004	-0.0000027				-0.0000000
	GLOBAL	-0.0000004	0.0014972				0.0001705
							0.0001705

LOADING - 4 SIDESWAY LOAD ALONG THE COLUMN 6

RESULTANT JOINT  
DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0009930
7	GLOBAL	0.0	0.0				-0.0012327

RESULTANT JOINT  
DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 5	GLOBAL	0.0350399	0.0031872				-0.0003500
	GLOBAL	0.0350399	0.0000020				-0.0003500
	GLOBAL	0.0350409	-0.0014357				0.0001271
	GLOBAL	0.0350419	-0.0000020				-0.0001589
	GLOBAL	0.0350419	-0.0014004				-0.0001589

LOADING - 5 SIDESWAY LOAD ALONG THE COLUMN 5

RESULTANT JOINT  
DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0025973
7	GLOBAL	0.0	0.0				0.0021409

RESULTANT JOINT  
DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 5	GLOBAL	-0.0754782	-0.0032711				0.0003590
	GLOBAL	-0.0754782	-0.0000044				0.0003590
	GLOBAL	-0.0754760	-0.0029356				-0.0002770
	GLOBAL	-0.0754738	0.0000044				0.0007497
	GLOBAL	-0.0754738	0.0066021				0.0007497

LOADING - 6 CON VERTICAL LOAD ON JOINT 1

RESULTANT JOINT  
DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.

		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0000519
7	GLOBAL	0.0	0.0				0.0001551

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIFORM	GLOBAL	-0.0046561	-0.0024568			0.0003006
	GLOBAL	-0.0046561	-0.0000063			0.0002067
	GLOBAL	-0.0046561	0.0015484			-0.0000516
	GLOBAL	-0.0046561	0.0000008			0.0000002
	GLOBAL	-0.0046556	0.0000028			0.0000002

LOADING - 7                    CONC VERTICAL LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			-0.0001500
7	GLOBAL	0.0	0.0			-0.0000501

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIFORM	GLOBAL	0.0045023	0.0000028			-0.0000002
	GLOBAL	0.0045023	0.0000008			-0.0000002
	GLOBAL	0.0045023	0.0014972			-0.0000495
	GLOBAL	0.0045023	-0.0000062			-0.0001999
	GLOBAL	0.0045028	-0.0022803			-0.0002877

LOADING - 10                    UNIF.LOAD ON BEAM+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			-0.0012720
7	GLOBAL	0.0	0.0			-0.0020383

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 GLOBAL GLOBAL GLOBAL GLOBAL GLOBAL	0.0572642	0.0113140				-0.0012367
	0.0572642	-0.0000235				-0.0012735
	0.0572642	-0.0215131				0.0002534
	0.0572642	-0.0000314				0.0002591
	0.0572653	0.0020283				0.0002257

LOADING - 15                      VERTICAL LOAD ON BEAM MIDSPAN+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	0.0	0.0				-0.0015739
7	0.0	0.0				-0.0017444

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 GLOBAL GLOBAL GLOBAL GLOBAL GLOBAL	0.0574406	0.0062040				-0.0006816
	0.0574406	0.0000014				-0.0006816
	0.0574420	-0.0051271				0.0002554
	0.0574434	-0.0000068				-0.0003408
	0.0574445	-0.0030055				-0.0003408

LOADING - 20                      SIDESWAY LOAD ON J 5+SIDESWAY LOAD ON COLUMN #6

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	0.0	0.0				-0.0026521
7	0.0	0.0				-0.0028919

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.

1 2 3 4 5	GLOBAL	0.0924801	0.0078428				
	GLOBAL	0.0924801	0.0000061				-0.0008612
	GLOBAL	0.0924829	-0.0000061				-0.0003812
	GLOBAL	0.0924857	-0.0014353				0.0003825
	GLOBAL	0.0924857	-0.0000061				-0.0006701
	GLOBAL	0.0924868	-0.0059031				-0.0006701

LOADING - 25                    3 CONCENTRATED LOADS ON J 1&3&5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				
7	GLOBAL	0.0	0.0				-0.0000129
							0.0000197

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0001534	-0.0009056				0.0001299
2	GLOBAL	-0.0001534	-0.0000082				0.0000360
3	GLOBAL	-0.0001534	-0.0020819				-0.0000017
4	GLOBAL	-0.0001534	-0.0000081				-0.0000292
5	GLOBAL	-0.0001533	-0.0007602				-0.0001170

LOADING - 30                    3 10N CONCENTRATED LOADS ON JOINTS 1&3&5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				
7	GLOBAL	0.0	0.0				-0.0000258
							0.0000394

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0003069	-0.0018112				0.0002598
2	GLOBAL	-0.0003069	-0.0000164				0.0000720
3	GLOBAL	-0.0003067	-0.0041638				-0.0000034
4	GLOBAL	-0.0003066	-0.0000163				-0.0000584
5	GLOBAL	-0.0003066	-0.0015605				-0.0002340

LOADING - 35                    2 @ 10N CONCENTRATED LOADS ON JOINTS 1&5



RESULTANT JOINT  
DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			-0.0001962
7	GLOBAL	0.0	0.0			0.0002099

RESULTANT JOINT  
DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0003077	-0.0049080			0.0006007
2	GLOBAL	-0.0003077	-0.0000109			0.0004129
3	GLOBAL	-0.0003067	0.0060913			-0.0000034
4	GLOBAL	-0.0003058	-0.0000108			-0.0003993
5	GLOBAL	-0.0003058	-0.0045550			-0.0005749

\$ FIXED-PIN SUPPORT CONDITION  
DELETIONS  
JOINT RELEASES  
6 MOM Z  
STIFFNES ANALYSIS  
PRINT JOINT STATUS

\*\*\*\*\*  
\* PROBLEM DATA FROM INTERNAL STORAGE \*  
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JOBTITLE - ANALYSIS	TEMPERATURE	TIME
JOINT	DEGF	SEC
JOINT	STATUS---/	
1	ACTIVE	
2	ACTIVE	
3	ACTIVE	
4	ACTIVE	
5	ACTIVE	
6	ACTIVE	
7	ACTIVE	

RELEASES-----/						ELASTIC SUPPORT RELEASES-----				
JOINT	FORCE	MOMENT	THETA 1	THETA 2	THETA 3	KFX	KFY	KFZ	KMX	KMY
7		Z	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\*  
 \* END OF DATA FROM INTERNAL STORAGE \*  
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LIST DISP ALL

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 \*RESULTS OF LATEST ANALYSES\*  
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PROBLEM - FRAME TITLE - ANALYSIS  
 ACTIVE UNITS CM KG RAD DEGF SEC  
 ACTIVE STRUCTURE TYPE PLANE FRAME  
 ACTIVE COORDINATE AXES X Y  
 LOADING - 1 UNIFORM LOAD ON BEAM

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS								
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/					
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.			
6	GLOBAL	0.0	0.0				0.0			
7	GLOBAL	0.0	0.0				-0.0006114			

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS								
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/					
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.			
UNRVRN1	GLOBAL	0.0080602	0.0069158				-0.0007537			
	GLOBAL	0.0080602	-0.0000272				-0.0007906			
	GLOBAL	0.0080592	-0.0210905				0.0000222			
	GLOBAL	0.0080592	-0.0000278				0.0006827			
	GLOBAL	0.0080591	0.0057866				0.0006827			
	GLOBAL	0.0080591	0.0057866				0.0006524			

LOADING - 2 SIDESWAY CONC.LOAD ON JOINT 5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				-0.0006636

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
1 2 3 4 5	GLOBAL	0.0221359	0.0035523				-0.0003901
	GLOBAL	0.0221359	0.0000021				-0.0003901
	GLOBAL	0.0221388	-0.0018127				-0.0001346
	GLOBAL	0.0221416	-0.0000021				-0.0001488
	GLOBAL	0.0221426	-0.0013118				-0.0001488

LOADING - 3

CONC. LOAD ON MIDSPAN OF BEAM

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				-0.0001363

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
1 2 3 4 5	GLOBAL	0.0018137	0.0016051				-0.0001767
	GLOBAL	0.0018137	-0.0000026				-0.0001767
	GLOBAL	0.0018132	-0.0050344				0.0000062
	GLOBAL	0.0018127	-0.0000028				0.0001518
	GLOBAL	0.0018127	0.0013334				0.0001518

LOADING - 4

SIDESWAY LOAD ALONG THE COLUMN 6

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			

6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				-0.0006369

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	0.0139098	0.0025268				-0.0002776
	GLOBAL	0.0139098	0.0000009				-0.0002776
	GLOBAL	0.0139114	-0.0025209				0.0000549
	GLOBAL	0.0139131	-0.0000009				0.0000580
	GLOBAL	0.0139131	0.0005095				0.0000580

LOADING - 5 SIDESWAY LOAD ALONG THE COLUMN 5

RESULTANT JOINT DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0005824

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	-0.0202089	-0.0015437				0.0001695
	GLOBAL	-0.0202089	-0.0000014				0.0001695
	GLOBAL	-0.0202082	-0.0000971				-0.0000879
	GLOBAL	-0.0202076	0.0000014				0.0001824
	GLOBAL	-0.0202076	0.0016067				0.0001824

LOADING - 6 CON VERTICAL LOAD ON JOINT 1

RESULTANT JOINT DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0001240

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIFORM	GLOBAL	-0.0004227	-0.0024223				0.0002968
	GLOBAL	-0.0004227	-0.0000000				0.0002029
	GLOBAL	-0.0004227	0.0016000				-0.0000478
	GLOBAL	-0.0004227	0.0000000				-0.0000111
	GLOBAL	-0.0004227	0.0000008				-0.0000111
	GLOBAL	-0.0004227	-0.0000969				-0.0000111

LOADING - 7                    CONC VERTICAL LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0000398

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIFORM	GLOBAL	0.0013112	-0.0000969				0.0000107
	GLOBAL	0.0013112	0.0000006				0.0000107
	GLOBAL	0.0013112	0.0013334				0.0000389
	GLOBAL	0.0013112	-0.0000061				-0.0001671
	GLOBAL	0.0013112	-0.0019919				-0.0002549
	GLOBAL	0.0013112					

LOADING - 10                    UNIF.LOAD ON BEAM+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				-0.0012750

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIFORM	GLOBAL	0.0301961	0.0104681				-0.0011439
	GLOBAL	0.0301961	-0.0000250				-0.0011807
	GLOBAL	0.0301969	-0.0229033				0.0001608

5	GLOBAL	0.0301977	-0.0000299				0.0005369
	GLOBAL	0.0301987	0.0044748				0.0005036

LOADING - 15                      VERTICAL LOAD ON BEAM MIDSPAN+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				-0.0008000

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
1	GLOBAL	0.0239496	0.0051573				-0.0005668
	GLOBAL	0.0239496	-0.0000005				-0.0005668
	GLOBAL	0.0239519	-0.0068472				0.0001408
	GLOBAL	0.0239543	-0.0000050				0.0000030
	GLOBAL	0.0239554	0.0000215				0.0000030
	GLOBAL	0.0239554	0.0000215				0.0000030

LOADING - 20                      SIDESWAY LOAD ON J 5+SIDESWAY LOAD ON COLUMN #6

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				-0.0013005

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
1	GLOBAL	0.0360458	0.0060791				-0.0006677
	GLOBAL	0.0360458	0.0000030				-0.0006677
	GLOBAL	0.0360502	-0.0043337				0.0001895
	GLOBAL	0.0360546	-0.0000030				-0.0000908
	GLOBAL	0.0360557	-0.0008024				-0.0000908
	GLOBAL	0.0360557	-0.0008024				-0.0000908

LOADING - 25                      3 CONCENTRATED LOADS ON J 1&3&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	0.0	0.0			0.0000275

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNM315	GLOBAL	-0.0004279	-0.0009142			0.0001309
	GLOBAL	-0.0004279	-0.0000082			0.0000370
	GLOBAL	-0.0004278	-0.0020960			-0.0000026
	GLOBAL	-0.0004277	-0.0000081			-0.0000264
	GLOBAL	-0.0004277	-0.0007554			-0.0001142

LOADING - 30

3 10N CONCENTRATED LOADS ON JOINTS 1&3&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	0.0	0.0			0.0000549

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNM315	GLOBAL	-0.0008558	-0.0018283			0.0002617
	GLOBAL	-0.0008558	-0.0000164			0.0000739
	GLOBAL	-0.0008556	-0.0041920			-0.0000053
	GLOBAL	-0.0008554	-0.0000152			-0.0000528
	GLOBAL	-0.0008554	-0.0015109			-0.0002284

LOADING - 35

2 10N CONCENTRATED LOADS ON JOINTS 1&5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.

6 GLOBAL 0.0 0.0 0.0  
 7 GLOBAL 0.0 0.0 0.0003276

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0044831	-0.0050384			0.0006150
2	GLOBAL	-0.0044831	-0.0000112			0.0004273
3	GLOBAL	-0.0044820	0.0058768			-0.0000177
4	GLOBAL	-0.0044809	-0.0000106			-0.0003565
5	GLOBAL	-0.0044809	-0.0041776			-0.0005321

\$ PIN-FIXED SUPPORT CONDITION  
 CHANGES  
 JOINT RELEASES  
 6 MOM Z  
 DELETIONS  
 JOINT RELEASES  
 7 MOM Z  
 STIFFNES ANALYSIS  
 PRINT JOINT STATUS

\*\*\*\*\*  
 \* PROBLEM DATA FROM INTERNAL STORAGE \*  
 \*\*\*\*\*

JOB ID - FRAME	LENGTH	WEIGHT	ANGLE	TEMPERATURE	TIME
ACTIVE UNITS -	CM	KG	RAD	DEGF	SEC
JOINT				STATUS---/	
JOINT					
1				ACTIVE	
2				ACTIVE	
3				ACTIVE	
4				ACTIVE	
5				ACTIVE	
6				ACTIVE	
7				ACTIVE	

JOINT	RELEASES			ELASTIC SUPPORT RELEASES						
JOINT	FORCE	MOMENT	THETA 1	THETA 2	THETA 3	KFX	KFY	KFZ	KMX	KMY



6                    Z     0.0     0.0     0.0                    0.0                    0.0                    0.0                    0.0                    0.0

\*\*\*\*\*  
 \* END OF DATA FROM INTERNAL STORAGE \*  
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LIST DISP ALL

\*\*\*\*\*  
 \*RESULTS OF LATEST ANALYSES\*  
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PROBLEM - FRAME     TITLE - ANALYSIS

ACTIVE UNITS CM    KG    RAD    DEGF    SEC

ACTIVE STRUCTURE TYPE    PLANE    FRAME

ACTIVE COORDINATE AXES    X    Y

LOADING - 1                    UNIFORM LOAD ON BEAM

RESULTANT JOINT

DISPLACEMENTS -    SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0006146
7	GLOBAL	0.0	0.0			0.0

RESULTANT JOINT

DISPLACEMENTS -    FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0082435	0.0059043			-0.0006427
2	GLOBAL	-0.0082435	-0.0000281			-0.0006796
3	GLOBAL	-0.0082459	-0.0210991			-0.0000296
4	GLOBAL	-0.0082476	-0.0000269			0.0007979
5	GLOBAL	-0.0082476	0.0067749			0.0007646

LOADING - 2                    SIDESWAY CONC.LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS -    SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X ROT.	Y ROT.	Z ROT.			

		X DISP.	Y DISP.	Z DISP.	
6	GLOBAL	0.0	0.0		-0.0006635
7	GLOBAL	0.0	0.0		0.0

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0221359	0.0013558				-0.0001488
	GLOBAL	0.0221359	0.0000021				-0.0001488
	GLOBAL	0.0221357	0.0018157				-0.0001346
	GLOBAL	0.0221357	-0.0000021				-0.0001390
	GLOBAL	0.0221386	-0.0034356				-0.0003902

LOADING - 3

CONC. LOAD ON MIDSPAN OF BEAM

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0001363
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0018127	0.0013789				-0.0001518
	GLOBAL	-0.0018127	-0.0000028				-0.0001518
	GLOBAL	-0.0018122	-0.0005044				-0.0000062
	GLOBAL	-0.0018127	-0.0000026				0.0001762
	GLOBAL	-0.0018137	0.0015521				0.0001767

LOADING - 4

SIDESWAY LOAD ALONG THE COLUMN 6

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0002533
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

FREE JOINTS

DISPLACEMENTS -

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0088095	0.0007354			-0.0000808
	GLOBAL	0.0088095	0.0000006			-0.0000808
	GLOBAL	0.0088098	-0.0000885			0.0000374
	GLOBAL	0.0088101	-0.0000006			-0.0000690
	GLOBAL	0.0088101	-0.0000006			-0.0000690
	GLOBAL	0.0088101	-0.0006075			-0.0000690

LOADING - 5 SIDESWAY LOAD ALONG THE COLUMN 5

RESULTANT JOINT DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0013127
7	GLOBAL	0.0	0.0			0.0

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0299235	0.0009869			-0.0001087
	GLOBAL	-0.0299235	-0.0000019			-0.0001087
	GLOBAL	-0.0299200	-0.0052754			-0.0001211
	GLOBAL	-0.0299165	0.0000019			0.0005936
	GLOBAL	-0.0299165	0.0000019			0.0005936
	GLOBAL	-0.0299165	0.0052252			0.0005936

LOADING - 6 CON VERTICAL LOAD ON JOINT 1

RESULTANT JOINT DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			-0.0000412
7	GLOBAL	0.0	0.0			0.0

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0013565	-0.0021484			0.0002667
	GLOBAL	-0.0013565	-0.0000061			0.0001728

UNIF	GLOBAL	-0.0013561	0.0013789				-0.0000403
	GLOBAL	-0.0013558	0.0000006				-0.0000111
	GLOBAL	-0.0013558	-0.0000969				-0.0000111

LOADING - 7                    CONC VERTICAL LOAD ON JOINT 5

RESULTANT JOINT

		DISPLACEMENTS -			SUPPORTS		
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0001199
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

		DISPLACEMENTS -			FREE JOINTS		
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIF	GLOBAL	0.0034352	-0.0000969				0.0000107
	GLOBAL	0.0034352	0.0000007				0.0000107
	GLOBAL	0.0034352	0.0015521				0.0000462
	GLOBAL	0.0034356	-0.0000062				-0.0001962
	GLOBAL	0.0034356	-0.0022481				-0.0002840

LOADING - 10                    UNIF.LOAD ON BEAM+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

		DISPLACEMENTS -			SUPPORTS		
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				-0.0000489
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

		DISPLACEMENTS -			FREE JOINTS		
JOINT		/-----DISPLACEMENT-----/			/-----ROTATION-----/		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
UNIF	GLOBAL	0.0138924	0.0072601				-0.0007914
	GLOBAL	0.0138924	-0.0000259				-0.0008283
	GLOBAL	0.0138411	-0.0192856				0.0001051
	GLOBAL	0.0138399	-0.0000290				0.0004078
	GLOBAL	0.0138910	0.0033393				0.0003744

LOADING - 15                    VERTICAL LOAD ON BEAM MIDSPAN+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
6	GLOBAL	0.0	0.0				-0.0005271
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
1	GLOBAL	0.0203232	0.0027347				-0.0003006
4	GLOBAL	0.0203232	-0.0000007				-0.0003006
5	GLOBAL	0.0203235	-0.0032208				0.0001284
4	GLOBAL	0.0203239	-0.0000048				-0.0002135
5	GLOBAL	0.0203250	-0.0018836				-0.0002135

LOADING - 20

SIDESWAY LOAD ON J 5+SIDESWAY LOAD ON COLUMN #6

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
6	GLOBAL	0.0	0.0				-0.0009168
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			
1	GLOBAL	0.0309455	0.0020912				-0.0002295
4	GLOBAL	0.0309455	0.0000027				-0.0002295
5	GLOBAL	0.0309465	0.0017251				0.0001720
4	GLOBAL	0.0309476	-0.0000027				-0.0004591
5	GLOBAL	0.0309487	-0.0040432				-0.0004591

LOADING - 25

3 CONCENTRATED LOADS ON J 1&3&5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			X ROT.	Y ROT.	Z ROT.
		X DISP.	Y DISP.	Z DISP.			



JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0041575	-0.0044906			0.0005549
2	GLOBAL	0.0041575	-0.0000107			0.0003671
3	GLOBAL	0.0041586	-0.0058619			0.0000119
4	GLOBAL	0.0041597	-0.0000111			-0.0004146
5	GLOBAL	0.0041597	-0.0046900			-0.0005902

\$ FIXED-FIXED SUPPORT CONDITION  
 DELETIONS  
 JOINT RELEASES  
 6 MOM Z  
 STIFFNES ANALYSIS  
 PRINT JOINT STATUS

\*\*\*\*\*  
 \* PROBLEM DATA FROM INTERNAL STORAGE \*  
 \*\*\*\*\*

JOB ID - FRAME	JOB TITLE - ANALYSIS	ACTIVE UNITS - LENGTH CM	WEIGHT KG	ANGLE RAD	TEMPERATURE DEG F	TIME SEC
JOINT					STATUS---/	
JOINT						
1					ACTIVE	
2					ACTIVE	
3					ACTIVE	
4					ACTIVE	
5					ACTIVE	
6					ACTIVE	
7					ACTIVE	

JOINT	RELEASES	ELASTIC SUPPORT RELEASES								
JOINT	FORCE	MOMENT	THETA 1	THETA 2	THETA 3	KFX	KFY	KFZ	KMX	KMY
JOINT										

\*\*\*\*\*  
 \* END OF DATA FROM INTERNAL STORAGE \*  
 \*\*\*\*\*

LIST DISP ALL

\*\*\*\*\*  
 \*RESULTS OF LATEST ANALYSES\*  
 \*\*\*\*\*

PROBLEM - FRAME TITLE - ANALYSIS  
 ACTIVE UNITS CM KG RAD DEGF SEC  
 ACTIVE STRUCTURE TYPE PLANE FRAME  
 ACTIVE COORDINATE AXES X Y

LOADING - 1 UNIFORM LOAD ON BEAM

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	0.0	0.0			0.0

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0000697	0.0053969			-0.0005869
	GLOBAL	-0.0000697	-0.0000276			-0.0008237
	GLOBAL	-0.0000727	-0.0194188			-0.0000016
	GLOBAL	-0.0000757	-0.0000273			0.0006302
	GLOBAL	-0.0000757	0.0052985			0.0005969
	GLOBAL	-0.0000757	0.0052985			0.0005969

LOADING - 2 SIDESWAY CONC.LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	0.0	0.0			0.0

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.



1 MEMBER	GLOBAL	0.0133114	0.0019036				-0.0002090
	GLOBAL	0.0133114	0.0000017				-0.0002090
	GLOBAL	0.0133132	0.0000007				-0.0001044
	GLOBAL	0.0133150	-0.0000017				-0.0002091
	GLOBAL	0.0133161	-0.0018417				-0.0002091

LOADING - 3                    CONC. LOAD ON MIDSPAN OF BEAM

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 MEMBER	GLOBAL	0.0000007	0.0012663				-0.0001395
	GLOBAL	0.0000007	-0.0000027				-0.0001395
	GLOBAL	-0.0000000	-0.0046618				-0.0000000
	GLOBAL	-0.0000007	-0.0000027				0.0001395
	GLOBAL	-0.0000007	0.0012245				0.0001395

LOADING - 4                    SIDESWAY LOAD ALONG THE COLUMN 6

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 MEMBER	GLOBAL	0.0054409	0.0009446				-0.0001038
	GLOBAL	0.0054409	0.0000004				-0.0001038
	GLOBAL	0.0054416	-0.0007806				0.0000259
	GLOBAL	0.0054422	-0.0000004				0.0000002
	GLOBAL	0.0054422	0.0000010				0.0000002

LOADING - 5                    SIDESWAY LOAD ALONG THE COLUMN 5

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0124649	-0.0000969				0.0000105
2	GLOBAL	-0.0124649	-0.0000010				0.0000105
3	GLOBAL	-0.0124633	-0.0016885				-0.0000614
4	GLOBAL	-0.0124618	0.0000010				0.0002353
5	GLOBAL	-0.0124618	0.0020716				0.0002353

LOADING - 6 CON VERTICAL LOAD ON JOINT 1

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0019044	-0.0021144				0.0002630
2	GLOBAL	-0.0019044	-0.0000061				0.0001691
3	GLOBAL	-0.0019040	0.0012663				-0.0000421
4	GLOBAL	-0.0019036	0.0000007				0.0000002
5	GLOBAL	-0.0019036	0.0000021				0.0000002

LOADING - 7 CONC VERTICAL LOAD ON JOINT 5

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.

		X DISP.	Y DISP.	Z DISP.	
6	GLOBAL	0.0	0.0		0.0
7	GLOBAL	0.0	0.0		0.0

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	0.0018409	0.0000021			-0.0000002
	GLOBAL	0.0018409	0.0000007			-0.0000002
	GLOBAL	0.0018413	0.0012245			0.0000407
	GLOBAL	0.0018417	-0.0000061			-0.0001635
	GLOBAL	0.0018417	-0.0019601			-0.0002513

LOADING - 10 UNIF.LOAD ON BEAM+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	0.0	0.0			0.0

RESULTANT JOINT

DISPLACEMENTS - FREE JOINTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 2 3 4 5	GLOBAL	0.0132417	0.0073005			-0.0007959
	GLOBAL	0.0132417	-0.0000259			-0.0008328
	GLOBAL	0.0132405	-0.0194192			0.0001028
	GLOBAL	0.0132393	-0.0000290			0.0004211
	GLOBAL	0.0132404	0.0034568			0.0003878

LOADING - 15 VERTICAL LOAD ON BEAM MIDSPAN+SIDESWAY LOAD ON JOINT 5

RESULTANT JOINT

DISPLACEMENTS - SUPPORTS

JOINT	DISPLACEMENT			ROTATION		
	X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0			0.0
7	GLOBAL	0.0	0.0			0.0

RESULTANT JOINT

FREE JOINTS

JOINT		DISPLACEMENTS -					
		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 5	GLOBAL	0.0133120	0.0031699				-0.0003485
	GLOBAL	0.0133120	-0.0000011				-0.0003485
	GLOBAL	0.0133132	-0.0046612				0.0001044
	GLOBAL	0.0133144	-0.0000044				-0.0000696
	GLOBAL	0.0133154	-0.0006172				-0.0000696

LOADING - 20

SIDESWAY LOAD ON J 5+SIDESWAY LOAD ON COLUMN #6

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1 5	GLOBAL	0.0187523	0.0028482				-0.0003128
	GLOBAL	0.0187523	0.0000021				-0.0003128
	GLOBAL	0.0187548	-0.0007800				0.0001303
	GLOBAL	0.0187573	-0.0000021				-0.0002089
	GLOBAL	0.0187583	-0.0018407				-0.0002089

LOADING - 25

3 CONCENTRATED LOADS ON J 1&3&5

RESULTANT JOINT		DISPLACEMENTS - SUPPORTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT		DISPLACEMENTS - FREE JOINTS					
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0000628	-0.0008460				0.0001234
2	GLOBAL	-0.0000628	-0.0000082				0.0000295

6	GLOBAL	-0.0000627	-0.0021710				-0.0000014
7	GLOBAL	-0.0000626	-0.0000081				-0.0000239
8	GLOBAL	-0.0000626	-0.0007335				-0.0001117

LOADING - 30                    3 @ 10N CONCENTRATED LOADS ON JOINTS 1&3&5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0001257	-0.0016919				0.0002467
2	GLOBAL	-0.0001257	-0.0000164				0.0000589
3	GLOBAL	-0.0001254	-0.0043420				-0.0000028
4	GLOBAL	-0.0001252	-0.0000163				-0.0000478
5	GLOBAL	-0.0001252	-0.0014671				-0.0002234

LOADING - 35                    2 10N CONCENTRATED LOADS ON JOINTS 1&5

RESULTANT JOINT

		DISPLACEMENTS - SUPPORTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
6	GLOBAL	0.0	0.0				0.0
7	GLOBAL	0.0	0.0				0.0

RESULTANT JOINT

		DISPLACEMENTS - FREE JOINTS			ROTATION		
JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.0001270	-0.0042246				0.0005256
2	GLOBAL	-0.0001270	-0.0000109				0.0003379
3	GLOBAL	-0.0001254	0.0049817				-0.0000028
4	GLOBAL	-0.0001239	-0.0000108				-0.0003267
5	GLOBAL	-0.0001239	-0.0039161				-0.0005023

FINISH

GOOD-BYE

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